

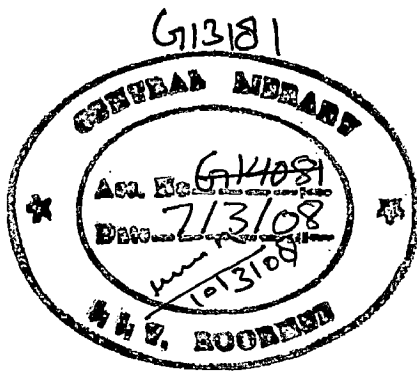
HYDROLOGICAL ANALYSIS FOR A MULTI-PURPOSE WATER RESOURCES PROJECT - A CASE STUDY

A DISSERTATION

*Submitted in partial fulfillment of the
requirements for the award of the degree
of*
MASTER OF TECHNOLOGY
in
WATER RESOURCES DEVELOPMENT

By

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JUNE, 2007**

CANDIDATE'S DECLARATION

I hereby declare that the dissertation titled “**HYDROLOGICAL ANALYSIS FOR A MULTI-PURPOSE WATER RESOURCES PROJECT – A CASE STUDY**”, which is being submitted in partial fulfillment of the requirement of the award of the Degree of Master of Technology in Water Resources Development (Civil) at Department of Water Resources Development and Management (WRD&M), Indian Institute of Technology Roorkee (IIT Roorkee), is an authentic record of my own work carried out during 2006 - 2007 under the supervision and guidance of **Dr. M.L. Kansal**, Associate Professor, WRD&M Dept., IIT Roorkee – India.

I have not submitted the matter embodied in this dissertation for the award of any other degree.

Place: IIT Roorkee,

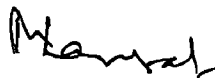
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CERTIFICATE

This is to certify that the above statement made by the candidate is correct to the best of my knowledge.



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(Eko Murwanto)

SYNOPSIS

Many important water resources projects in the past have failed due to improper assessment of the hydrological conditions. Therefore, the hydrological analysis for a water resources project should of necessity precede structural and other detailed design studies. It involves the collection of relevant data and analysis of the data by applying the principles and theories of hydrology to seek solutions to practical problems. The probabilistic analysis of hydrology data is an important component of present day hydrological studies and enables the engineer to take suitable design decision consistent with economic and other criteria to be taken in a given project.

In many hydrological studies, the average rainfall over a specified area due to a storm, or due to number of storms, is required. For determination of the average precipitation over an area, a large number of raingauges are installed. The rainfall data of various raingauges is used to estimate the Mean Areal Precipitation over a catchment area.

It is well-known that streamflow varies over a water year. One of the popular methods of studying this streamflow variability is through flow duration curve. Flow Duration Curves find considerable use in water resources planning and development activities. One of the important uses is evaluating various dependable flows in the planning of water resources engineering projects.

In planning water resources projects, the peak magnitude of the flood have to be adopted properly. If the selected design flood is too high, it results in a unnecessary costly structure while adoption of a low design flood can results in the loss of the structure itself causing thereby a disaster to the people residing downstream, besides damaging valuable immovable properties.

Sedimentation of reservoir is a natural process, since large part of the silt eroded from the catchment and transported by the river, gets deposited on the bed of the reservoir. The deposition of the sediment will automatically reduce the water storing capacity of the reservoir, and if this process of deposition continues longer, a stage is likely to reach when the whole reservoir may get silted up and become useless. Therefore, it becomes necessary to estimate the sedimentation rates in planning and executing suitable measures for controlling sedimentation in order to prolong the life of the reservoir and its benefits.

This dissertation deals with a hydrological analysis approaches for the case study in planning a Multi-purpose Water Resources Project on Gola river near Kathgodam in district Nainital at Jamrani in the state of Uttarakhand, India. The study has been carried out to estimate Mean Areal Precipitation (MAP) over the given catchment, derive discharge data for Dam site, determine 75 % dependable annual water availability at Dam site by Flow Duration Curve, estimate Design Flood at Dam site and to estimate sedimentation flow rate at the Dam site.

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CHAPTER 1

INTRODUCTION

1.1. GENERAL

All life on earth is dependent, one way or another, on water. The study of the science of water is, therefore, important. Water occurs on the earth in all its three states, viz. liquid, solid and gaseous, and in various degrees of motion. Evaporation of water from water bodies such as oceans and lakes, formation and movement of clouds, rain and snowfall, streamflow and groundwater movement are some examples of the dynamic aspects of water. The various aspects of water related to the earth can be explained in terms of a cycle known as the hydrologic cycle.

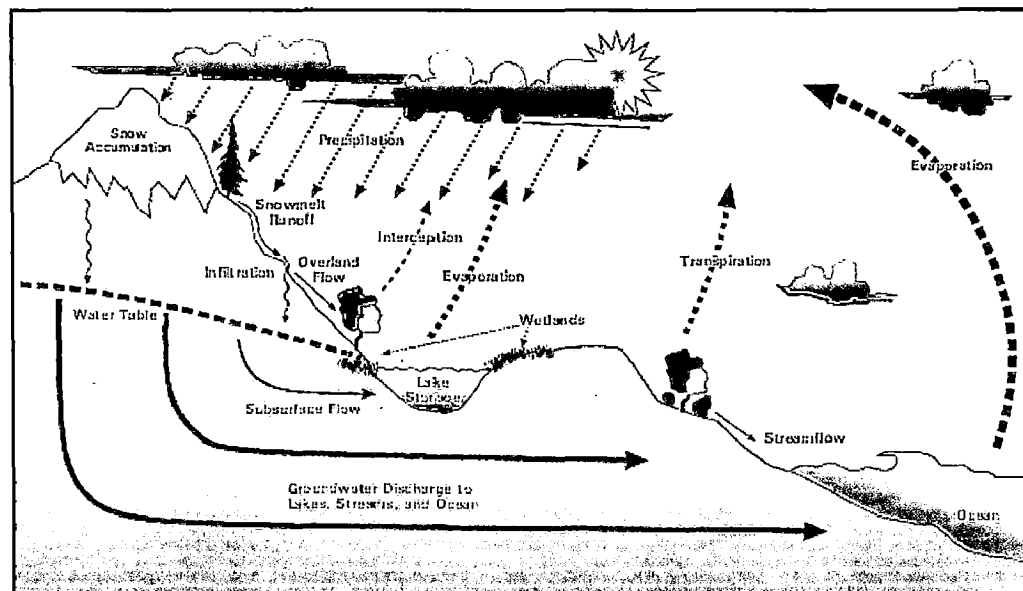


Fig. 1.1 Model of a Hydrologic Cycle

A convenient starting point to describe the cycle is in the oceans. Water in the oceans evaporates due to the heat energy provided by solar radiation. The water vapour moves upward and form clouds. While much of the clouds condense and fall back to the oceans as rain, a part of the clouds is driven to the land areas by winds.

There, they condense and precipitate onto the landmass as rain, snow, hail, sleet, etc. A part of the precipitation may evaporate back to the atmosphere even while falling. Another part may be intercepted by vegetation, structures and other such surface modifications from which it may be either evaporated back to atmosphere or move down to the ground surface.

A portion of the water that reaches the ground enters the earth's surface through infiltration, enhance the moisture content of the soil and reach the groundwater body. Vegetation sends a portion of the water from under the ground surface back to the atmosphere through the process of transpiration. The precipitation reaching the ground surface after meeting the needs of infiltration and evaporation moves down the natural slope over the surface and through a network of gullies, streams and rivers to reach the ocean. The groundwater may come to the surface through springs and other outlets after spending a considerably longer time than the surface flow. The portion of the precipitation which by a variety of paths above and below the surface of the earth reaches the stream channel is called runoff. Once it enters a stream channel, runoff becomes stream flow.

The variation of streamflow in space and time plays an important role in the design and operation of water-resources engineering projects, such as those for irrigation, water supply, flood control, water power and navigation. In all these projects hydrological investigations for the proper assessment of the streamflow is necessary. The hydrological study of a project should of necessity precede structural and other detailed design studies. It involves the collection of relevant data and analysis of the data by applying the principles and theories of hydrology to seek solutions to practical problems.

Many important projects in the past have failed due to improper assessment of the hydrological factors. Some typical failures of hydraulic structures are:

- Overtopping and consequent failure of an earthen dam due to an inadequate spillway capacity,

- Inability of a large reservoir to fill up with water due to overestimation of the stream flow. Such failure, often-called hydrologic failure underscore the uncertainty aspect inherent in hydrological studies, and
- Failure of bridges and culverts due to excess flood flow.

Various phases of the hydrological cycle, such as rainfall, runoff, evaporation and transpiration are all non-uniformly distributed both in time and space. Further, practically all hydrologic phenomena are complex and at the present level of knowledge, they can at best be interpreted with the aid of probability concepts. Hydrological events are treated as random processes and the historical data relating to the event are analysed by statistical methods to obtain information on probabilities of occurrence of various events. The probabilistic analysis of hydrologic data is an important component of present-day hydrological studies and enables the engineer to take suitable design decisions consistent with economic and other criteria to be taken in a given project.

1.2. OBJECTIVE AND SCOPE OF STUDY

The main objective of this study is to collect and carry out the hydrological analysis for planning a proposed Multipurpose Water Resources Project on Gola River near Kathgodam in district Nainital near Jamrani village in the state of Uttarakhand, India.

The study has been carried out with following scope :

- to estimate Mean Areal Precipitation (MAP) over the Gola river catchment based on rainfall data recorded at a number of raingauges installed in the area.
- to derive relationship between the discharge at Jamrani Dam site and the discharge at Gola Barrage by regression analysis.

- to determine 75% dependable annual water availability at Jamrani Dam site and to draw a Flow Duration Curve at Jamrani Dam site.
- to estimate flood at Jamrani Dam Site based on annual peak flow from the derived data at Jamrani Dam site.
- to estimate sedimentation flow rate at Jamrani Dam site based on the observed sediment data and the derived discharge at Jamrani Dam site.

1.3. SOURCES OF DATA

The main components of the hydrological cycle are rainfall (precipitation), evaporation, transpiration, infiltration, runoff and ground water. Depending upon the problems at hand, a hydrologist would require data relating to the various relevant phases.

The hydro-meteorological data normally required are :

1. Weather records : temperature, humidity and wind velocity,
2. Precipitation data,
3. Stream-flow records,
4. Evaporation and transpiration data,
5. Infiltration characteristics of the area,
6. Groundwater characteristics, and
7. Physical and geological characteristics of the area under consideration.

In India, meteorological data, including weather and rainfall data, are collected by the India Meteorological Department (IMD) and by some state government agencies. Stream flow data of various rivers and streams are usually available with the state irrigation department. Flow in major rivers are monitored by Central government agencies such as the Central Water Commission (CWC). Groundwater data is normally available with the Central Groundwater Board and State Government groundwater development organisations. Data on evaporation, transpiration and infiltration is available

with State Government organisations, such as the Irrigation Department and Department of Agriculture. The physical data of the basin can be obtained from the study of topographical maps available with the Survey of India. The geological characteristics of the basin can be obtained from Geological Survey of India and the State Geology Directorate.

1.4. ORGANISATION OF DISSERTATION REPORT

The study is presented in seven chapters. The contents of these chapters are briefly outlined below.

CHAPTER 1 : It deals with the general introduction of the subject and highlights the objective of the study.

CHAPTER 2 : It presents theories which are adopted in a hydrological analysis for water resources planning.

CHAPTER 3 : It describes details of the study area on Gola River near Kathgodam in district Nainital, Uttarakhand, India and the data availability of its hydro meteorological data.

CHAPTER 4 : It deals with estimation for Mean Areal Precipitation, water availability and the correlation between Jamrani Dam and Gola Weir. It further determines dependable annual flow at the dam site.

CHAPTER 5 : It deals with the estimation of the Design Flood which can be used in the water resources project planning.

CHAPTER 6 : It discusses a process of the deposition of sediment in the reservoir is known as Reservoir Sedimentation, and to estimate annual sedimentation rate of the proposed reservoir at the dam site.

CHAPTER 7 : It gives the summary and the conclusion of the study.



CHAPTER 2

HYDROLOGICAL ANALYSIS IN WATER RESOURCES PLANNING

2.1. INTRODUCTION

The water resources structures that we normally come across in practice are storage structures like dams, barrages, regulators, irrigation conveyance channels, bridges, etc. To plan these structures we need one or more of the following informations :

- (i) Rainfall data of the catchment,
- (ii) Catchment characteristics and the land use pattern,
- (iii) Daily stream flows for determining the storage capacity of a reservoir,
- (iv) Flood of certain frequency,
- (v) Intake rate of sedimentation to estimate useful life of the reservoir.

As rainfall and general climatic condition affects our daily lives more directly than does streamflow, the other data in the form of records of rainfall, temperature, humidity, and pressure etc. are initiated long before streamflow records are obtained. Furthermore rainfall and general climatic records require little skill and training on the part of the observer, whereas reliable streamflow records demand the services of an engineer or at least of a skilled technician. Consequently, long-term data is available in respect of rainfall and climatic records for most of the drainage basins in India, whereas streamflow records are comparatively few and far between. Sometimes, good records are available on the main stream, whereas the problem at hand calls for a knowledge of the water availability of a tributary from the site of the available records or vice versa.

In planning water resources projects, the peak magnitude of the flood that have to be adopted in the design of irrigation, hydro-power and flood-control projects are of great importance to an engineer. If the selected design flood is too high, it results in a conservative and costly structure while

adoption of a low design flood will cause damage. It will result in the loss of the structure itself causing thereby untold misery to the people residing downstream, besides damaging valuable immovable properties. The catchment characteristics helps in knowing the sediment rate and hence the useful life of the reservoir.

2.2. MEAN AREAL PRECIPITATION (MAP) ESTIMATION

The Rainfall recorded by a raingauge represents only the rainfall at that station. It is also called the *Point Rainfall*. In many hydrological studies, the average of rainfall over a specified area due to a storm, or due to a number of storms, is required. For determination of the average precipitation over an area, a large number of raingauge are installed. To convert the point rainfall values at various stations into an average value over a catchment the following three methods are in use :

1. Arithmetic Mean Method
2. Thiessen Polygon Method
3. Isohyetal Map Method

1. Arithmetic Mean Method

This method is suitably applied for a basin where the gauges are uniformly distributed and the individual gauge catches do not vary much from the mean. The basin should be a reasonably flat area. The assumption made is that all gauges weight equally. This method gives fairly good results if the topographic influences on precipitation and aerial representativeness are considered while selecting the gauge site. It is the simplest form in which the average of precipitation over the basin is obtained by taking simple arithmetic mean of all the gauged

amounts within the basin :
$$P_{av} = \frac{P_1 + P_2 + \dots + P_n}{n} = \frac{1}{n} \sum_{i=1}^n P_i \quad (2.1)$$

where P_1, P_2, \dots, P_n are the precipitation recorded by n number of gauges located within the basin. This method gives a rough estimate of the average precipitation. It does not account for the topographic and other influences. For use of this method, no gauge station located outside the boundary of the watershed should be considered. This method does not give accurate results and hence is rarely used in practice.

2. Thiessen Polygon Method

In the Thiessen Polygon Method, the rainfall recorded at each rain gauge stations is given a weightage on the basis of the area which it represents. This method is better than the Arithmetic Mean Method which gives equal weightage to all the stations. The following procedure is used.

Procedure :

- (i) The positions of the rain gauge stations are marked on the plan of the catchment area over which the average rainfall depth is required.
- (ii) The adjacent rain gauge stations are joined by straight lines; thus the entire area is divided into a series of triangles (see Fig. 2.1). The rain gauge stations which are outside the catchment area but are in its neighbourhood should also be considered.
- (iii) Perpendicular bisectors are then drawn on the connecting lines to form polygons around stations. Each polygon contains only one rain gauge stations. The entire area of a particular polygon is nearer to the rain gauge stations contained therein than to any other rain gauge stations. Thus, each polygon represents the area of the influence of that rain gauge station.
- (iv) For determination of the average of rainfall, the boundary of the catchment is taken as the outer limit of the Thiessen polygons. The areas of the polygons are determined either with a planimeter or with an overlay grid.

(v) The average precipitation of the area is given by

$$P_{av} = \frac{P_1.A_1 + P_2.A_2 + \dots + P_n.A_n}{A_1 + A_2 + \dots + A_n} \quad (2.2)$$

or

$$P_{av} = \frac{\sum P_i A_i}{\sum A_i} = \frac{\sum P_i A_i}{A} \quad (2.2a)$$

where A_1, A_2, \dots, A_n are the areas of the Thiessen polygons representing the stations 1, 2,n and P_1, P_2, \dots, P_n are the corresponding precipitations, and A is the total area.

Sometimes above equations are written as $P_{av} = \sum P_i \left(\frac{A_i}{A} \right)$. (2.2b)

The ratio (A_i / A) is called the *weightage factor* for the stations.

Thus, weightage is given to various stations according to the influence areas. In this method, the stations situated outside the catchment area are also effectively used. Once the weightage factors have been determined, the computation of the average precipitation for different storms becomes easy. However, the method has one drawback that it is inflexible. A new Thiessen polygon network is required when there is a change in the location of the raingauges or when new rain gauge stations are set up. Moreover, no adjustment can be made for variations due to altitudes and other factors. However the advantage of this method is that, it is much more accurate than the previous methods and the procedure of computation becomes simple, once the areas of the polygons are measured. This method is popularly applied to most of the field problems.

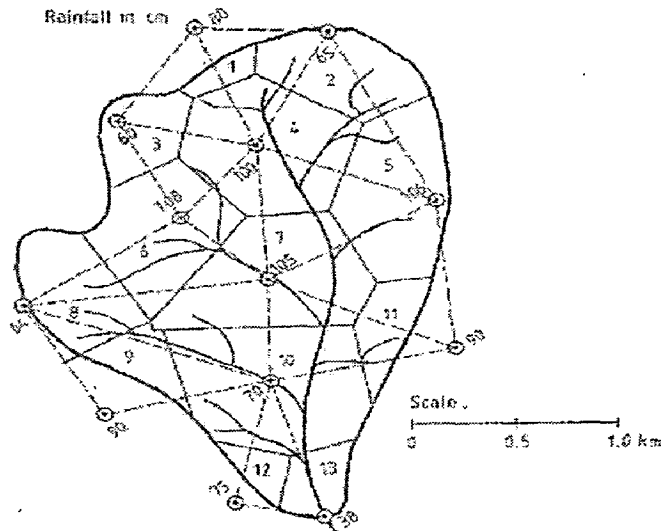


Fig.2.1 Typical Thiessen Polygon map

3. Isohyetal Map Method

Isohyets are the contours of equal rainfall depth. For plotting of an isohyetal map, rain gauge stations are marked on the plan of the catchment. The rainfall recorded at these stations is also marked on the plan. The rain gauge stations located outside the catchment but in its neighbourhood are also marked. The isohyets of various rainfall depths are then drawn by interpolating the values of the rainfall. Generally, a linear variation of rainfall depth is assumed between the two rain gauge stations. The procedure is similar to one used for drawing of the elevation contours from the spot levels in surveying. Fig. 2.2 shows a typical isohyetal map. For determination of the average rainfall depth, the catchment area between adjacent isohyets are measured. The catchment boundary is taken as the extreme limit of the areas. The average depth of the rainfall of an area between two adjacent isohyets is usually taken equal to the mean of the two isohyets values.

For the area between the last isohyet and the boundary of the catchment, a suitable value of the average depth should be taken, depending upon

the distance of the outside isohyet and the actual boundary. The average of the rainfall of the entire catchment is given by

$$P_{av} = \frac{P_{12}A_{12} + P_{23}A_{23} + \dots + P_{(n-1),n}A_{(n-1),n}}{A_{12} + A_{23} + \dots + A_{(n-1),n}} \quad (2.3)$$

or

$$P_{av} = \frac{\sum P_{ij} A_{ij}}{\sum A_{ij}} \quad (2.3a)$$

where P_{12} is the average precipitation over the area A_{12} between two adjacent isohyets marked 1 and 2 and so on.

The isohyetal map method is the most accurate method. In this method, all the relevant data is fully utilised and properly interpreted. Moreover, the method can be used to make adjustments for variations in altitudes of the stations due to orographic influences.

Hence while constructing the isohyetal map for a hilly region, the interpolation between gauge stations can be done according to the difference in elevations of two stations, and not according to the distance between them. This is more logical because the precipitation generally depends upon the altitude. However, the method has disadvantage that it is laborious and not easily adaptable to a computer.

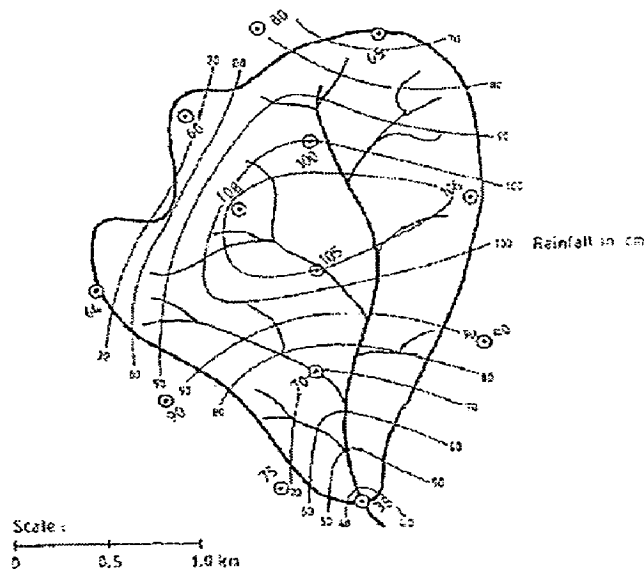


Fig. 2.2. Typical Isohyetal Map

2.3. REGRESSION ANALYSIS

The technique of regression analysis is a procedure for fitting an equation to a set of data. This techniques are used to identify the mathematical dependence between the observed values of physically related variables and thus can account for the additional information contained in the correlated sequences of events. However, the correlation between the desired statistical parameter as the dependent variable should be attempted only with the other physically and climatically related variable of the basin or region as independent variables. The procedure is certainly better than using relatively short historical sequences in hydrologic analyses.

Regression analysis determines the relationships between a *dependent* variable and an *independent* variable. The general regression formula between the independent variable x , and the dependent variable y , is given as

$$y = b_0 + b_1x + b_2x^2 + \dots + b_nx^n + \varepsilon \quad (2.4)$$

The constraints b_0, b_1, \dots, b_n are unknown parameters to be determined from available data. The random error ε has zero mean and a constant standard deviation. The simplest form of the regression model assumes that the dependent variable varies linearly with the independent variable, that is,

$$y^* = a + bx \quad (2.5)$$

The constant a and b are determined using **the least-square criterion**, which minimizes the sum of the square of the differences between observed and estimated values. Given the i -th raw data point (y_i, x_i) , $i = 1, 2, \dots, n$, the sum of the squares of the deviations between observed and estimated value is defined as

$$S = \sum_{i=1}^n (y_i - a - bx_i)^2 \quad (2.6)$$

The values of a and b are determined by solving the following necessary conditions for the minimization of S :

$$\frac{\partial S}{\partial a} = -2 \sum_{i=1}^n (y_i - a - bx_i) = 0 \quad (2.7)$$

$$\frac{\partial S}{\partial b} = -2 \sum_{i=1}^n (y_i - a - bx_i)x_i = 0 \quad (2.8)$$

After some algebraic manipulations, we obtain the following solution :

$$b = \frac{\sum_{i=1}^n y_i x_i - n\bar{y}\bar{x}}{\sum_{i=1}^n x_i^2 - n\bar{x}^2} \quad (2.9)$$

$$a = \bar{y} - b\bar{x} \quad (2.10)$$

where

$$\bar{x} = \frac{\sum_{i=1}^n x_i}{n}, \bar{y} = \frac{\sum_{i=1}^n y_i}{n} \quad (2.11)$$

The equations show that we need to compute b first, from which a can be computed. The estimates of a and b are valid for any probability distribution of y_i . However, if y_i is normally distributed with a constant standard deviation, a confidence interval can be established on the mean value of the estimator at $x = x^0$ (i.e., $y^0 = a + bx^0$) as

$$(a + bx^0) \pm t_{\frac{\alpha}{2}, n-2} \sqrt{\frac{\sum_{i=1}^n (y_i - y_i^*)^2}{n-2}} \sqrt{\frac{1}{n} + \frac{(x^0 - \bar{x})^2}{\sum_{i=1}^n x_i^2 - n\bar{x}^2}} \quad (2.12)$$

For future (predicted) values of the dependent variable, y , we are interested in determining its **prediction interval** (rather than the confidence interval on its mean value). As would be expected, the prediction interval of a future value is wider than the confidence interval on the mean value. Indeed, the formula for the prediction interval is the same as that of the confidence interval except that the term $\frac{1}{n}$ under the second square root is replaced with

$$\frac{(n+1)}{n}$$

We can test how well the linear estimator $y^* = a + bx$ fits the raw data by computing the **correlation coefficient**, r , using the formula

$$r = \frac{\sum_{i=1}^n y_i x_i - n\bar{y}\bar{x}}{\sqrt{(\sum_{i=1}^n x_i^2 - n\bar{x}^2)(\sum_{i=1}^n y_i^2 - n\bar{y}^2)}} \quad (2.13)$$

where $-1 \leq r \leq 1$.

If $r = \pm 1$, then a perfect linear fit exist between x and y . In general, the closer the value of $|r|$ to 1, the better the linear fit. If $r = 0$, then y and x may or not be independent, in the sense that two dependent variables may yield $r = 0$.

2.4. RAINFALL RUNOFF MODELLING

Rainfall is the source of water for runoff generation in the channel. During occurrence of rainfall, a part of it is intercepted by the vegetations, buildings and several objects lying over the ground surface, which does not reach the land surface. Few portion of rainfall also infiltrates into the soil, and lastly the rest amount of rainfall makes some depth of water over the ground surface, which tends to move from one place to another under effect of land gradient is called overland flow, which ultimately meets to the streams, channels etc. is known as runoff. Thus, runoff may be defined as that portion of rainfall which flows through the rivers, streams, nala etc.

Runoff Computation.

The commonly used methods to compute runoff rate are :

1. Rational Method,
2. SCS-Curve Number Method,
3. Hydrograph Method.

1. Rational Method

It is a common method for computing the peak runoff rate from small catchments. The peak runoff refers to the discharge rate used for design of hydraulic structures, that must carry the runoff. Rational method involves following formula for computing the peak runoff :

$$Q_p = \frac{C.I.A}{360} \quad (2.14)$$

where, Q_p = peak runoff (m^3/s)
 C = runoff coefficient
 I = rainfall intensity (mm/hr)
 A = area of catchment (ha).

2. SCS Curve Number Method

This method also known as *hydrologic soil cover complex number method*, was developed by Ogrosky and Mockus (1957) for determining peak rate of runoff from small catchments. A runoff *curve number* (CN) is developed through field studies by measuring runoff from different soils at various locations. The antecedent moisture condition and the physical characteristics of the watershed are correlated to give hydrologic soil groups. Soil of any catchments can be classified into the following four hydrologic soil groups.

- Group A* : It is characterized by low runoff generating potential. This group of soils are mainly constituted by sands or gravel particles. These soils involve high infiltration rate.
- Group B* : Soils of this group have moderate infiltration rate, even if they are thoroughly wetted. Soils are composed of moderately fine to moderately coarse particles.
- Group C* : This group of soil contains low infiltration rate. The soil may be composed of a thin hard layer which impedes the downward movement of water. Soils are generally constituted with moderately fine to fine particles.
- Group D* : This group of soils are characterized by high runoff potential due to very low infiltration rate. Clay soil is the example of this group.

Antecedent Moisture Content (AMC) is wetness status of the catchment. There are following three levels of AMC.

- AMC-I : It refers to the lowest runoff generating potential of the catchment because of dryness of the soil.
- AMC-II : Average moisture status regarding runoff generating potential.
- AMC-III : It refers highest runoff generating potential of the catchment, because of saturation of soil from antecedent rains.

In India, most of rain gauges are non-recording type. Rainfall data obtained from such non-recording gauges have been used to establish rainfall runoff relationships. The following relation is normally used for small catchments:

$$\frac{\text{Actual retention of rainfall}}{\text{Potential max. retention}} = \frac{\text{Direct runoff}}{\text{Rainfall} - \text{Initial abstraction}}$$

or
$$\frac{P - I_a - Q_d}{S} = \frac{Q_d}{P - I_a}$$

or
$$Q_d = \frac{(P - I_a)^2}{(P - I_a + S)} = \frac{(P - 0.2S)^2}{(P + 0.8S)} \quad (2.15)$$

where : Q_d is the runoff depth (cm) over the catchment,
 P is the mean rainfall (cm) over the catchment,
 S is the potential maximum retention (cm),
 I_a is the initial losses consisting of interception, depression storage and infiltration.

For black soil, I_a is taken as 0.1 S for AMC-II and III type, I_a is taken as 0.3 S for AMC-I type. For all other types, I_a is 0.2 S. In general the value of I_a is taken as 0.2 S. For all soil regions of India, except black soil region of

AMC-II and III, Q_d is calculated as
$$Q_d = \frac{(P - 0.3S)^2}{(P + 0.7S)} \quad (2.16a)$$

For black soil of AMC-II and III
$$Q_d = \frac{(P - 0.1S)^2}{(P + 0.9S)} \quad (2.16b)$$

Now, S (cm) is related to curve number (CN) as follows :

$$CN = \frac{2540}{25.4 + S} \quad (2.17)$$

The peak discharge in m^3/s can be calculated from the following relation

$$Q_p(m^3/s) = \frac{0.0208 \times A \times Q_d}{t_p} \quad (2.18)$$

where for small catchments, time to peak t_p in h is obtained as

$$t_p = 0.6t_c + t_c^{1/2} \quad (2.19)$$

where A is the catchment area (Ha), Q_d is the depth of runoff (cm) is obtained from equation (2.15) or (2.16) depending on the type of soil and Q_p is the peak discharge (m^3/s).

3. Hydrograph Method

Hydrograph is the graphical presentation of instantaneous runoff rate against time. It is used for computing the runoff from the catchment. The unit hydrograph is mainly used for the purpose. The hydrograph will be described in detail in sub-Chapter 2.6.2.

2.5. FLOW DURATION CURVE

A Flow Duration Curve can be drawn if a continuous record of daily flow (or average weekly or average monthly flow) is available for a long period. The flow duration curve represents the data in a condensed form and is extremely useful for the estimation of the available power.

The flow duration curves drawn from the record of the daily flow will be somewhat different from that obtained from the record of the average weekly flow or average monthly flow. It is observed that the shape of the curve depends upon the variability of the flow data. Because the variation of the daily flow is larger than that of the weekly flow, the flow duration curve computed from the average daily flow will show greater variability than the one from the average weekly flow. Likewise, the flow duration curve computed from the average weekly flow would show greater variability than that from the average monthly flow.

The flow duration curve drawn from the mean weekly flow rates will be more approximately than that drawn from the mean daily flow, because there is considerably variation in the flow which is not indicated in the mean weekly flow rates. Likewise, the error involved in the use of mean monthly flow rates is more than that in the use of mean weekly flow rates. The error in the use of the monthly flow rates may vary from 5 to 15 percent, depending upon the characteristics of the stream and the extent of utilisation of flow. However, the difference between the daily flow-duration curve and the monthly flow-duration curve will be negligible for a stream with a constant steady discharge. On the other hand, the difference would be quite large for very flashy streams.

Procedure for drawing flow-duration curve

A Flow duration curve is a plot between the discharge as ordinate and the percentage of time that discharge is equalled or exceeded as abscissa (see Fig. 2.3). Before drawing the flow-duration curve, the stream flow data are collected, as explained earlier. Let us assume that the average daily flow data are available for one year (365 days) for which the flow-duration curve is to be drawn. The following procedure is used for plotting the flow-duration curve from the daily flow.

1. From the available record, the mean daily flow rates differing by a convenient magnitude. These flow rates are entered in column (1) of Table, in the order of descending magnitude.
2. The number of days a particular discharge has equalled or exceeded out of 365 days is entered in column(2) of the table. Of course, the lowest discharge would be equalled or exceeded all the 365 days.
3. The percentage of time a particular discharge is equalled or exceeded is computed from the relation.

$$P = \left[\frac{m}{n} \right] \times 100 \quad (2.20)$$

Where n is the total number of events, m is the number of times a particular event (in this case, discharge) has equalled or exceeded. The value of m is obtained from column (2) of the table. The values of p are entered in column (3).

4. A plot is then made between the discharge Q as ordinate and the percentage of time p as abscissa (Fig. 2.3). The lowest discharge would of course be exceeded 100% of time.

Similar curves can be prepared using the mean weekly, mean monthly or mean annual flow rates. This method of computation is known as the Total Period Method.

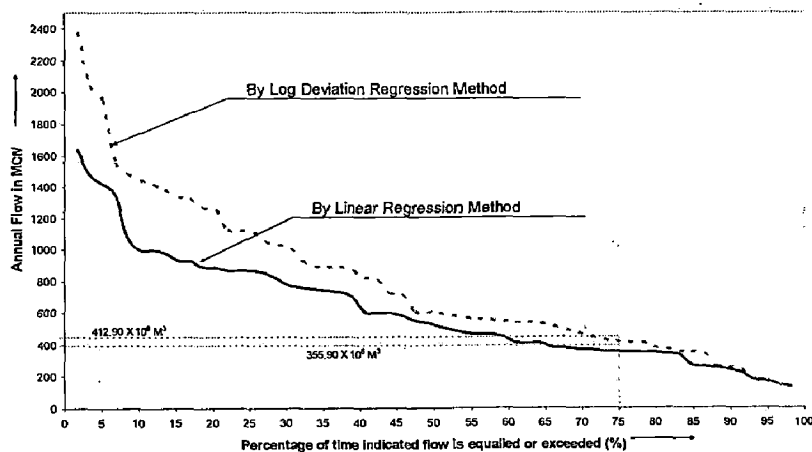


Fig. 2.3. Typical Flow Duration Curve

2.6. FLOOD ESTIMATION

A Flood is an unusually high stage in a river, normally the level at which the river overflows its bank and inundates the adjoining area. The damages caused by floods in terms of loss of life, property and economic loss due to disruption of economic activity are all too well-known. Crores of rupees are spent every year in flood control and flood forecasting. The hydrograph of extreme floods and stages corresponding to flood peaks provide valuable data for purposes of hydrologic design. Further, of the various characteristics of the flood hydrograph, probably the most important and widely used parameter is **the flood peak**. At a given location in a stream, flood peaks vary from year to year and their magnitude constitutes a hydrologic series which enable one to assign a frequency to a given flood peak value. In the design of practically all hydraulic structures the peak flow that can be expected with an assigned frequency (say 1 in 100 years) is of primary importance to adequately proportion the structure to accommodate its effect. The design of bridges, culvert waterways and spillways for dams and estimation of scour at a hydraulic structure are some examples wherein flood peak values are required.

To estimate the magnitude of a flood peak the following alternative methods are available :

1. Empirical Method,
2. Unit Hydrograph Technique,
3. Flood Frequency Analysis.

The use of a particular method depends upon

- (i) the desired objective,
- (ii) the available data,
- (iii) the importance of the project.

1. Empirical Method

The empirical method used for the estimation of the flood peak are essentially regional formulae based on statistical correlation of the observed peak and important catchment properties. To simplify the form of the equation, only a few of the many parameters affecting the flood peak are used. For example, almost all formulae use the catchment area as a

parameter affecting the flood peak and most of them neglect the flood frequency as a parameter. In view of these, the empirical formula are applicable only in the region from which they were developed and when applied to other areas they can at best give approximate values.

Flood-Peak-Area Relationships

By far the simplest of the empirical relationships are those which relate the flood peak to the drainage area. The maximum flood discharge Q_p from a catchment area A is given by these formulae as

$$Q_p = f(A) \quad (2.21)$$

While there are a vast number of formulae of this kind proposed for various parts of the world, only a few popular formulae used in various parts of India are given below,

- Dickens Formula (1865)

$$Q_p = C_D \cdot A^{3/4} \quad (2.22)$$

Where Q_p = maximum flood discharge (m^3/s)
 A = catchment area (km^2)
 C_D = Dickens constant with value between 6 to 30

The following are some guidelines in selecting the value of C_D :

	Value of C_D
North-Indian plains	6
North-Indian hilly regions	11 – 14
Central India	14 – 28
Coastal Andhra and Orissa	22 - 28

For actual use the local experience will be of aid in the proper selection of C_D .

Dickens formula is used in the central and northern parts of the country.

- Ryves Formula (1884)

$$Q_p = C_R \cdot A^{2/3} \quad (2.23)$$

Where Q_p = maximum flood discharge (m^3/s)
 A = catchment area (km^2)
and C_R = Ryves coefficient

This formula originally developed for the Tamil Nadu region, is in use in Tamil Nadu and parts of Karnataka and Andhra Pradesh. The values of C_R recommended by Ryves for use are :

$$\begin{aligned} C_R &= 6.8 \text{ for areas within 80 km from the east coast} \\ &= 8.5 \text{ for areas which are 80 – 160 km from the east coast} \\ &= 10.2 \text{ for limited areas near hills} \end{aligned}$$

However, various major reservoir projects built in Tamil Nadu since 1950 have adopted considerably much higher values of C_R than the above.

- Inglis Formula (1930)

This formula is based on flood data of catchment in Western Ghats in Maharashtra.

The flood peak Q_p in m^3/s is expressed as

$$Q_p = \frac{124 A}{\sqrt{(A + 10.4)}} \quad (2.24)$$

where A is the catchment area in km^2 .

Above equation with small modification is the constant in the numerator (124) is in use Maharashtra for designs in small catchments.

- Other Formulae

There are many such empirical formulae developed in various parts of the world.

There are some empirical formulae which relate the peak discharge to the basin area and also include the flood frequency. Fuller's formula (1914) derived for catchments in USA is a typical one of this kind and is given by

$$Q_{TP} = C_f A^{0.8} (1 + 0.8 \log T) \quad (2.25)$$

where Q_{TP} = maximum 24-h flood with a frequency of T years in m^3/s ,
 A = catchment area in km^2 ,
 C_f = a constant with values between 0.18 to 1.88.

2. Unit Hydrograph Technique (UH),

An Unit Hydrograph is the hydrograph of direct runoff resulting from unit depth of 1 cm of rainfall excess generated uniformly over the basin for a specified duration (D -hours). The term unit depth of rainfall excess means

excess rainfall over and above all losses in the basin under consideration. The duration is the period of rainfall excess which is assumed to be uniformly distributed over the basin area. The specified duration is important as the shape and the peak of unit hydrograph of a basin depends on it. Thus for period of 3-h rainfall excess over the basin, the unit hydrograph is named as 3-h unit hydrograph. Sketch of an UH is shown in Figure 2.4. A watershed can have as many unit-hydrographs depending on the number of corresponding periods of rainfall excess. An unit hydrographs of 2, 6 or 12 h indicates that it is the duration of rainfall excess giving rise to the unit hydrograph. It never means the duration of the occurrence of unit hydrograph.

The concept of unit hydrograph was originally put forth by Sherman in 1932. The present definition is more refined and an universally accepted one based on the following principles and assumptions.

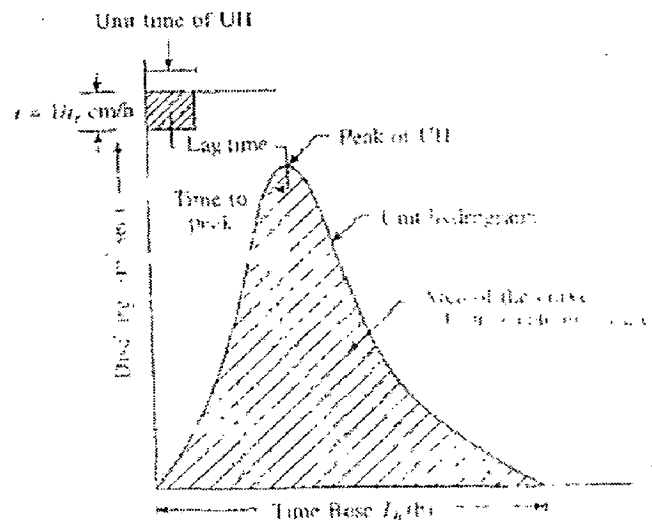


Fig. 2.4. Sketch of an Unit Hydrograph
Assumptions and Conditions in Unit Hydrograph

1. *The principle of time invariance* applies to the unit hydrograph, which means that a given rainfall excess will produce the same direct runoff hydrograph whatever may be the season of the year.
2. *The principle of linearity* is that when a D-hour rainfall excess of 1 cm depth over the basin produces a direct runoff hydrograph of 1 cm, then x cm depth of rainfall excess of the same duration (D-hour) over the same basin will produce a runoff of x cm depth. Conversely, the ordinates of the direct runoff hydrograph of D-hour duration may be brought to unit

hydrograph of D-hour by dividing the ordinates of DRH by $(x/1)$. Similarly, if x_1 cm of rainfall excess produces y_1 cm of runoff and x_2 cm produces y_2 cm, then $(x_1 + x_2)$ cm of rainfall excess will produce $(y_1 + y_2)$ cm of DRH.

3. A storm of duration of rainfall excess t_r hours will always produce a surface runoff hydrograph of base T_b -hours regardless the intensity of rainfall in the period t_r . Time base T_b of runoff is sum of t_c and t_r where t_c is the time of concentration of the watershed upto the desired outlet.
4. An unit hydrograph is a lumped response of the catchment at the basin outlet.
5. The area of DRH due to unit hydrograph is the area of the basin multiplied by 1 cm.
6. When the duration of rainfall excess is D-h, the excess rainfall intensity is $1/D$ -cm/h. This is because the total rainfall producing a UH is 1 cm. For 1 h duration of rainfall excess, the intensity of rainfall excess is 1 cm/h.
7. Since the consideration for UH is the rainfall excess, the antecedent or subsequent storm conditions have no action to present derivation of UH.

Limitations of Unit Hydrograph

1. The maximum catchment limitation for derivation or application of unit hydrograph theory is upto 5000 km^2 . For catchment exceeding this area, the basic assumption of uniform rainfall distribution over the basin due to the storm may be violated. However, the limitation on size of catchment area should also take into account the orographic features of the basin.
2. The application of UH is not suitable for very long basins.
3. The lower limit on the size of a basin to which the UH concept apply should preferably be more than 2 km^2 .
4. When a large portion of basin is covered with snow, the UH principle should not be applied.
5. The duration of rainfall excess should preferably be $1/3$ to $1/5$ of basin lag.

6. UH should not be derived from a catchment where large storage exists.
7. If there is high variation in the rainfall intensity over the basin then for such storms UH should not be derived.

Fluctuations of intensities of rainfall in D-hour is taken care of by the catchment characteristics producing runoff. Total affect of variation of intensity on unit hydrograph is usually neglected. However it is not possible to quantify this limitation of variation in UH.

Uses of Unit Hydrograph

The important purposes for which a unit hydrograph can be used are :

1. *Computation of flood hydrograph for design of structure* : When the Probable Maximum Precipitation (PMP) or the Standard Project Storm (SPS) for a basin is known, the UH is convoluted over the excess rainfall of the histogram blocks of PMP or SPS to obtain the design flood at the project site.
2. *Extension of the flow records at a site* : The method is more or less the same as (1). All storm precipitation depths, i.e. rainfall excess for the entire period under consideration are multiplied successively by UH ordinates and added up to compute the runoff volumes. The work is enormous and cannot be handled manually. Use of a digital computing machine is necessary to calculate the runoff values for each storm.
3. *Flood forecasting models* : Unit Hydrograph developed for a basin are stored in a computer. Knowing the excess rainfall depths from telemeter gauges, flood can be forecasted for the basin by convoluting the UH over the excess storm rainfall and carrying out the channel routing if necessary.
4. *Comparing the catchment characteristics* : Two unit hydrographs of the same unit durations derived from two adjoining basins can be used to compare the hydro-meteorological characteristics of the basins.

3. Flood Frequency Analysis

Flood frequency analysis considers the annual peak flows at a site for all the years. The method of analysis and predicting flood from the data runoff

peaks is called flood frequency analysis. It gives only the magnitude of flood peak of desired recurrence interval or return period, but doesn't provide information about the complete hydrograph or the flood volume. Prediction of flood peaks from the flood data of recorded maximum series are reliable when is carried out for return periods of less than the data length. However, when data is to be extrapolated, for example when flood peaks of 1000 or 10000 years are required to be predicted from an annual maximum services of say 30-40 years, then the prediction should be carried out with caution as the sample data may not be true representative of the population. There may be long term trend or a cycle associated with the system. For such predictions, confidence bands or limits are to be estimated at 95% or other acceptable percentages, depending on the precision requirement.

Frequency analysis for some important theoretical distributions are given below.

Gumbel's Distribution Method

This extreme value distribution was introduced by Gumbel (1941) and is commonly known as Gumbel's distribution. Gumbel probability distribution is widely used for extreme value analysis of hydrologic and meteorological data like floods, maximum rainfalls, maximum wind speed and other events. Gumbel defined a flood as the largest of the 365 daily flows and the annual series of flood flows constitute a series of largest values of flows. According to his theory of extreme events, the probability of occurrence of an event equal to or larger than a value x_0 is

$$P(X \geq x_0) = 1 - e^{-e^{-y}} \quad (2.26)$$

In which y is a dimensionless variable given by

$$y = \alpha (x - a)$$

$$a = \bar{x} - 0.45005 \sigma_x$$

$$\alpha = 1.2825 / \sigma_x$$

Thus

$$y = \frac{1.2825(x - \bar{x})}{\sigma_x} + 0.577 \quad (2.27)$$

where \bar{x} = mean and σ_x = standard deviation of the variate X. In practice it is the value of X for a given P that is required and as such Eq. (2.26) is transposed as

$$y_p = - \ln [- \ln (1 - P)] \quad (2.28)$$

Noting that the return period $T = 1/P$ and designating y_T = the value of y, commonly called the reduced variate, for given T

$$y_T = - [\ln \cdot \ln (T/(T-1))] \quad (2.29)$$

$$y_T = - [0.834 + 2.303 \log \log (T/(T-1))] \quad (2.29a)$$

Now rearranging Eq. (2.27), the value of the variate X with a return period T is

$$x_T = \bar{x} + K \sigma_x \quad (2.30)$$

where

$$K = \frac{(y_T - 0.577)}{1.2825} \quad (2.31)$$

Further, Eqs. (2.30) and (2.31) constitute the basic Gumbel's equations and are applicable to an infinite sample size (i.e. $N \rightarrow \infty$).

Since practical annual data series of extreme events such as floods, maximum rainfall depths, etc, all have finite lengths of record, Eq.(2.31) is modified to account for finite N as given below for practical use.

Gumbel's equations for practical use.

Equation (2.30) giving the value of the variate X with a recurrence interval T is used as

$$x_T = \bar{x} + K \sigma_{n-1} \quad (2.32)$$

where σ_{n-1} = standard deviation of the sample size of size N

$$= \sqrt{\frac{\sum (x - \bar{x})^2}{N - 1}}$$

$$K = \text{frequency factor expressed as } = \frac{y_T - \bar{y}_n}{S_n} \quad (2.33)$$

in which y_T = reduced variate, a function of T and is given by

$$y_T = - \left[\ln \cdot \ln \frac{T}{T-1} \right] \quad (2.34)$$

or

$$y_T = - \left[0.834 + 2.303 \log \log \frac{T}{T-1} \right]$$

\bar{y}_n = reduced mean, a function of sample size N and is given in Table 2.1 ; for $N \rightarrow \infty$, $\bar{y}_n \rightarrow 0.577$

S_n = reduced standard deviation, a function of sample size N and is given in Table 2.2 ; for $N \rightarrow \infty$, $S_n \rightarrow 1.2825$

These equations are used under the following procedure to estimate the flood magnitude corresponding to a given return based on an annual flood series.

1. Assemble the discharge data and note the sample size N . Here the annual flood value is the variate X . Find \bar{x} and σ_{n-1} for the given data.
2. Using Tables 1C and 2C of Appendix-C to determine \bar{y}_n and S_n appropriate to given N .
3. Find y_T for a given T by Eq. (2.34).
4. Find K by Eq. (2.33).
5. Determine the required x_T by Eq. (2.32).

Gumbel Probability Paper

The Gumbel probability paper is an aid for convenient graphical representation of Gumbel's distribution. It consists of an abscissa specially marked for various convenient values of the return period T . To construct the T scale on the abscissa, first construct an arithmetic scale of y_T values, say from -2 to +7. For selected values of T , say 2, 10, 50, 100, 500 and 1000, find the values of y_T by Eq.(2.34) and mark off those positions on the abscissa. The T -scale is now ready for use.

The ordinate of a Gumbel paper on which the value of the variate, x_T (flood discharge, maximum rainfall depth, etc.) are plotted may have either an arithmetic scale or logarithmic scale. Since by Eqs (2.29) and (2.30) x_T varies linearly with y_T , a Gumbel distribution will plot as a straight line on a Gumbel probability paper. This property can be used advantageously for graphical extrapolation, wherever necessary.

To verify whether the given data follow the assumed Gumbel's distribution, the following procedure may be adopted. The value of x_T for some return periods $T < N$ are calculated by using Gumbel's formula and plotted as x_T vs T on a convenient paper such as a semi-log, log-log or Gumbel probability paper. The use of Gumbel probability paper results in a straight line for x_T vs T plot. Gumbel's distribution has the property which gives $T = 2.33$ years for the average of the annual series when N is very large. Thus the value of a flood with $T = 2.33$ years is called the *mean annual flood*. In

graphical plots this gives a mandatory point through which the line showing variation of x_T with T must pass. For the given data, values of return periods (plotting positions) for various recorded values, x of the variate are obtained by the relation $T_p = (N + 1)/m$ and plotted on the graph described above.

A good fit of observed data with the theoretical variation line indicates the applicability of Gumbel's distribution to the given data series. By extrapolation of the straight line x_T vs T , values of x_T for $T > N$ can be determined easily.

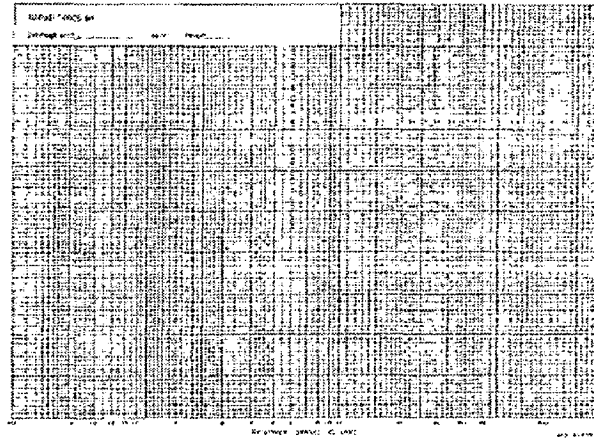


Fig. 2.5. Gumbel's Probability Paper

Log-Pearson Type III Distribution

This distribution is extensively used in USA for projects sponsored by the US Government. In this the variate is first transformed into logarithmic form (base 10) and the transformed data is then analysed. If X is the variate of random hydrologic series, then the series of Z variates where

$$z = \log x \quad (2.35)$$

are first obtained. For this z series, for any recurrence interval T , gives

$$z_T = \bar{z} + K_z \sigma_z \quad (2.36)$$

where K_z = a frequency factor which is a function of recurrence interval T and the coefficient of skew C_s ,

σ_z = standard deviation of the Z variate sample

$$= \sqrt{\frac{\sum (z - \bar{z})^2}{(N - 1)}} \quad (2.36 a)$$

and

C_s = coefficient of skew of variate Z

$$= \frac{N \sum (z - \bar{z})^3}{(N-1)(N-2)(\sigma z)^3} \quad (2.36 \text{ b})$$

\bar{z} = mean of the z values

N = sample size = number of years of record

The variations of $K_z = f(C_s, T)$ is given in Table 3C of Appendix-C.

After finding z_T by Eq. (2.36), the corresponding value of x_T is obtained by Eq. (2.35) as

$$x_T = \text{antilog}(z_T) \quad (2.37)$$

Sometimes, the coefficient of skew C_s , is adjusted to account for the size of the sample by using the following relation proposed by Hazen (1930)

$$\hat{C}_s = C_s \left(\frac{1+8.5}{N} \right) \quad (2.38)$$

where \hat{C}_s = adjusted coefficient of skew. However the standard procedure for use of log-Pearson Type III distribution adopted by US Water Resources Council does not include this adjustment for skew.

When the skew is zero, i.e. $C_s = 0$, the log-Pearson Type III distribution reduces to *log normal distribution*. The log-normal distribution plots as a straight line on logarithmic probability paper.

2.7. SEDIMENT SAMPLING & ANALYSIS

2.7.1. Sediment Sampling

Sediment is fragmented material that originates from chemical or physical disintegration of rocks. Sediment sampling involves collection of known volume of water-sediment mixture from the stream which is further analysed to determine the quantity of sediment yield. In case bed load is not measured, 2.5 to 15 % of the suspended load is generally added depending upon bed material to arrive at the total sediment yield. Sediment sampling can be taken through :

- (a) depth integration sediment sampling method
- (b) point integration sediment sampling method.

a. Depth Integration Sediment Sampling

Depth integrated samples are taken with a sampler that has an intake which points directly into the current. The sample is collected as it traverse the depth of the stream at a uniform speed. For stream less than 5 metre deep, the sampler is lowered to the bottom of the stream at a uniform rate and raised back to the surface at a uniform rate, but not necessarily the same rate. Streams from 5 to 10 metre deep are usually integrated in one direction only. Deeper streams are integrated in more than one sampling trip.

This method is advantageous to the fact that a single sample provides a discharge weighted concentration or a complete sampling vertical or for major fraction of a sampling vertical in very deep streams. However, it leaves an unsampled zone at the bottom which may required separate computation of unmeasured suspended sediment.

b. Point Integration Sediment Sampling

This method involves lowering the sampler to the sampling point, opening the valve for the desired sampling time, then closing the valve at the end of sampling period. It is useful to determine sediment distribution.

2.7.2. Analysis of Sediment Sample

The sediment samples collected from a stream/river are analysed by two methods namely : (a) Gravimetric method and ; (b) Hydrometric method. Hydrometric method needs trained staff and is mostly adopted where laboratory has been set up and equipped for analysis.

Based on soil particle size the following standards are adopted for classifying coarse, medium and fine sediment particles :

- i) Coarse sediment ----- Particles above 0.2 mm in diameter.
- ii) Medium sediment ----- Particles between 0.075 to 0.20 mm.
- iii) Fine sediment ----- Particles below 0.075 mm.

In addition, there are various kinds of salts dissolved in water along with organic matters. In case of low sediment concentration, the estimation of suspended sediment, particularly the fine sediment is difficult by hidrometric

method and may give inaccurate results, the while same can be estimated easily by adopting gravimetric method of sediment analysis.

2.8. CONCLUSION

In Water Resources Structures Projects Planning, we need the following information :

- (i) Rainfall data of the catchment,
- (ii) Catchment characteristics and the land use pattern,
- (iii) Daily stream flows for determining the storage capacity of a reservoir,
- (iv) Flood of certain frequency,
- (vi) Intake rate of sedimentation to estimate useful life of the reservoir.

For determination of the average precipitation over an area, a large number of rain gauges are installed, and to convert the point rainfall values at various stations into an average value over a catchment (Mean Areal Precipitation), the following three methods can be used :

- (i) Arithmetic Mean Method
- (ii) Thiessen Polygon Method
- (iii) Isohyetal Map Method

Thiessen Polygon Method is the most popular applied to most of the field problems.

Regression Analysis technique is a procedure for fitting an equation to a set of data. This techniques are used to identify the mathematical dependence between the observed values of physically related variables and thus can account for the additional information contained in the correlated sequences of events. This procedure is certainly better than using relatively short historical sequences in hydrological analyses.

Computation of accurate runoff rate or volume due to any storm from the catchment is a difficult task, because it depends on several factors related to

the catchment and atmosphere, prediction of whom is not easy. However, the commonly used methods are given below :

- (i) Rational Method
- (ii) SCS – Curve Number Method
- (iii) Hydrograph Method

A Flow Duration Curve can be drawn if a continuous record of daily flow (or average weekly or average monthly flow) is available for a long period. The flow duration curve represents the data in a condensed form and is extremely useful for the estimation of the available power.

The Design Flood is considered as inflow at the upstream of the structure which the structure should pass safely. To estimate the magnitude of a flood peak, the following alternative methods are :

- (i) Empirical Method
- (ii) Unit Hydrograph Technique
- (iii) Flood Frequency Analysis

Sediment sampling can be taken through :

- (a) - depth integration sediment sampling method
- (b) point integration sediment sampling method.

The sediment samples collected from a stream/river are analysed by two methods namely :

- (a) Gravimetric method and ;
- (b) Hydrometric method.



CHAPTER 3

STUDY AREA AND DATA AVAILABILITY

3.1. BACKGROUND

Jamrani Dam Project is a Multi-purpose Water Resources Project to be executed in Kumaon region in the state of Uttarakhand, India (Fig. 3.1). Jamrani Dam Project consists of a diversion system (Gola Barrage) in its first phase on Gola River at Kathgodam in district Nainital, Uttarakhand and 130.6 metres high Roller Compacted Concrete Dam in second phase across river Gola about 10 kilometres upstream from Kathgodam near Jamrani village. It was sanctioned by Central Water Commission (C.W.C), Government of India in 1975. The first phase of the project was completed in 1982 and side by side investigations were continued for the main dam (the second phase).

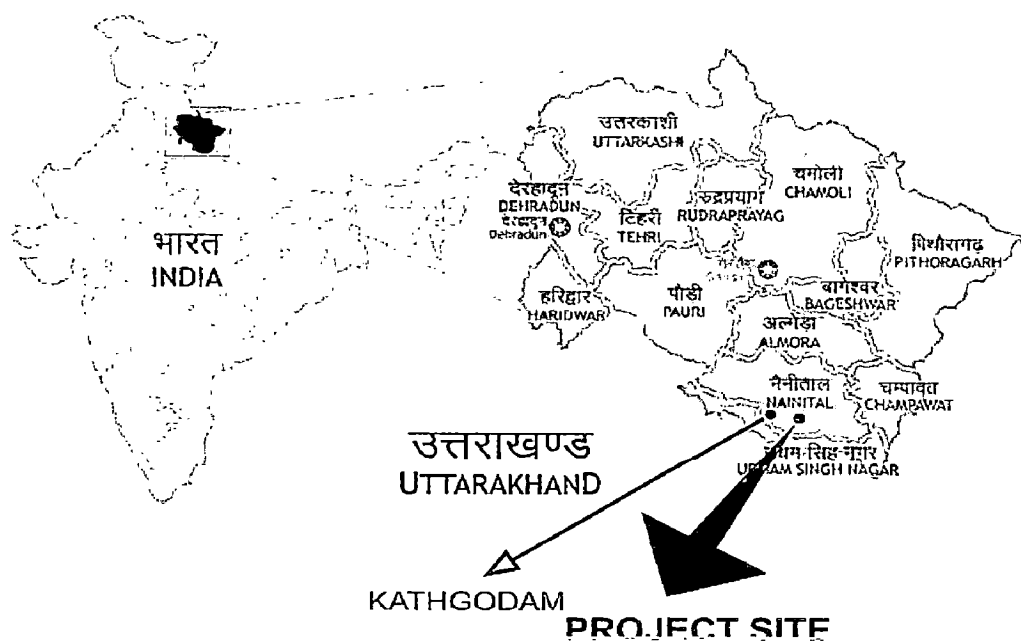


Fig. 3.1 Index Map of Proposed Jamrani Dam Site.

Gola river originates and flows through south eastern Kumaon Himalaya in the State of Uttarakhand. It is fed largely by the runoff during monsoon season, when its discharge is high but progressively gets reduced in winter and summer season.

Haldwani town is the gateway of Kumaon hills and a number of industrial units like sugar, cotton, soya oil, yarn, dairy, paper, electric goods and watches have been established around the town and other commercial units are proposed to be set up soon.

The growth and improvement has lead to an acute water shortage in the area of Tarai and Bhabar. Tarai and Bhabar areas mainly constitute of fertile agricultural land for which there is an acute shortage of water. At present there is no dependable source of water to meet the drinking water of the Haldwani town and nearby villages and as well as the need of irrigation water for agriculture. Though number of tubewells have been constructed and are in construction to meet this requirement but due to huge extraction of water, the water table has gone down considerably. Most of the surface water sources are either dried up or the discharge of them has been reduced which has affected the assured supply of water. On the other hand, a large quantity of water flows away during Monsoon period in Gola river.

3.2. THE PROJECT

Jamrani project consist of constructing a 130.6 metres high roller compacted concrete dam on river Gola near Jamrani village, about 10 kilometres upstream of Kathgodam in district Nainital, Uttarakhand. The latitude and longitude of the dam site are 29^o16'15" N and 79^o37' E respectively.

This dam will creates a storage reservoir of 208.6 MCM capacity, out of this 144.3 MCM as live storage and 64.3 MCM as the dead storage. Water for irrigation is proposed to be released through an outlet provided in the body of the dam. The water stored in the reservoir is to be utilised in the lean period for irrigation. This is to be picked up at Gola Barrage, already completed against this project as phase one, and diverted to two main feeders from left and right banks of the barrage. This project also provides for constructing of about 36 kms of feeder channels to augment the supplies in the existing channels of Tarai and Bhabar area, which are mostly completed except two feeders.

Salient Features of the Project :

1.	Type of Dam	Roller compacted concrete gravity dam
2.	Height above river bed	130.6 metres
3.	Top level	765.6 metres
4.	Type of spillway	Ogee type
5.	Waterway	4 bays of 13.25 metre each
6.	Width of piers	3.75 metres
7.	Crest level	749.00 metres
8.	Gates	Radial of size 13.25 x 13.5 metres
9.	Catchment area at dam site	450 km ²
10.	Catchment area at Gola Barrage site	600 km ²
11.	Normal rainfall in the catchment	1540 mm
12.	Reservoir capacity at Full Reservoir Level (762.0 M)	208.6 MCM
13.	Reservoir capacity at dead reservoir Level (716.63 M)	64.3 MCM
14.	Total live storage	144.3 MCM
15.	Water spread at full reservoir level	450 Hectares
16.	River bed slope	14 m/km
17.	River bed level at Dam site	635.00 metres
18.	Maximum tail water level	643.00 metres
19.	Gola Barrage (completed 1982)	
	a) Length	81.00 metres
	b) Crest level of water way	506.50 metres
	c) Crest level of sluice bay	506.50 metres
	d) D/S floor level	502.00 metres
	e) Design discharge	3250 cumecs
	f) Pond level	510.75 metres

(source : 1989 - Report on Hydrology)

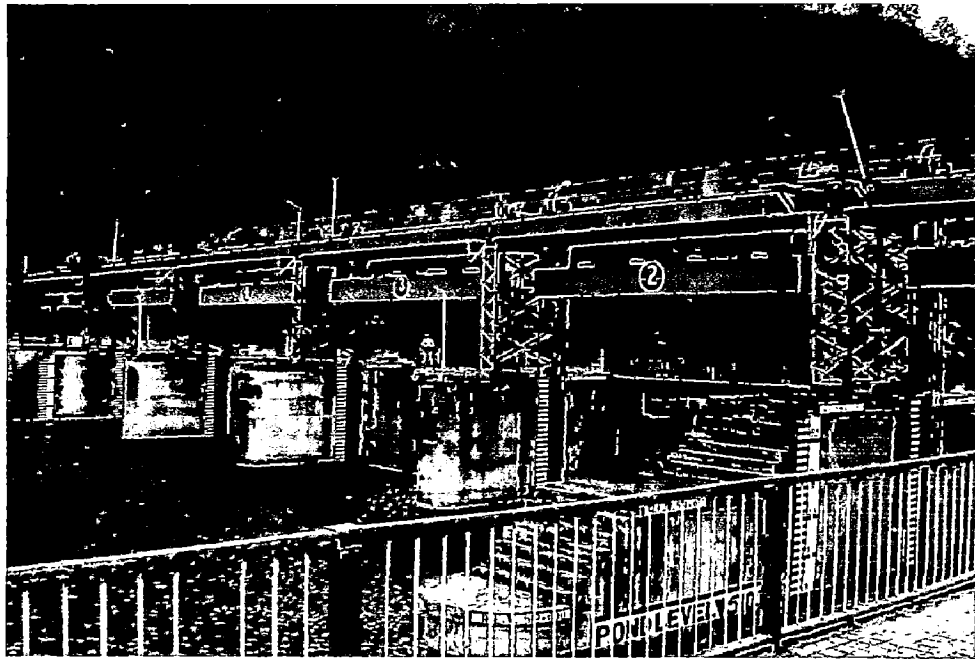


Fig. 3.2 Upstream view of Gola Barrage at Kathgodam

3.3. ADVANTAGES OF THE PROJECT

This project will help in providing an additional irrigation of 12993 Hect. in Uttarakhand and 47607 Hect. in Uttar Pradesh and the irrigation potential will increase from 52.4% to 92.7%. Construction of this Dam will help in solving the main problem of drinking water in Haldwani and Bhabar area, also ground water table in the Bhabar area will increase considerably causing the increase in continuous discharge of tubewell.

At present the flood of Gola river erodes the valuable agricultural land, damages other valuable property including human life. Construction of Dam will check the erosion, damages of property and meandering of river. Thus by constructing Dam the losses in crores will be saved. Construction of Dam will help in providing the facilities to rowing, yatching, boating and will develop the area for tourism, which also help in providing employment to local persons and will ultimately be helpful in changing the living standard and style of the peoples. The proposed generation of 30 MW electricity is being taken as indirect profit as electricity generation though will depend solely on the availability of water in the reservoir.

After the construction of the Dam, this entire area will have sufficient water for irrigation, drinking purpose. Thus the construction of Dam is the only solution to fulfill the developing needs of people and area.



Fig. 3.3 Site for the proposed Jamrani Dam, located 10 km upstream of Gola Barrage

3.4. CLIMATE

The region has a dry season from October till May and a wet season from June to September when approximately 90% of the average rainfall of 1542 mm is received. May and June have very high temperatures (average maximum temperature between 35⁰ to 40⁰C and average minimum temperature varying between 16⁰ to 25⁰ C), whereas December and January are cooler months average maximum temperature between 17⁰ to 24⁰C and average minimum temperature varying between 5⁰ to 10⁰ C). The relative humidity at Pantnagar varies from 75 to 80 in July to September, whereas it is lowest (40 to 50) in April. At Dam site, it varies between 62 to 66 in July to September and 32 to 40 in March. The maximum wind velocity at dam site is observed as 80 km/hr in April and 15 km/hour in January (1989 report). The pan evaporation at Pantnagar (from data of 2002-2006) is minimum in December-January (1 to 2 mm/day) and maximum in April-May (8-9

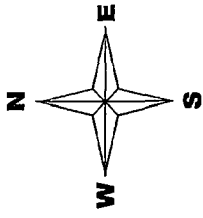
mm/day). Due to development activities around Nainital and the adjoining areas, there is an overall ecological change and, the area is now considered to be good and prosperous from all angles.

3.5. HYDROLOGICAL AND METEOROLOGICAL DATA

3.5.1. Catchment Area

The catchment area at the proposed Jamrani dam site is 450 km², drained by the Gola River and its tributary called Kalsa river. The catchment area is hilly and valley is narrow with sides having steep slopes. A number of nalas drain off the basin into the river. The hills are bare with cultivation on the terraces here and there. There are no storage tanks or lakes in the catchment. The shape of the catchment is somewhat 'D' shaped. The compactness factor of the catchment (perimeter of catchment/circumference of a circle having area equal to catchment) is 1.27. The form factor of the catchment (average width of the catchment/axial length of the catchment) is 1.3. The catchment area plan is shown in Fig.3.4.

River Gola faces problems of flash flood during rainy season. The river flowing through mountaineous terrain, descends into plains at Kathgodam. Extensive flood plain deposits comprising of sand and boulders are present downstream of Kathgodam. Gola river originates and flows through southeastern Kumaun Himalaya in district Nainital. It is not a snowfed river. The river is fed largely by the runoff during monsoon season when its discharge is high but progressively gets reduced in the dry season. The flow is not sufficient to meet even the existing demand of drinking water. The river bed slope varies from 14 m/km at the dam site to 23 m/km in the upper reaches and that of its tributary Kalsa from 13 m/km to 23 m/km. The longitudinal section of river Gola is shown in Fig 3.5.



LEGEND

--- ---	Catchment Boundary
—	River and Stream
—	Road
- - - -	Railway Line
●	Rain gauge Stations Symon Type
▲	Rain gauge Automatic Type
◐	Lake

CATCHMENT AREA AT	
1	JAMRANI DAM450 Km ²
2	GOLA HEAD WORKS600 Km ²

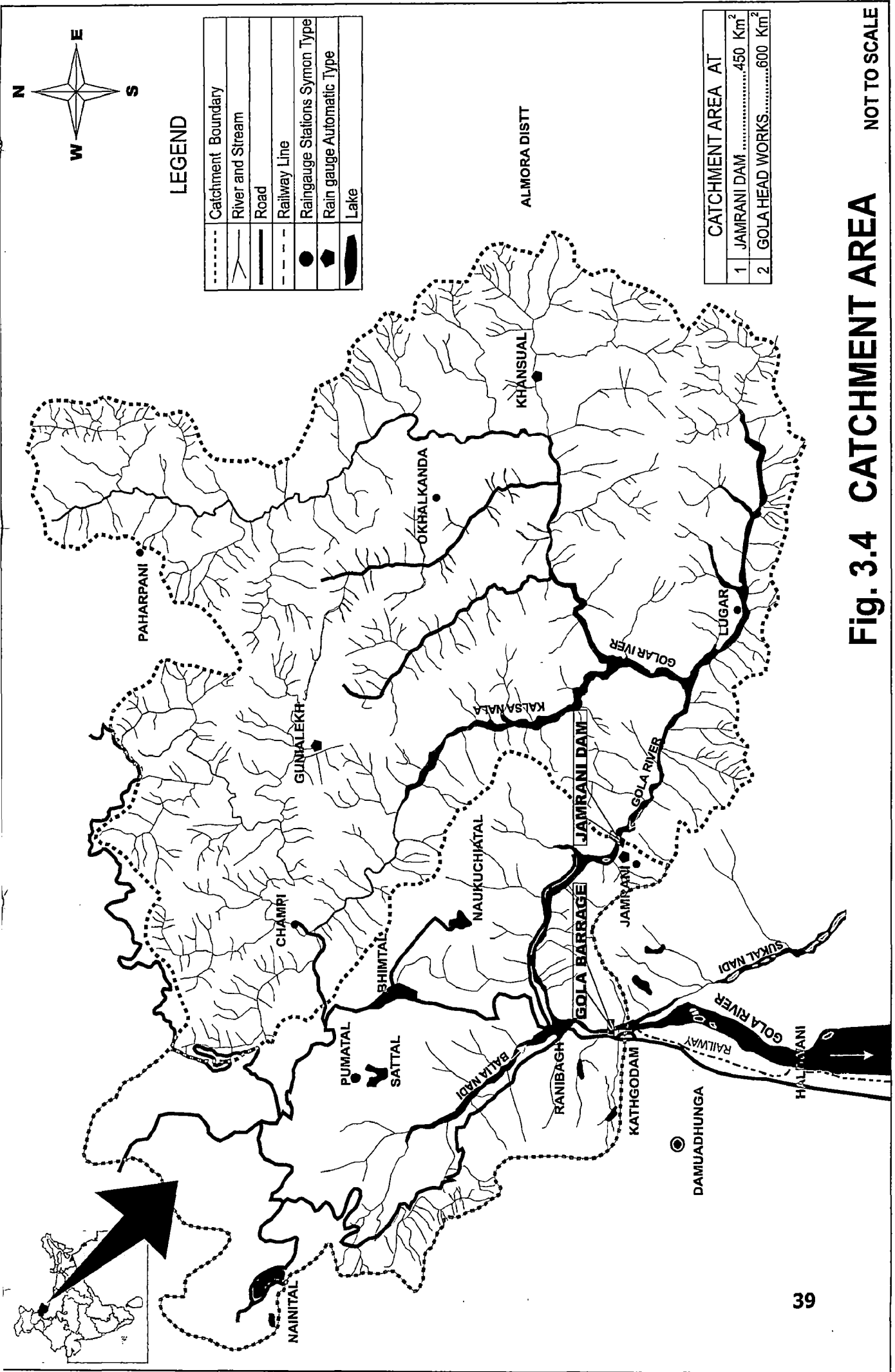


Fig. 3.4 CATCHMENT AREA

NOT TO SCALE

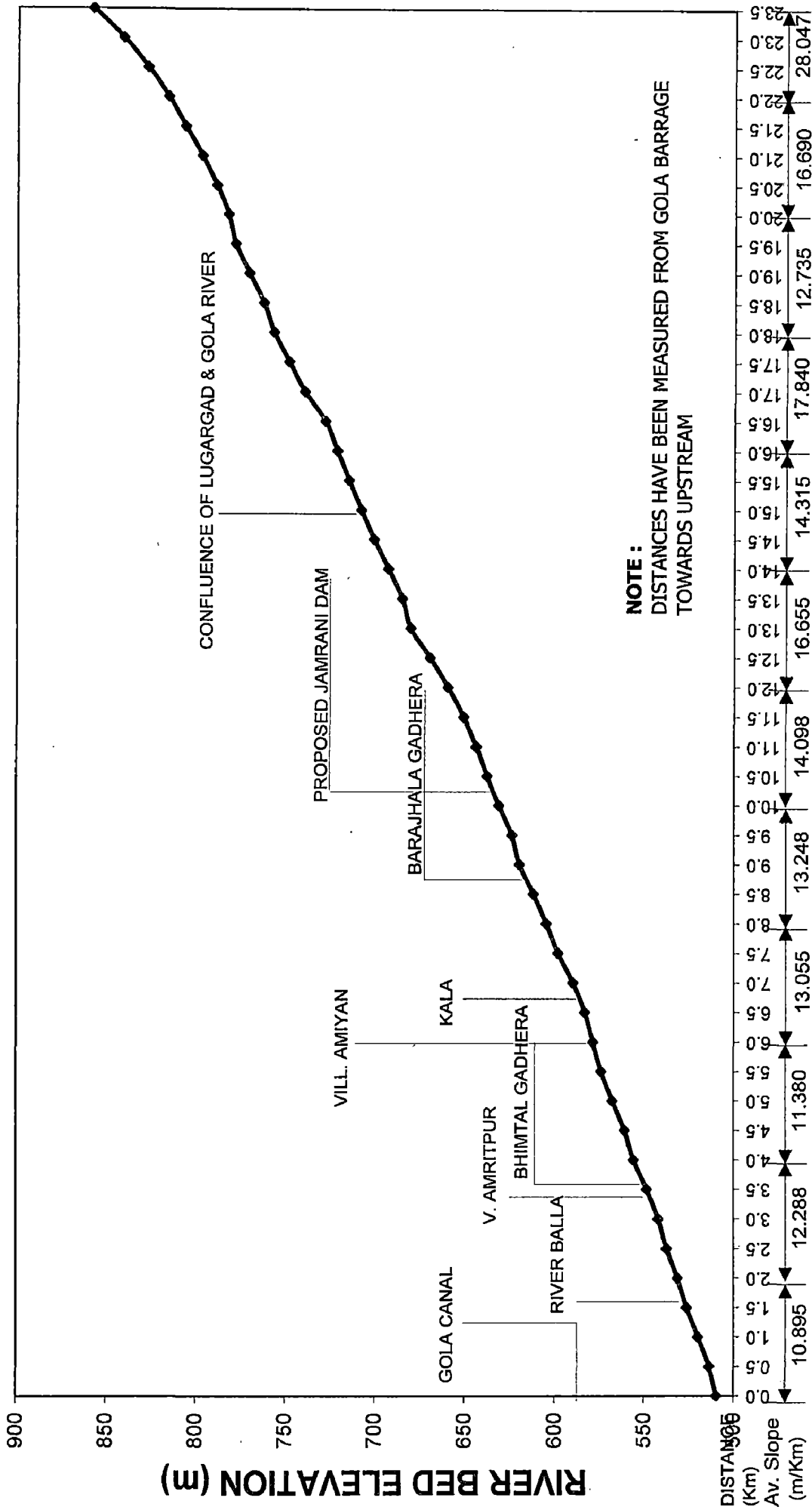


Fig-3.5 L - SECTION OF GOLA RIVER 0.00 to 23.5 Km

3.5.2. Precipitation data

In the year 1979 some Symon type rain gauges were installed in catchment area at Jamrani Dam site, Lugar, Okhalkanda, Paharpani and Champi. In the year 1984, three self recording rain gauges or pluviograph stations were established at dam site, Gunialekh and Khansu. However, the simultaneous short interval rainfall data for the flood events is only available at dam site. Some (4 years) of all ten-daily observed rainfall data (27 years) within catchment are given in Table-1.A, Table-2.A, Table-3.A & Table-4.A of Appendix-A.

3.5.3. Stream flow data at Dam site

Automatic river gauge recorder was established at dam site in 1984. It has been reported that the Gauge and discharge measurements were taken three to four times during the day and average discharge measurement were recorded as the daily discharge till 1993. The discharge site washed away during flood in 1993. After 1993, the discharge at dam site is measured in a very crude way by float method. During the visit to the dam site, it was informed that the discharge is taken by throwing a wooden piece in the river an at a fix section and then by measuring the time of reaching the same at some downstream section (about 100 m downstream). Further, the time of travel is related to average velocity and by multiplying with the average cross-sectional area, the discharge is measured. This discharge is further converted to 10-daily discharge data and is provided as the stream flow data at Dam site. Ten daily discharges observed at dam site since March 1977 till June 2006 are tabulated in Table-5A of Appendix-A.

3.5.4. Stream flow data at Gola weir

Long term gauge data of river Gola are available only at Gola Weir (now barrage) where it is being observed at 8 A.M. daily since 1948 till date. For the computation of discharge at Gola weir, the existing weir has been considered as broad crested weir with a vertical fall of about 2.5 to 3 m downstream. The observed 10 daily discharges since July 1948 till June 2006 are tabulated in Table-6.A of Appendix-A.

3.5.5. Temperature, humidity, wind velocity and Pan Evaporation data

Since temperature affects evaporation and snowmelt, it is needed in many water resources study. It is a measure of the ability of the atmosphere and water to receive and transfer heat. Temperature varies primarily with the magnitude of solar radiation and follows diurnal and seasonal cycles. Relative Humidity is the ratio (in %) of mixing ratio to the saturation mixing ratio has a significant influence on evapotranspiration. Relative Humidity does not vary drastically over a short time. Wind velocity is inputs in calculation of evapotranspiration, controlled by local pressure anomalies which in turn is influenced by the temperature and local topographic features. Evaporation is an important component of the hydrologic cycle. Pan evaporation provides an estimate of open water evaporation.

The apparatus of pan evaporation was fixed in 1975 at dam site and observations recorded are shown in Table-7.A of Appendix-A.

The data of maximum and minimum temperature, humidity and wind velocity is being observed since 1981 are also available.



CHAPTER 4

MEAN AREAL PRECIPITATION AND ANNUAL FLOW ESTIMATION FOR JAMRANI PROJECT

4.1. MEAN AREAL PRECIPITATION ESTIMATION

In this study, the Thiessen Polygon Method is adopted to estimate Mean Areal Precipitation (MAP) over a catchment area for Jamrani Dam site. In the Thiessen Polygon Method, the rainfall recorded at each rain gauge stations is given a weightage on the basis of the area which it represents. This method is better than the Arithmetic Mean Method which gives equal weightage to all the stations. This method is popularly applied to most of the field problems.

In the catchment area, since year 1979 some Symon type rain gauges were installed at Jamrani Dam site, Lugar, Okhalkanda, Paharpani and Champi. After that, in the year 1987, three self recording rain gauges or pluviograph stations were established at Jamrani Dam site and two other locations viz. at Gunialekh and Khansu. According to above information on Catchment Area of 450 Km² for Jamrani Dam site there were five rain gauge station since year 1979 until year 1987, and there are seven rain gauge stations since year 1987 till date.

The following is procedure for delineating area for each station in the catchment area based on seven rain gauge stations basis.

- (a). All the gauges in and around the catchment area are accurately marked on a map drawn to scale.
- (b). Consecutive stations are joined by dotted straight lines, forming triangles.
- (c). Perpendicular bisectors are drawn to these dotted lines such that the bisectors form a polygon around each station.
- (d). Each station on the map is thus enclosed by a polygon. A polygon represents an area for which the station rainfall is the representative.

(e). Area of each polygon is measured by a planimeter. Sum of the areas of all the polygons must be equal to total area of the catchment area. Areas of each stations are indicated on Fig. 4.1.

(f). Thiessen weights are computed by dividing the area of each polygon by the total area of the basin. Thus, sum of Thiessen weights for all stations should be equal to unity. If there are seven stations in and around the catchment area then seven thiessen polygons are drawn. Sum all the seven thiessen weights must be equal to unity.

(g). The average precipitation is computed from the relation

$$P_{av} = \frac{A_1P_1 + A_2P_2 + A_3P_3 + \dots + A_nP_n}{A_1 + A_2 + A_3 + \dots + A_n}$$

or
$$P_{av} = P_1W_1 + P_2W_2 + P_3W_3 + \dots + P_nW_n$$

in which P_1, P_2, \dots, P_n represents precipitation at stations 1, 2, 3, ..., n, and A_1, A_2, \dots, A_n , represents the area of polygons representing the corresponding stations, A is the total area of basin which is the sum of all the polygons and W_1, W_2, \dots, W_n are Thiessen weights computed as $W_1 = A_1 / A, W_2 = A_2 / A, \dots, W_n = A_n / A$ such that $W_1 + W_2 + \dots + W_n = 1.00$.

These calculations are indicated in examples on Table-1A, 2A, 3A & 4A of Appendix-A.

(h). Mean Annual Precipitation for each year from 1979-1980 to 2005-2006 are tabulated in Table-4.1 and Fig. 4.2, is found the average Precipitation $P_{av} = 1541.83$ mm and Standard Deviation $S_n = 334.36$.

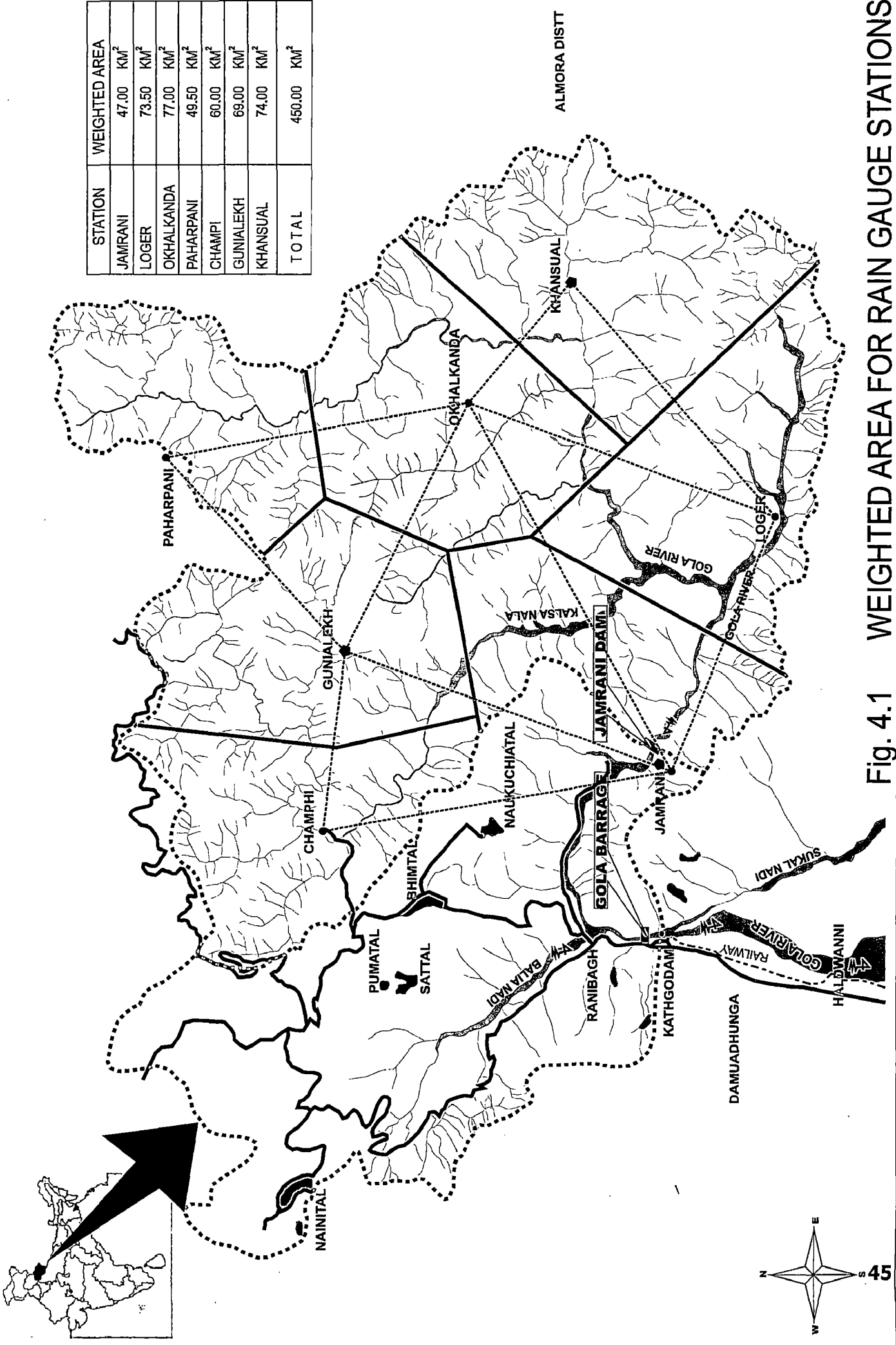


Fig. 4.1 WEIGHTED AREA FOR RAIN GAUGE STATIONS

Table-4.1**Catchment Annual Rainfall in (mm)**

No.	Year (July to June)	Weighted Annual Rainfall (mm)
1	1979 - 80	1166.60
2	1980 - 81	1759.90
3	1981 - 82	1204.15
4	1982 - 83	1422.96
5	1983 - 84	1558.99
6	1984 - 85	1024.55
7	1985 - 86	1920.20
8	1986 - 87	1283.48
9	1987 - 88	1149.21
10	1988 - 89	1458.02
11	1989 - 90	1461.26
12	1990 - 91	1912.27
13	1991 - 92	1170.72
14	1992 - 93	1553.29
15	1993 - 94	2191.90
16	1994 - 95	1292.60
17	1995 - 96	1955.32
18	1996 - 97	1597.21
19	1997 - 98	1768.68
20	1998 - 99	2078.55
21	1999 - 2000	2071.17
22	2000 - 01	1828.01
23	2001 - 02	1039.78
24	2002 - 03	1471.67
25	2003 - 04	1527.25
26	2004 - 05	1389.92
27	2005 - 06	1371.65

$$\bar{P} = 1,541.83 \text{ mm}$$

$$S_n = 334.36$$

see Table-1.A - 4.A for some example from detail calculations

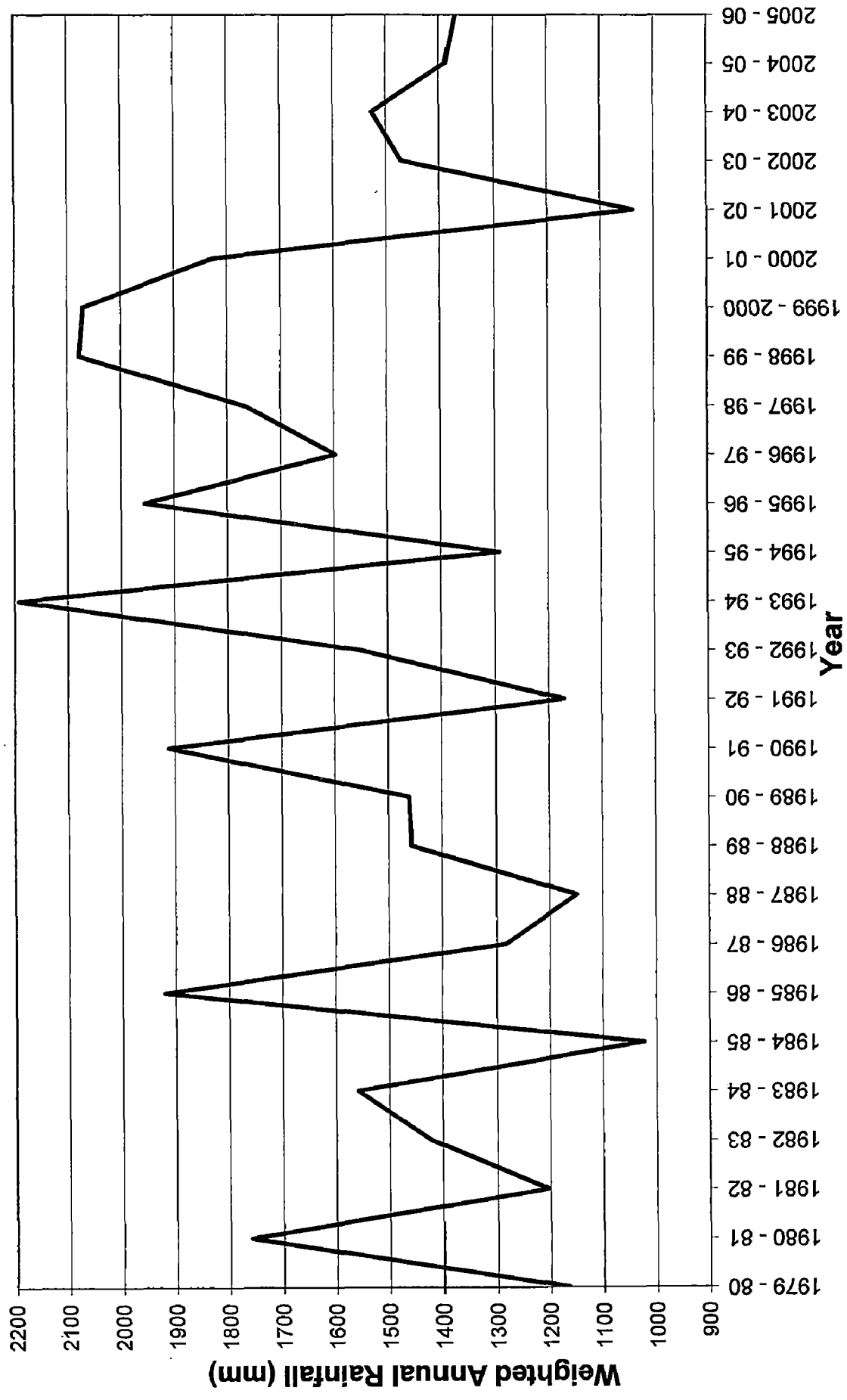


Fig. 4.2 CATCHMENT ANNUAL RAINFALL FOR JAMRANI DAM SITE

4.2. TEN-DAILY DISCHARGE CORRELATIONS

The 10-daily discharge observed at Gola Barrage during 58 years (1948 – 2006), whereas 10-daily discharge observed at Jamrani Dam site only 29 years (1977 – 2006). The aim of the study is to derive 10-daily discharge at Jamrani Dam site during 1948 until 2006 based on 10-daily discharge observed at Gola Barrage. The technique of Regression Analysis is adopted to derive that data by both Direct Linear Regression method and Log Deviation Regression Method.

The 10-daily discharge observed at both Gola Barrage and Jamrani Dam site are plotted for 36 separated periods namely for each ten-daily in a year. The water year has been considered to commence from 1st July every year to 30th June of the next year.

To develop correlation for the above 36 periods in a year, two approaches were basically used, viz :

- (i) Direct linear regression
- (ii) Log deviation method

The data for the period 1977 to 2006 are used in the analysis. For direct linear regression ten daily discharge (Q_J) at Jamrani are plotted against corresponding ten daily observed discharge (Q_G) at Gola weir for the different periods as examples shown in Fig. 1B of Appendix-B. Similarly for log deviation method, logarithmic deviations Q_{JD} and Q_{GD} about its mean of 10 daily observed discharges at the two sites are plotted as shown in Fig. 2B of Appendix-B. These figures show that the scatter in case of logarithmic deviations relation is larger than linear relation. The results of the correlation and correlation coefficient using the two approaches (Direct Linear Regression and Log Deviation Regression) are for illustration purpose, one set of computation are shown in Table-1B and 2B of Appendix-B and tabulated in Table-4.2.

The correlations used are :

$$Q_J = a Q_G + b \text{ (Direct linear regression) } \dots\dots\dots 4.1$$

$$Q_{JD} = a Q_{GD} + b \text{ (Log deviation regression) } \dots\dots\dots 4.2$$

where

Q_J is ten daily discharge in cumecs day at Jamrani Dam.

Q_G is ten daily discharge in cumecs day at Gola weir.

Q_{JD} is logarithmic deviation of ten daily discharge about its mean at Jamrani Dam.

Q_{GD} is logarithmic deviation of ten daily discharge about its mean at Gola weir.

a and b are the coefficients.

r is the coefficient of correlation.

It can be seen from the table-4.2 that relation derived using the first approach is better because the coefficient of correlation in each period are mostly higher than that of log deviation. It can also be seen that a physically meaningful relation in respect of river reach losses are obtained only using direct linear regression.

The relations obtained as above are used to derive the ten daily yield series at Jamrani Dam site from observed long term ten daily discharge at Gola weir from 1948 to 1976. From 1977 onwards observed flows at Jamrani site are included in the annual yield series. Table-4.3 and Table-4.4 show the up-to-date 10 daily, monthly and annual yield series at Jamrani Dam site using Linear and Log deviation correlation respectively.

4.3. DEPENDABLE FLOWS

Dependable flows have been estimated on annual basis by drawing flow duration curve. To draw this curve the annual yield series is arranged in descending order and ranking is done by assigning the first rank to the highest value and last rank to the lowest value. The probability of exceedance of a particular value is obtained by dividing its rank by (N + 1), where "N" is the total number of values. The use of (N + 1), ensures that the low flow can be lower than the lowest observed flow and the probability of exceedance of lowest flow in series is 100%. Table-4.5 and Table-4.6 show the annual yield

series in descending order together with rank, derived by linear and log deviation methods respectively. A flow duration curve is shown in Fig. 4.3. The 75% dependable annual flow is estimated as 355.9 MCM and 412.9 MCM by linear and log deviation method respectively.

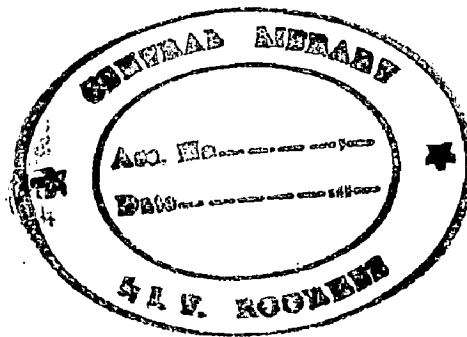


TABLE-4.2
RESULTS OF REGRESSION ANALYSIS FOR TEN-DAILY OBSERVED DISCHARGE OF GOLA RIVER AT JAMRANI DAM AND GOLA WEIR

Type of Regression	JULY			AUGUST		
	1	2	3	1	2	3
Linear Regression	a	57.04	6.95	441.69	11.09	107.74
	b	0.92	0.53	0.03	0.85	0.77
	r	0.88	0.97	0.07	0.93	0.69
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	0.82	0.90	1.08	1.07	1.28
	r	0.80	0.97	0.94	0.93	0.70
SEPTEMBER						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
OCTOBER						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
NOVEMBER						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
DECEMBER						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
JANUARY						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
FEBRUARY						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
MARCH						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
APRIL						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
MAY						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85
JUNE						
Linear Regression	a	54.49	18.81	105.40	-4.01	-10.53
	b	0.92	0.64	0.59	1.02	0.96
	r	0.74	0.98	0.41	0.95	0.91
Log Deviation Regression	a	0.00	0.00	0.00	0.00	0.00
	b	1.10	0.98	0.89	1.01	1.09
	r	0.72	0.95	0.50	0.92	0.85

Table-4.3

Derived 10 - Daily Discharge of Gola River at Jamrani Dam Site by using Linear Regression Method

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1948-1949	1	113.64	354.74	927.47	239.04	138.54	82.92	42.82	263.11	62.90	31.04	37.95	64.32	2,358.49
	2	154.01	1,251.26	563.40	459.26	124.65	61.56	38.01	34.29	33.16	36.45	43.22	29.23	2,828.50
	3	176.93	5,151.02	414.18	198.90	97.11	49.55	38.17	40.24	31.26	23.67	66.69	52.34	6,340.05
TOTAL		444.58	6,757.02	1,905.05	897.20	360.29	194.03	119.00	337.65	127.31	91.16	147.86	145.89	11,527.04
1949-1950	1	177.50	1,130.13	934.85	559.23	103.09	56.34	46.52	53.49	38.45	33.95	25.85	22.38	3,181.79
	2	941.17	1,990.00	920.92	254.73	87.52	47.28	43.11	46.72	37.06	27.14	34.40	596.39	5,026.43
	3	1,290.93	2,749.98	1,755.35	217.35	68.25	54.67	51.93	32.79	382.07	24.00	51.26	767.06	7,445.64
TOTAL		2,409.60	5,870.11	3,611.12	1,031.31	258.85	158.29	141.56	133.00	457.58	85.10	111.52	1,385.83	15,653.86
1950-1951	1	938.48	2,842.12	1,142.41	276.74	130.86	79.58	54.31	55.91	34.67	34.20	45.57	26.05	5,660.91
	2	1,396.10	3,334.70	1,184.69	166.52	97.39	64.41	55.08	39.04	35.57	29.59	52.33	25.77	6,481.18
	3	2,596.13	2,711.18	962.44	139.53	76.37	65.77	53.21	26.71	38.57	24.77	25.64	95.40	6,815.74
TOTAL		4,930.71	8,888.00	3,289.54	582.79	304.62	209.77	162.60	121.67	108.81	88.56	123.54	147.23	18,957.83
1951-1952	1	42.51	480.77	47.24	189.25	90.16	57.79	54.04	37.87	59.50	40.65	41.98	26.05	1,167.81
	2	317.31	96.79	47.44	129.20	66.64	51.65	53.58	38.13	55.74	36.45	42.76	74.21	1,009.88
	3	433.01	4,725.39	25.01	119.44	60.51	55.77	59.05	46.80	49.49	32.64	45.28	733.28	6,385.68
TOTAL		792.83	5,302.95	119.68	437.89	217.31	165.20	166.68	122.80	164.73	109.74	130.02	833.55	8,563.37
1952-1953	1	92.60	1,744.39	1,006.68	102.09	90.36	33.66	28.50	26.46	22.06	22.48	23.33	19.32	3,211.93
	2	881.53	2,248.99	1,148.25	85.44	74.74	30.09	49.23	25.65	25.38	20.38	28.08	123.70	4,741.29
	3	435.49	2,447.73	206.31	106.65	51.22	31.43	45.77	18.29	24.83	18.68	29.24	1,113.61	4,529.24
TOTAL		1,409.61	6,441.11	2,361.24	294.17	216.31	95.17	123.50	70.40	72.08	61.54	80.65	1,256.66	12,482.46
1953-1954	1	238.83	460.76	1,148.62	125.62	85.92	43.04	115.03	67.88	70.66	43.25	28.18	18.04	2,445.82
	2	2,473.66	218.68	395.03	117.32	72.11	36.41	41.41	55.60	56.98	29.48	31.04	62.98	3,590.70
	3	2,336.85	844.42	266.69	110.34	56.35	39.97	50.57	48.75	53.74	24.92	34.55	144.85	4,011.99
TOTAL		5,049.33	1,523.86	1,810.34	353.27	214.38	119.43	207.01	172.23	181.37	97.65	93.77	225.87	10,048.51
1954-1955	1	56.98	945.96	496.70	1,538.26	237.62	83.24	61.37	39.29	51.35	22.98	22.83	21.88	3,578.46
	2	416.30	1,224.06	450.71	742.20	138.99	68.22	61.74	41.92	47.52	21.89	25.12	29.44	3,268.12
	3	1,342.61	1,346.59	371.40	286.15	97.43	69.62	92.57	37.65	36.27	20.26	38.94	194.77	3,934.25
TOTAL		1,815.89	3,516.60	1,318.82	2,566.61	474.04	221.08	215.68	118.87	135.14	65.13	86.89	246.09	10,780.84
1955-1956	1	627.85	1,034.11	628.36	2,490.77	398.31	133.07	81.43	51.01	47.67	40.40	24.72	405.57	5,963.26
	2	628.80	1,489.86	435.03	2,336.40	194.30	108.09	68.07	39.09	55.24	34.21	46.76	696.13	6,131.98
	3	675.81	853.25	625.67	618.82	143.26	102.92	71.13	40.97	45.54	25.44	504.86	639.76	4,347.42
TOTAL		1,932.45	3,377.21	1,689.06	5,445.99	735.87	344.09	220.63	131.07	148.44	100.06	576.33	1,741.46	16,442.65
1956-1957	1	1,862.75	771.38	807.41	714.52	512.04	106.74	76.29	49.09	44.66	36.31	22.95	20.92	5,025.05
	2	592.74	651.16	501.76	1,789.92	205.94	70.24	86.22	33.48	45.69	60.16	27.85	26.80	4,091.96
	3	709.97	-	385.61	648.59	130.84	73.83	82.57	26.47	12.44	26.74	31.13	32.06	2,160.24
TOTAL		3,165.45	1,422.54	1,694.79	3,153.02	848.82	250.80	245.08	109.04	102.79	123.21	81.93	79.79	11,277.25
1957-1958	1	82.00	404.84	344.63	136.60	170.56	58.04	58.33	48.47	43.40	19.32	32.15	21.90	1,420.23
	2	270.08	117.87	1,170.22	135.83	78.36	80.71	49.57	39.48	37.72	17.78	72.74	26.42	2,096.78
	3	952.77	331.19	184.04	101.24	64.80	70.71	63.45	36.35	33.21	15.17	35.52	24.03	1,912.48
TOTAL		1,304.85	853.90	1,698.89	373.67	313.71	209.46	171.35	124.30	114.33	52.28	140.42	72.35	5,429.49

(cumec day)

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1958-1959	1	726.58	287.17	413.01	689.75	230.15	90.11	61.24	47.54	62.61	36.12	27.55	22.62	2,694.43
	2	779.47	103.34	338.28	371.25	158.30	60.49	54.67	48.45	45.28	28.91	32.32	35.06	2,055.82
	3	1,328.05	990.50	824.58	295.58	115.44	81.27	71.37	49.23	37.93	26.21	39.49	48.13	3,907.77
TOTAL		2,834.11	1,381.00	1,575.87	1,356.58	503.88	231.87	187.28	145.22	145.81	91.25	99.36	105.81	8,658.02
1959-1960	1	361.57	785.65	319.10	324.71	222.47	81.10	51.73	45.93	41.07	36.62	26.80	21.23	2,317.97
	2	250.37	717.21	495.87	317.20	157.95	57.08	52.36	38.23	42.79	30.57	30.52	27.72	2,217.84
	3	732.69	586.98	448.51	244.41	104.26	60.53	62.09	31.09	38.22	26.55	31.56	33.00	2,399.87
TOTAL		1,344.63	2,089.83	1,263.48	886.32	484.67	198.70	166.18	115.24	122.08	93.74	88.87	81.95	6,935.69
1960-1961	1	103.09	688.80	1,051.26	623.45	178.23	41.78	65.48	75.38	49.51	31.16	24.28	27.49	2,987.23
	2	795.75	612.91	433.03	433.03	117.62	49.13	92.79	61.60	54.49	24.65	32.37	115.20	3,169.78
	3	375.17	976.73	449.03	268.19	93.21	50.16	64.09	59.52	41.58	21.80	38.57	194.54	2,632.57
TOTAL		1,274.01	2,278.44	2,307.84	1,324.67	389.06	141.07	222.36	196.50	145.58	77.61	95.22	337.23	8,789.58
1961-1962	1	517.13	1,080.19	452.75	152.66	232.06	77.63	67.44	79.10	235.17	42.14	29.51	23.08	2,988.85
	2	755.75	1,508.64	298.62	531.33	130.05	86.09	64.87	52.96	61.21	33.49	29.42	120.38	3,672.80
	3	708.13	1,420.31	222.74	283.69	100.29	90.72	175.05	80.42	49.02	28.66	36.68	193.29	3,388.99
TOTAL		1,981.00	4,009.14	974.11	967.68	462.40	254.44	307.36	212.48	345.40	104.28	95.60	336.75	10,050.64
1962-1963	1	68.96	228.95	218.57	225.26	127.83	68.69	52.46	45.62	49.61	42.88	38.89	25.69	1,193.40
	2	115.76	543.89	175.41	218.20	99.09	58.31	53.92	38.90	58.56	35.36	41.36	48.67	1,487.42
	3	570.21	493.47	2,105.00	151.83	73.51	58.27	62.09	35.30	48.01	29.67	36.25	50.39	3,714.01
TOTAL		754.93	1,266.31	2,498.97	595.29	300.43	185.27	168.47	119.82	156.18	107.91	116.51	124.76	6,394.83
1963-1964	1	153.55	331.94	857.16	233.91	171.06	84.88	62.56	59.63	64.84	35.13	36.31	24.43	2,115.39
	2	253.51	493.32	837.70	234.43	114.85	78.97	62.69	23.40	56.81	29.43	38.17	40.41	2,263.69
	3	346.05	2,602.45	427.35	209.07	95.35	67.48	70.81	56.76	37.45	29.04	48.82	95.17	4,085.80
TOTAL		753.10	3,427.70	2,122.21	677.40	381.26	231.33	196.06	139.79	159.10	93.61	123.30	160.01	8,464.88
1964-1965	1	406.68	886.59	985.02	403.95	140.96	68.50	52.46	42.64	46.60	51.69	25.91	22.58	3,133.57
	2	800.50	517.63	665.58	198.60	96.75	64.30	65.21	44.32	42.37	36.19	32.61	29.88	2,593.92
	3	1,681.97	766.92	1,117.32	167.33	77.35	54.92	89.85	49.88	51.08	29.19	37.90	36.35	4,160.05
TOTAL		2,889.14	2,171.13	2,767.92	769.87	315.05	187.71	207.52	136.85	140.05	117.06	96.42	88.81	9,887.54
1965-1966	1	100.71	562.52	401.07	101.63	78.04	43.55	34.31	27.58	27.20	25.83	19.87	21.18	1,443.48
	2	204.96	131.05	237.92	72.87	54.57	37.59	34.68	28.48	27.68	22.15	22.80	28.53	903.27
	3	175.49	1,058.30	149.48	91.89	44.52	38.57	34.01	18.45	26.66	18.87	31.98	211.07	1,899.29
TOTAL		481.16	1,751.87	788.47	266.38	177.13	119.71	103.00	74.51	81.54	66.85	74.65	260.78	4,246.04
1966-1967	1	97.00	783.76	246.37	100.33	84.81	45.00	44.34	37.06	34.86	41.52	34.73	24.60	1,574.38
	2	84.07	340.32	213.13	73.06	65.51	36.97	42.16	30.88	38.64	35.83	35.85	36.52	1,032.94
	3	591.89	288.51	128.61	90.08	49.14	48.08	45.93	27.85	58.10	33.99	40.34	49.61	1,452.14
TOTAL		772.96	1,412.59	588.11	263.48	199.45	130.05	132.43	95.79	131.60	111.33	110.93	110.73	4,059.45
1967-1968	1	269.67	629.44	700.74	183.73	121.27	63.02	54.44	58.39	63.19	37.43	29.63	25.71	2,236.64
	2	639.75	607.22	626.38	152.07	77.29	52.09	52.09	45.52	46.94	29.17	33.65	132.58	2,494.75
	3	588.85	2,055.17	334.92	157.00	65.00	60.53	72.01	68.43	44.71	27.99	40.59	674.24	4,189.42
TOTAL		1,498.27	3,291.82	1,662.04	492.79	263.55	175.64	178.54	172.34	154.83	94.58	103.87	832.53	8,920.81
1968-1969	1	230.66	199.35	192.55	161.76	87.74	51.61	46.78	36.44	36.71	29.18	29.69	21.16	1,123.63
	2	276.72	632.21	203.61	82.37	69.41	45.82	52.36	33.91	30.84	26.83	44.32	26.15	1,524.54
	3	255.09	560.01	261.51	72.95	56.16	55.65	50.65	29.95	29.67	23.14	42.35	37.29	1,474.42
TOTAL		762.47	1,391.57	657.68	317.07	213.30	153.08	149.79	100.30	97.21	79.15	116.37	84.60	4,122.59

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1969-1970	1	45.00	637.80	540.11	319.25	129.04	63.65	48.57	41.84	29.92	23.60	19.93	46.39	1,945.09
	2	560.47	277.25	475.97	212.55	84.18	53.16	74.53	34.63	26.35	21.01	21.93	233.40	2,075.42
	3	459.17	801.45	1,597.81	194.88	70.00	58.70	63.93	30.03	26.48	18.72	31.31	737.96	4,090.44
	TOTAL	1,064.65	1,716.49	2,613.88	726.68	283.22	175.50	187.02	106.50	82.75	63.33	73.18	1,017.76	8,110.96
1970-1971	1	291.45	323.99	376.71	130.43	81.88	43.74	25.20	23.55	32.54	18.76	16.21	45.98	1,410.43
	2	107.14	638.50	258.42	83.26	65.86	35.24	23.93	20.80	22.20	17.63	56.85	312.30	1,642.13
	3	297.17	397.54	191.73	83.03	43.48	32.83	28.01	14.97	22.23	15.46	93.11	146.18	1,365.74
	TOTAL	695.77	1,360.02	826.86	296.72	191.22	111.81	77.15	59.32	76.97	51.85	166.17	504.45	4,418.30
1971-1972	1	522.22	277.00	714.40	98.32	103.09	50.86	32.40	33.78	26.91	19.26	14.83	18.55	1,911.59
	2	204.01	388.00	334.53	85.24	60.32	46.05	32.23	29.11	24.11	17.89	21.18	20.05	1,262.72
	3	203.25	226.14	141.49	100.25	58.63	42.90	64.25	16.43	21.82	16.18	17.77	27.07	936.17
	TOTAL	929.48	891.14	1,190.42	283.81	222.04	139.80	128.88	79.31	72.84	53.33	53.78	65.67	4,110.47
1972-1973	1	149.36	146.78	228.43	127.50	88.75	46.19	32.99	24.35	21.38	20.38	18.61	21.19	925.91
	2	310.30	234.49	787.05	123.75	66.22	37.36	32.77	24.07	33.33	19.86	20.31	81.23	1,771.13
	3	206.21	147.86	186.48	173.81	42.64	35.33	42.41	14.64	24.00	18.00	29.36	241.81	1,162.55
	TOTAL	665.87	529.14	1,201.96	425.06	197.60	119.29	108.17	63.06	78.71	58.24	68.28	344.23	3,859.60
1973-1974	1	103.41	378.27	207.11	188.28	139.35	59.61	43.48	31.73	19.15	22.61	17.35	19.25	1,229.59
	2	116.27	158.33	272.15	152.36	86.45	50.64	32.50	31.32	21.46	20.17	18.34	36.03	996.02
	3	174.61	163.09	180.11	154.37	66.04	52.96	36.33	20.07	23.77	17.81	19.24	20.99	929.39
	TOTAL	394.29	699.69	659.37	495.01	291.83	163.21	112.32	83.12	64.37	60.59	54.92	76.27	3,155.00
1974-1975	1	43.68	238.13	162.33	60.81	36.93	29.18	46.72	46.73	37.29	23.10	16.09	19.77	760.77
	2	32.83	112.18	134.13	29.80	36.68	30.76	37.81	42.84	33.99	19.97	17.35	31.55	559.87
	3	389.33	118.08	85.39	54.58	29.51	38.45	65.37	41.05	29.25	17.91	20.88	339.70	1,229.49
	TOTAL	465.84	468.39	381.86	145.19	103.12	98.39	149.89	130.62	100.53	60.98	54.32	391.02	2,550.13
1975-1976	1	329.08	683.15	874.07	188.21	107.43	51.99	41.24	33.09	37.58	24.78	24.40	21.18	2,416.19
	2	374.83	232.28	591.54	177.31	81.77	49.18	39.57	39.76	30.25	26.26	34.93	45.43	1,723.12
	3	244.53	277.45	305.77	156.09	60.84	48.75	41.77	41.62	26.01	20.60	25.58	34.33	1,283.33
	TOTAL	948.45	1,192.88	1,771.37	521.61	250.03	149.93	122.58	114.48	93.84	71.63	84.91	100.93	5,422.64
1976-1977	1	44.53	323.08	223.05	132.18	76.02	47.71	32.66	31.73	31.86	22.88	23.67	23.68	1,013.05
	2	151.74	1,093.25	217.75	88.21	64.51	40.95	29.17	27.48	25.09	22.67	36.39	31.68	1,828.89
	3	292.21	388.32	178.86	94.59	49.20	48.39	31.13	20.48	24.70	22.42	34.51	92.15	1,276.96
	TOTAL	488.48	1,804.65	619.66	314.98	189.73	137.04	92.96	79.68	81.65	67.97	94.57	147.51	4,118.89
1977-1978	1	75.20	184.62	206.93	128.05	85.72	63.10	44.68	38.51	57.92	27.50	16.43	14.34	943.00
	2	156.83	817.10	351.99	95.67	74.24	44.44	44.36	53.58	83.33	28.64	16.48	16.87	1,778.53
	3	463.58	218.68	176.20	99.54	61.05	53.74	45.83	28.78	54.47	24.53	19.35	99.42	1,345.17
	TOTAL	695.61	1,215.40	735.12	323.26	221.01	161.28	134.87	120.87	195.72	80.67	52.26	130.63	4,066.70
1978-1979	1	450.00	926.66	988.82	170.77	80.02	61.43	42.83	48.89	61.82	34.78	21.80	18.85	2,906.67
	2	553.00	339.00	345.50	116.69	70.22	54.02	43.77	51.47	47.74	30.09	37.77	31.68	1,720.95
	3	209.00	357.00	230.00	102.23	65.67	49.21	55.76	53.64	45.22	27.33	32.53	96.00	1,323.59
	TOTAL	1,212.00	1,622.66	1,564.32	389.69	215.91	164.66	142.36	154.00	154.78	92.20	92.10	146.53	5,951.21
1979-1980	1	82.77	126.82	138.84	83.22	43.37	27.37	28.01	27.84	32.57	20.76	18.04	19.39	649.00
	2	251.35	607.07	17.35	55.43	39.25	25.06	24.43	23.78	26.85	20.07	16.58	30.85	1,238.07
	3	396.05	370.06	97.64	54.12	33.44	28.26	26.04	19.81	21.75	19.86	15.58	38.93	1,121.54
	TOTAL	730.17	1,103.95	353.83	192.77	116.06	80.69	78.48	71.43	81.17	60.69	50.20	89.17	3,008.61

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1980-1981	1	134.07	501.54	517.36	215.47	65.56	35.38	30.64	34.57	25.98	22.14	18.64	17.05	1,618.40
	2	272.21	324.88	433.43	155.91	44.89	33.17	27.80	28.91	23.00	22.12	22.99	16.10	1,405.44
	3	504.37	178.07	393.12	115.62	36.99	35.05	68.51	21.26	25.34	21.06	19.56	281.62	1,700.57
TOTAL	910.68	1,004.49	1,343.91	487.00	147.44	103.60	103.60	126.95	84.74	74.32	65.32	61.19	314.77	4,724.41
1981-1982	1	192.02	472.00	184.36	77.00	40.00	46.00	36.30	40.00	54.00	40.00	40.00	45.23	1,261.41
	2	916.45	231.00	152.56	70.00	43.00	31.00	33.99	36.00	42.00	34.50	46.00	60.00	1,696.50
	3	816.57	198.00	51.56	78.00	50.00	35.50	39.86	24.00	45.50	32.00	35.70	46.00	1,452.69
TOTAL	1,925.04	901.00	388.48	225.00	133.00	112.50	112.50	110.15	100.00	141.50	101.00	121.70	151.23	4,410.60
1982-1983	1	77.11	291.28	1,027.02	442.66	50.98	33.42	32.78	25.31	33.44	18.78	31.47	21.96	2,086.21
	2	56.46	276.82	802.37	249.02	44.19	30.64	27.88	28.01	20.66	43.80	35.19	22.14	1,637.18
	3	370.04	1,559.09	838.82	68.87	37.21	35.62	33.59	20.92	22.78	44.93	51.49	130.86	3,214.22
TOTAL	503.61	2,127.19	2,668.21	760.55	132.38	99.68	99.68	94.25	74.24	76.88	107.51	118.15	174.96	6,937.61
1983-1984	1	144.08	242.00	298.11	354.76	68.91	57.30	55.09	67.57	13.15	13.36	13.15	29.16	1,356.64
	2	50.29	76.84	308.10	188.56	64.20	57.21	55.09	75.06	13.36	13.17	14.78	35.98	952.64
	3	655.74	121.90	1,819.88	139.16	60.21	57.24	62.05	88.30	14.75	13.14	19.67	368.71	3,420.75
TOTAL	850.11	440.74	2,426.09	682.48	193.32	171.75	171.75	172.23	230.93	41.26	39.67	47.60	433.85	5,730.03
1984-1985	1	523.76	1,190.47	1,062.50	685.33	38.41	23.62	23.52	16.97	14.82	11.16	12.53	9.52	3,612.61
	2	348.56	1,063.62	1,024.98	158.67	29.84	22.46	20.22	16.33	13.21	10.52	12.43	14.65	2,735.49
	3	1,352.27	1,395.48	960.99	54.77	27.24	22.91	19.12	12.43	12.82	9.91	12.41	31.07	3,911.42
TOTAL	2,224.59	3,649.57	3,048.47	898.77	95.49	68.99	68.99	62.86	45.73	40.85	31.59	37.37	55.24	10,259.52
1985-1986	1	70.50	608.68	547.22	437.62	183.32	78.74	57.37	34.20	37.54	24.69	20.87	22.46	2,123.21
	2	224.96	436.09	635.57	3,219.80	108.84	62.62	47.72	67.88	31.92	23.19	21.01	34.81	4,914.41
	3	922.98	880.49	382.59	404.23	84.53	71.79	44.41	34.47	31.55	22.18	28.13	92.59	2,999.94
TOTAL	1,218.44	1,925.26	1,565.38	4,061.65	376.69	213.15	213.15	149.50	136.55	101.01	70.06	70.01	149.86	10,037.56
1986-1987	1	656.89	970.98	414.94	458.14	187.52	91.79	88.78	57.18	29.18	18.95	46.26	80.79	3,101.40
	2	1,182.79	599.01	1,121.72	324.48	144.37	185.25	82.77	63.99	23.86	17.14	25.68	20.25	3,791.31
	3	1,253.21	896.32	605.43	228.94	102.52	151.94	76.76	39.85	22.98	15.74	18.35	17.92	3,429.96
TOTAL	3,092.89	2,466.31	2,142.09	1,011.56	434.41	428.98	428.98	248.31	161.02	76.02	51.83	90.29	118.96	10,322.67
1987-1988	1	139.82	127.00	353.50	34.03	39.30	29.49	26.54	21.04	20.98	19.91	55.66	15.48	882.75
	2	536.10	394.00	163.50	66.81	36.80	28.56	25.51	20.45	67.12	19.07	15.34	15.39	1,388.65
	3	292.50	957.00	170.00	70.32	32.38	27.38	27.02	17.83	24.99	19.25	16.54	36.66	1,691.87
TOTAL	968.42	1,478.00	687.00	171.16	108.48	85.43	85.43	79.07	59.32	113.09	58.23	87.54	67.53	3,963.27
1988-1989	1	188.54	340.50	139.42	130.78	43.22	32.75	51.12	26.65	20.41	18.22	14.82	15.66	1,022.09
	2	163.50	224.00	128.39	72.37	38.22	29.36	44.18	23.92	21.94	17.05	13.48	25.56	801.97
	3	321.00	171.00	192.81	59.95	42.03	33.10	33.11	18.20	20.38	15.82	15.35	52.16	974.91
TOTAL	673.04	735.50	460.62	263.10	123.47	95.21	95.21	128.41	68.77	62.73	51.09	43.65	93.38	2,798.97
1989-1990	1	40.13	445.00	763.00	178.30	49.37	36.79	27.14	22.89	185.34	35.79	20.21	21.10	1,825.06
	2	24.83	322.80	404.00	99.19	44.77	32.07	26.28	64.86	45.27	26.75	23.22	18.60	1,132.64
	3	591.00	563.00	286.00	75.61	43.75	32.19	26.82	61.88	43.11	20.32	26.84	30.36	1,801.38
TOTAL	655.96	1,330.80	1,453.00	353.10	137.89	101.05	101.05	80.24	149.63	273.72	83.36	70.27	70.06	4,759.08
1990-1991	1	2,208.61	1,417.00	802.00	225.00	62.35	36.16	57.25	21.36	36.11	18.31	15.38	19.02	4,918.55
	2	575.00	435.00	857.00	146.63	51.65	30.09	31.94	24.44	28.46	19.73	13.81	30.38	2,244.13
	3	404.00	1,160.00	363.00	89.42	45.41	75.12	27.98	23.87	24.28	16.73	24.96	22.89	2,277.66
TOTAL	3,187.61	3,012.00	2,022.00	461.05	159.41	141.37	141.37	117.17	69.67	88.85	54.77	54.15	72.29	9,440.34

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1991-1992	1	22.92	144.62	120.78	68.58	29.03	27.75	28.18	34.74	20.07	15.76	13.18	19.41	545.02
	2	26.13	174.18	97.94	47.66	37.93	24.07	27.86	31.46	18.32	14.91	12.37	22.41	485.24
	3	53.33	182.16	96.63	41.28	29.26	39.36	30.19	20.41	19.19	13.73	14.13	69.35	609.02
TOTAL		102.38	450.96	315.35	157.52	96.22	91.18	86.23	86.61	57.58	44.40	39.68	111.17	1,639.28
1992-1993	1	27.49	239.90	47.84	16.44	37.51	17.68	20.81	18.40	18.40	22.74	14.67	12.63	494.51
	2	40.34	85.89	67.10	47.09	32.39	25.37	20.46	18.62	18.73	17.99	16.24	13.25	403.47
	3	265.94	107.96	25.46	54.02	31.82	25.97	20.85	15.50	40.83	15.88	14.94	16.05	635.22
TOTAL		333.77	433.75	140.40	117.55	101.72	69.02	62.12	52.52	77.96	56.61	45.85	41.93	1,533.20
1993-1994	1	32.61	280.23	1,588.86	46.46	63.01	35.29	28.76	20.27	24.92	19.02	14.96	14.67	2,169.06
	2	94.24	194.89	3,108.50	59.43	47.59	32.67	34.72	29.24	22.48	16.72	18.28	13.77	3,672.53
	3	407.97	127.83	320.10	84.70	37.61	33.70	32.71	25.83	22.74	15.39	16.44	26.35	1,151.57
TOTAL		534.82	602.95	5,017.46	190.59	148.21	101.66	96.19	75.34	70.14	51.33	49.68	54.79	6,993.16
1994-1995	1	36.35	213.76	91.46	72.47	38.35	26.37	27.91	25.99	24.98	21.14	18.50	13.61	610.89
	2	67.00	333.25	78.02	59.33	33.62	24.80	31.53	24.95	25.03	20.65	16.82	12.14	727.14
	3	94.00	193.29	59.84	47.81	29.18	26.70	27.71	21.57	24.05	19.40	16.88	63.14	623.57
TOTAL		197.35	740.30	229.32	179.61	101.15	77.87	87.15	72.51	74.06	61.19	52.20	88.89	1,961.60
1995-1996	1	53.69	60.31	1,961.25	107.97	39.78	30.48	23.83	23.80	26.64	19.99	13.74	18.49	2,379.97
	2	120.51	464.97	321.62	70.76	33.69	28.11	28.51	26.74	22.11	16.10	14.02	40.80	1,187.94
	3	62.07	139.81	155.39	68.13	31.59	28.69	27.62	27.85	24.39	14.39	13.76	140.08	733.77
TOTAL		236.27	665.09	2,438.26	246.86	105.06	87.28	79.96	78.39	73.14	50.48	41.52	199.37	4,301.68
1996-1997	1	121.32	138.83	520.97	75.80	42.23	34.91	27.82	25.26	20.90	20.38	20.38	15.58	1,073.07
	2	185.66	243.63	261.96	61.89	42.91	31.29	26.45	22.54	21.52	20.04	17.07	18.07	953.03
	3	124.16	376.57	133.54	60.05	37.82	31.86	29.94	16.77	21.44	18.62	16.57	34.60	901.94
TOTAL		431.14	759.03	916.47	197.74	122.96	98.06	84.21	64.57	63.86	67.73	54.02	68.25	2,928.04
1997-1998	1	46.29	209.20	76.60	69.00	51.31	52.08	43.75	31.00	33.23	36.62	41.94	17.85	708.87
	2	81.20	138.10	75.80	63.00	48.34	61.55	41.45	30.00	33.54	34.48	28.88	46.00	682.34
	3	79.10	82.50	71.00	67.90	42.93	45.52	36.86	24.47	35.30	29.98	22.40	102.50	640.46
TOTAL		206.59	429.80	223.40	199.90	142.58	159.15	122.06	85.47	102.07	101.08	93.22	166.35	2,031.67
1998-1999	1	1,206.85	774.29	482.86	309.71	366.39	96.49	51.85	44.30	40.46	21.45	12.43	17.08	3,424.18
	2	531.02	1,751.14	379.47	854.94	200.17	63.99	49.86	36.40	41.23	15.79	14.37	91.37	4,029.72
	3	773.48	1,668.05	645.82	577.83	103.38	57.55	57.74	31.49	36.81	14.37	12.89	116.77	4,096.19
TOTAL		2,511.35	4,193.48	1,508.16	1,742.48	669.95	218.03	159.45	112.19	118.50	51.60	39.68	225.22	11,550.09
1999-2000	1	468.07	845.16	472.79	346.81	110.44	36.09	26.02	34.32	19.07	14.80	17.13	1,436.36	3,827.06
	2	459.17	498.85	818.88	254.75	75.23	36.07	22.18	27.71	17.43	15.62	14.50	946.23	3,186.62
	3	864.68	444.66	552.23	207.27	41.35	32.78	21.50	20.52	17.63	15.68	17.31	1,451.84	3,687.45
TOTAL		1,791.92	1,788.67	1,843.90	808.83	227.02	104.94	69.70	82.55	54.13	46.10	48.94	3,834.43	10,701.13
2000-2001	1	976.41	1,001.12	2,711.28	646.92	123.24	32.59	24.71	17.92	16.16	14.19	12.21	28.03	5,604.78
	2	1,839.71	2,839.24	1,083.72	207.78	59.25	29.07	21.41	18.22	15.09	14.45	14.25	19.89	6,162.08
	3	1,563.94	2,312.50	751.29	212.44	39.99	26.09	21.54	14.40	14.88	13.88	14.88	412.60	5,399.09
TOTAL		4,380.06	6,152.86	4,546.29	1,067.14	222.48	87.75	67.66	50.54	46.79	42.52	41.34	460.52	17,165.95
2001-2002	1	524.60	622.50	263.58	153.04	50.42	22.76	18.47	19.23	56.86	18.39	18.88	20.92	1,789.65
	2	535.68	690.67	201.40	34.74	32.27	21.23	18.16	68.54	19.45	16.93	16.31	23.68	1,679.06
	3	749.21	429.60	327.97	68.37	25.61	21.67	20.95	16.72	20.60	19.85	17.45	24.67	1,742.67
TOTAL		1,809.49	1,742.77	792.95	256.15	108.30	65.66	57.58	104.49	96.91	55.17	52.64	69.27	5,211.38

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
2002-2003	1	257.84	157.33	1,115.17	283.52	53.17	26.42	23.63	28.63	232.25	18.31	14.70	13.51	2,224.48
	2	174.58	515.04	830.52	257.45	42.06	24.85	21.52	418.79	49.38	16.68	13.56	13.61	2,378.04
	3	170.36	376.91	342.22	351.66	33.17	23.66	23.50	212.92	26.46	15.35	14.05	16.48	1,606.74
TOTAL	602.78	1,049.28	2,287.91	892.63	128.40	74.93	74.93	68.65	660.34	308.09	50.34	42.31	43.60	6,209.26
2003-2004	1	21.33	279.45	246.44	330.37	155.08	44.56	22.95	24.42	18.24	15.64	16.04	24.94	1,199.46
	2	1,148.40	869.70	417.04	229.81	159.59	34.19	21.62	21.87	17.54	14.59	13.04	28.43	2,975.82
	3	236.94	749.92	1,074.73	182.76	151.80	29.52	23.70	17.87	17.99	17.31	19.15	20.11	2,541.80
TOTAL	1,406.67	1,899.07	1,738.21	742.94	466.47	108.27	108.27	68.27	64.16	53.77	47.54	48.23	73.48	6,717.08
2004-2005	1	169.25	193.58	255.37	206.91	64.97	30.32	19.19	30.84	19.23	15.15	13.29	13.93	1,032.03
	2	223.37	266.69	202.62	283.04	49.81	25.72	23.66	34.70	17.23	13.76	12.63	12.08	1,165.31
	3	177.06	2,192.08	481.59	205.84	40.39	23.39	34.75	18.12	17.10	13.42	13.86	43.76	3,261.36
TOTAL	569.68	2,652.35	939.58	695.79	155.17	79.43	79.43	77.60	83.66	53.56	42.33	39.78	69.77	5,458.70
2005-2006	1	134.18	64.92	90.00	276.05	81.56	40.58	20.90	19.12	18.10	16.83	13.77	20.34	796.35
	2	222.56	126.60	289.78	159.47	77.24	32.48	22.60	18.89	20.32	15.91	19.76	14.58	1,020.19
	3	116.64	129.52	2,401.21	114.95	65.65	24.96	23.41	14.12	20.40	14.51	17.20	19.90	2,962.47
TOTAL	473.38	321.04	2,780.99	550.47	224.45	98.02	98.02	66.91	52.13	58.82	47.25	50.73	54.82	4,779.01

Table-4.4

Derived 10 - Daily Discharge of Gola River at Jamrani Dam Site by using Log Deviation Regression Method

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1948-1949	1	181.17	540.67	1,290.56	365.51	193.36	121.92	60.86	345.79	88.69	36.90	45.35	136.27	3,407.07
	2	314.63	2,087.94	871.94	526.01	157.44	89.51	49.40	47.48	48.02	41.14	59.93	41.12	4,334.54
	3	312.01	6,542.64	591.66	260.51	145.26	65.86	53.35	62.72	38.56	28.91	83.39	70.96	8,255.82
TOTAL	807.81	9,171.25	2,754.16	1,152.02	496.05	175.27	106.95	188.67	248.35	15,997.42	4,520.39	8,077.98	9,968.69	22,567.06
1949-1950	1	289.62	1,552.46	1,301.05	833.18	150.89	82.68	66.24	68.55	59.96	40.75	30.57	24.43	4,520.39
	2	1,705.64	3,200.38	1,442.90	317.34	110.89	67.83	56.07	72.07	53.00	28.97	47.01	975.89	8,077.98
	3	1,815.91	3,572.28	2,476.55	283.46	103.08	73.17	70.03	43.52	514.24	29.42	63.91	923.12	9,968.69
TOTAL	3,811.17	8,325.13	5,220.50	1,453.98	364.86	223.67	184.14	192.34	184.14	627.20	99.14	141.48	1,923.44	22,567.06
1950-1951	1	1,581.84	3,786.41	1,595.83	422.93	184.16	116.99	77.57	71.75	55.52	41.08	54.67	34.22	8,022.98
	2	2,509.57	5,225.34	1,864.15	227.35	123.27	93.84	71.73	58.31	51.09	32.16	73.27	35.42	10,365.51
	3	3,577.93	3,524.28	1,362.19	186.66	114.95	89.00	71.59	36.55	48.48	30.59	31.57	122.30	9,196.08
TOTAL	7,669.34	12,536.03	4,822.18	836.94	422.38	299.84	223.67	220.89	166.61	155.09	103.83	159.51	191.94	27,584.57
1951-1952	1	60.39	705.13	40.38	289.67	135.40	84.82	77.18	47.89	84.70	49.61	50.28	34.22	1,659.67
	2	603.20	349.44	47.94	189.27	84.73	74.46	69.78	56.67	76.85	41.14	59.25	115.26	1,767.98
	3	657.72	6,016.08	44.72	161.67	91.77	74.73	78.67	59.61	63.28	42.56	56.36	882.85	8,230.03
TOTAL	1,321.31	7,070.65	133.03	640.62	311.90	234.01	234.01	225.63	164.18	224.83	133.31	165.89	1,032.33	11,657.68
1952-1953	1	145.44	2,354.00	1,403.07	156.92	135.64	49.20	40.03	32.80	40.70	25.58	27.49	16.27	4,427.13
	2	1,600.25	3,590.40	1,805.95	144.63	94.87	41.74	64.08	34.31	37.84	20.13	37.74	196.87	7,668.80
	3	661.07	3,198.36	299.52	145.76	78.19	40.02	62.57	26.88	29.84	21.32	36.11	1,336.32	5,935.94
TOTAL	2,406.75	9,142.76	3,508.53	447.31	308.70	130.95	130.95	166.68	93.99	108.38	67.03	101.34	1,549.46	18,031.88
1953-1954	1	393.75	679.02	1,604.65	192.75	130.32	63.05	165.89	87.58	97.81	53.05	33.42	12.86	3,514.16
	2	4,413.74	532.99	603.05	177.15	91.58	51.34	53.85	87.98	78.44	32.03	42.08	96.74	6,260.96
	3	3,227.90	1,214.88	384.38	150.35	85.69	52.20	68.39	61.85	69.04	30.81	42.81	181.26	5,569.55
TOTAL	8,035.39	2,426.89	2,592.09	520.26	307.59	166.59	166.59	288.12	237.40	245.29	115.89	118.31	290.86	15,344.67
1954-1955	1	84.96	1,312.14	678.75	2,344.32	312.06	122.39	87.84	49.77	75.13	26.24	26.87	23.09	5,143.56
	2	778.13	2,046.98	691.97	814.67	175.42	99.62	80.46	63.47	66.36	22.10	33.41	41.47	4,914.04
	3	1,885.68	1,836.12	531.54	369.04	145.73	94.48	119.31	49.10	45.36	23.73	48.36	240.78	5,389.22
TOTAL	2,748.77	5,195.24	1,902.26	3,528.02	633.21	316.49	316.49	287.61	162.35	186.84	72.07	108.63	305.34	15,446.82
1955-1956	1	1,054.35	1,427.17	865.73	3,795.07	504.57	195.95	117.02	65.27	70.79	49.28	29.18	1,046.26	9,220.65
	2	1,153.65	2,447.23	666.93	2,441.07	244.75	160.14	88.73	58.39	76.21	38.22	65.11	1,140.27	8,580.70
	3	985.50	1,225.80	888.89	782.85	212.71	141.98	93.31	52.92	57.92	31.61	636.48	771.34	5,881.31
TOTAL	3,193.50	5,100.20	2,421.55	7,018.99	962.03	204.93	498.08	299.07	176.58	204.93	119.11	730.78	2,957.87	23,682.66
1956-1957	1	3,151.35	1,084.34	1,120.04	1,089.69	640.82	157.08	109.54	62.73	67.26	43.87	27.03	20.54	7,574.28
	2	1,089.92	1,184.26	773.50	1,883.55	259.35	102.68	112.50	48.33	64.02	72.15	37.40	37.11	5,664.77
	3	1,031.62	154.08	551.51	819.88	194.56	100.49	107.19	36.27	13.04	33.58	38.50	46.78	3,127.48
TOTAL	5,272.89	2,422.67	2,445.06	3,793.12	1,094.72	360.24	360.24	329.22	147.33	144.32	149.60	102.93	104.44	16,366.53
1957-1958	1	127.44	606.05	462.76	209.48	231.72	85.19	83.42	61.91	65.78	21.40	38.27	23.14	2,016.55
	2	519.74	381.18	1,841.04	196.04	99.41	118.58	64.53	59.08	53.85	16.73	103.19	36.49	3,489.86
	3	1,359.40	579.96	268.22	139.03	98.04	96.05	84.00	47.62	41.20	15.99	44.04	37.20	2,810.74
TOTAL	2,006.58	1,567.19	2,572.01	544.55	429.17	299.81	299.81	231.95	168.61	160.83	54.12	185.50	96.83	8,317.14

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1958-1959	1	1,222.02	452.50	559.87	1,051.97	303.11	132.53	87.65	60.68	88.35	43.62	32.65	25.06	4,060.01
	2	1,419.90	359.30	512.42	436.22	199.63	87.89	71.20	75.16	63.49	31.28	43.95	50.73	3,351.17
	3	1,866.02	1,395.60	1,168.44	380.77	172.05	111.10	93.61	62.40	47.60	32.78	49.05	65.94	5,445.35
TOTAL	1	4,507.95	2,207.40	2,240.74	1,868.96	674.78	331.51	252.45	198.25	199.44	107.68	125.64	141.72	12,856.52
	2	602.19	1,102.96	426.50	495.99	293.91	119.23	73.82	58.55	63.04	44.28	31.72	21.36	3,333.55
	3	484.91	1,283.71	764.09	381.07	199.18	82.71	68.17	56.85	60.31	33.46	41.31	38.63	3,494.40
1959-1960	1	1,062.29	896.40	639.91	317.12	155.71	81.52	82.35	41.57	48.00	33.29	39.04	47.90	3,445.09
	2	2,149.39	3,283.07	1,830.50	1,194.18	648.80	283.45	224.35	156.97	171.36	111.02	112.07	107.88	10,273.03
	3	163.26	976.59	1,466.37	950.99	240.91	61.19	133.54	97.50	72.96	37.06	28.64	38.06	4,267.09
1960-1961	1	1,448.67	1,126.66	1,261.85	499.24	148.63	70.64	85.35	98.73	75.26	25.70	44.03	182.81	5,067.56
	2	579.64	1,378.56	640.64	346.70	139.56	66.73	84.78	74.21	52.56	26.06	47.89	240.50	3,677.82
	3	2,191.57	3,481.81	3,368.87	1,796.94	529.10	198.56	303.67	270.44	200.78	88.83	120.57	461.37	13,012.48
TOTAL	1	866.34	1,487.30	616.32	233.94	305.40	114.11	96.67	102.42	291.16	51.58	35.04	26.30	4,226.58
	2	247.04	1,022.72	252.31	280.07	125.40	84.58	70.22	58.05	80.45	39.71	57.21	73.16	2,390.91
	3	842.94	780.72	2,967.95	201.96	110.77	78.30	82.35	46.41	61.28	38.03	44.97	68.63	5,324.32
TOTAL	1	1,195.28	2,179.92	3,503.97	826.55	416.70	263.78	227.45	162.60	214.81	130.31	148.68	175.06	9,445.15
	2	248.94	510.93	1,900.70	357.69	232.32	124.81	89.57	76.67	90.97	42.31	43.35	29.90	3,038.16
	3	490.46	946.56	1,310.00	296.64	145.16	115.94	81.70	30.27	78.23	31.96	52.53	59.54	3,638.99
1963-1964	1	540.32	3,389.76	610.17	273.16	142.69	91.44	92.93	71.05	46.96	37.08	60.83	122.02	5,478.40
	2	1,279.72	4,847.25	3,110.87	927.48	520.17	332.18	264.20	177.99	216.16	111.36	156.71	211.46	12,155.55
	3	678.78	1,234.67	1,372.29	616.67	196.26	100.63	74.88	54.20	69.54	64.21	30.65	24.96	4,517.74
1964-1965	1	1,457.06	983.17	1,035.12	260.08	122.46	93.67	85.00	67.77	59.78	40.80	44.37	42.19	4,291.45
	2	2,343.82	1,119.00	1,579.86	221.24	116.38	73.52	116.01	63.15	65.44	37.30	47.05	51.89	5,834.65
	3	4,479.65	3,336.84	3,987.28	1,097.98	435.10	267.81	275.89	185.12	194.76	142.31	122.06	119.04	14,643.84
TOTAL	1	159.21	811.81	542.92	156.22	120.88	63.80	48.48	34.28	46.74	30.01	23.25	21.22	2,058.82
	2	404.67	401.02	352.14	131.81	69.60	53.13	45.03	39.39	41.02	22.44	30.01	39.96	1,630.21
	3	310.07	1,479.48	219.65	127.40	68.40	50.20	48.31	27.06	32.32	21.61	39.58	260.21	2,684.28
TOTAL	1	873.95	2,692.31	1,114.71	415.43	258.88	167.12	141.82	100.73	120.08	74.06	92.84	321.39	6,373.31
	2	152.91	1,100.50	323.20	154.24	128.99	65.94	63.07	46.82	55.75	50.76	41.43	30.34	2,213.93
	3	191.05	716.16	312.55	132.01	83.30	52.19	54.82	43.69	55.01	40.32	49.13	53.13	3,783.37
TOTAL	1	872.21	527.16	190.32	125.15	75.15	63.77	62.76	37.85	74.96	44.60	50.13	67.70	2,191.76
	2	1,216.17	2,343.82	826.07	411.40	287.44	181.90	180.66	128.36	185.72	135.69	140.68	151.17	6,189.07
	3	446.13	899.12	968.53	281.26	172.67	92.54	77.76	75.03	89.03	45.35	35.19	33.31	3,215.92
1967-1968	1	1,173.00	1,118.08	972.52	212.61	98.08	75.14	67.82	69.92	65.61	31.62	45.90	211.46	4,141.76
	2	868.10	2,712.72	480.27	208.39	98.33	81.52	94.38	84.44	56.80	35.48	50.44	812.45	5,583.31
	3	2,487.23	4,729.92	2,421.33	702.25	369.07	249.19	239.96	229.39	229.39	211.45	112.44	131.52	12,940.99
TOTAL	1	379.89	337.91	246.76	247.80	132.50	75.70	66.62	46.00	57.91	34.44	35.27	21.17	1,681.97
	2	531.48	1,155.71	297.35	141.50	88.20	65.62	68.17	49.11	45.05	28.56	61.54	36.05	2,568.34
	3	417.53	863.04	377.10	103.84	85.41	74.56	68.48	40.27	36.40	28.11	52.67	53.01	2,200.41
TOTAL	1	1,328.90	2,356.66	921.22	493.13	306.10	215.88	203.28	135.38	139.36	91.11	149.47	110.22	6,450.71

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1969-1970	1	64.62	910.04	740.39	487.67	181.98	93.47	69.22	53.14	49.93	27.06	23.33	88.46	2,789.31
	2	1,032.90	621.18	732.31	274.32	106.71	76.76	97.19	50.40	39.33	20.99	38.73	377.63	3,458.39
	3	693.04	1,161.72	2,255.14	255.51	105.64	78.91	84.58	40.36	32.08	21.34	38.73	888.43	5,655.53
TOTAL		1,790.56	2,692.94	3,727.83	1,017.50	394.34	249.13	250.99	143.89	121.34	69.39	90.79	1,354.52	11,903.22
1970-1971	1	483.12	500.55	508.33	200.08	125.48	64.08	35.23	28.95	53.01	20.66	18.79	87.36	2,125.63
	2	231.81	1,165.18	384.88	142.41	83.75	49.56	30.97	25.63	34.03	16.52	79.90	507.66	2,752.30
	3	474.34	662.04	279.03	116.38	66.88	42.02	41.03	23.06	26.32	53.61	116.73	182.84	2,047.10
TOTAL		1,189.27	2,327.77	1,772.24	458.87	276.11	155.65	107.24	77.64	113.36	53.63	215.42	777.85	6,925.02
1971-1972	1	874.98	439.24	987.94	151.17	150.89	74.59	45.70	42.48	46.40	21.32	17.09	14.21	2,865.99
	2	403.00	787.97	506.43	144.43	76.81	65.96	41.83	40.51	36.46	16.86	27.63	25.99	2,173.87
	3	347.54	450.00	208.42	137.80	89.02	56.38	84.97	24.74	25.76	17.52	21.64	40.83	1,504.61
TOTAL		1,629.52	1,677.20	1,702.79	433.41	316.71	196.92	172.50	107.72	108.62	55.70	66.36	81.02	6,544.47
1972-1973	1	241.83	269.32	297.72	195.62	133.71	67.70	46.56	30.01	39.90	22.80	21.71	21.26	1,388.15
	2	590.82	556.80	1,229.11	183.72	84.19	53.38	42.54	31.48	48.23	19.45	26.35	126.83	2,992.89
	3	351.54	353.16	271.65	229.30	65.65	45.59	58.49	22.69	28.72	20.29	36.27	296.86	1,780.20
TOTAL		1,184.19	1,179.28	1,798.48	608.64	283.54	166.67	147.59	84.18	116.85	62.54	84.33	444.95	6,161.24
1973-1974	1	163.80	571.38	267.44	288.19	194.33	87.51	61.82	39.77	37.28	25.75	20.17	16.08	1,773.52
	2	247.94	442.11	406.81	212.91	109.56	72.93	42.19	44.46	33.07	19.86	23.46	52.33	1,707.63
	3	308.88	372.00	262.70	205.12	99.85	70.73	51.12	28.92	28.40	20.00	23.49	33.57	1,504.78
TOTAL		720.62	1,385.49	936.96	706.22	403.73	231.17	155.13	113.16	98.75	65.61	67.12	101.99	4,985.94
1974-1975	1	62.37	388.52	203.84	94.05	71.63	42.59	66.53	59.61	58.60	26.40	18.63	17.47	1,110.25
	2	100.49	372.61	186.39	87.87	47.17	42.76	49.13	65.10	49.08	19.58	22.02	44.95	1,087.14
	3	598.75	316.32	129.58	80.99	46.46	50.03	86.33	53.01	35.84	20.15	25.56	413.57	1,856.59
TOTAL		761.61	1,077.45	519.82	262.91	165.26	135.37	201.99	177.73	143.51	66.14	66.21	475.99	4,053.98
1975-1976	1	547.02	969.21	1,214.71	288.09	156.09	76.26	58.56	41.57	58.94	28.62	28.80	21.22	3,489.08
	2	704.86	553.47	916.88	238.36	103.69	70.72	51.44	59.60	44.31	27.81	47.77	67.82	2,886.72
	3	403.27	513.48	439.30	207.26	92.25	64.73	57.72	53.66	31.44	24.24	31.49	49.48	1,968.31
TOTAL		1,655.15	2,036.16	2,570.89	733.71	352.02	211.71	167.72	154.83	134.69	80.67	108.06	138.51	8,344.11
1976-1977	1	63.81	499.37	290.08	202.75	118.46	69.94	46.08	39.77	31.86	22.88	23.67	23.68	1,432.35
	2	310.63	1,849.98	319.93	147.46	82.06	58.23	37.83	37.58	25.09	22.67	36.39	31.68	2,959.53
	3	467.64	650.64	260.94	130.76	75.24	64.21	44.81	29.39	24.70	22.42	34.51	92.15	1,897.41
TOTAL		842.08	2,999.99	870.95	480.98	275.76	192.37	128.72	106.74	81.65	67.97	94.57	147.51	6,289.28
1977-1978	1	75.20	184.62	206.93	128.05	85.72	63.10	44.68	38.51	57.92	27.50	16.43	14.34	943.00
	2	156.83	812.10	351.99	95.67	74.24	44.44	44.36	53.58	83.33	28.64	16.48	16.87	1,778.53
	3	463.58	218.68	176.20	99.54	61.05	53.74	45.83	28.78	54.47	24.53	19.35	99.42	1,345.17
TOTAL		695.61	1,215.40	735.12	323.26	221.01	161.28	134.87	120.87	195.72	80.67	52.26	130.63	4,066.70
1978-1979	1	450.00	926.66	988.82	170.77	80.02	61.43	42.83	48.89	61.82	34.78	21.80	18.85	2,906.67
	2	553.00	339.00	345.50	116.69	70.22	54.02	43.77	51.47	47.74	30.09	37.77	31.68	1,720.95
	3	209.00	357.00	230.00	102.23	65.67	49.21	55.76	53.64	45.22	27.33	32.53	96.00	1,323.59
TOTAL		1,212.00	1,622.66	1,564.32	389.69	215.91	164.66	142.36	154.00	154.78	92.20	92.10	146.53	5,951.21
1979-1980	1	82.77	126.82	138.84	83.22	43.37	27.37	28.01	27.84	32.57	20.76	18.04	19.39	649.00
	2	251.35	607.07	117.35	55.43	39.25	25.06	24.43	23.78	26.85	20.07	16.58	30.85	1,238.07
	3	396.05	370.06	97.64	54.12	33.44	28.26	26.04	19.81	19.81	19.86	15.58	38.93	1,121.54
TOTAL		730.17	1,103.95	353.83	192.77	116.06	80.69	78.48	71.43	81.17	60.69	50.20	89.17	3,008.61

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1980-1981	1	134.07	501.54	517.36	215.47	65.56	35.38	30.64	34.57	25.98	22.14	18.64	17.05	1,618.40
	2	272.24	324.88	433.43	155.91	44.89	33.17	27.80	28.91	23.00	22.12	22.99	16.10	1,405.44
	3	504.37	178.07	393.12	115.62	36.99	35.05	68.51	21.26	25.34	21.06	19.56	281.62	1,700.57
TOTAL	910.68	1,004.49	1,343.91	487.00	147.44	103.60	103.60	126.95	84.74	74.32	65.32	61.19	314.77	4,724.41
1981-1982	1	192.02	472.00	194.36	77.00	40.00	46.00	36.30	40.00	54.00	34.50	40.00	45.23	1,261.41
	2	916.45	231.00	152.56	70.00	43.00	31.00	33.99	36.00	42.00	34.50	46.00	60.00	1,696.50
	3	816.57	198.00	51.56	78.00	50.00	35.50	39.86	24.00	45.50	32.00	35.70	46.00	1,452.69
TOTAL	1,925.04	901.00	388.48	225.00	133.00	112.50	112.50	110.15	100.00	141.50	101.00	121.70	151.23	4,410.60
1982-1983	1	77.11	291.28	1,027.02	442.66	50.98	33.42	32.78	25.31	33.44	18.78	31.47	21.96	2,086.21
	2	56.46	276.82	802.37	249.02	44.19	30.64	27.88	28.01	20.66	43.80	35.19	22.14	1,637.18
	3	370.04	1,559.09	838.82	68.87	37.21	35.62	33.59	20.92	22.78	44.93	51.49	130.86	3,214.22
TOTAL	503.61	2,127.19	2,668.21	760.55	132.38	99.68	99.68	94.25	74.24	76.88	107.51	118.15	174.96	6,937.61
1983-1984	1	144.08	242.00	298.11	354.76	68.91	57.30	55.09	67.57	13.15	13.36	13.15	29.16	1,356.64
	2	50.29	76.84	308.10	188.56	64.20	57.21	55.09	75.06	13.36	13.17	14.78	35.98	952.64
	3	655.74	121.90	1,819.88	139.16	60.21	57.24	62.05	88.30	14.75	13.14	19.67	368.71	3,420.75
TOTAL	850.11	440.74	2,426.09	682.48	193.32	171.75	171.75	172.23	230.93	41.26	39.67	47.60	433.85	5,730.03
1984-1985	1	523.76	1,190.47	1,062.50	685.33	38.41	23.62	23.52	16.97	14.82	11.16	12.53	9.52	3,612.61
	2	348.56	1,063.62	1,024.98	158.67	29.84	22.46	20.22	16.33	13.21	10.52	12.43	14.65	2,735.49
	3	1,352.27	1,395.48	960.99	54.77	27.24	22.91	22.91	12.43	12.82	9.91	12.41	31.07	3,911.42
TOTAL	2,224.59	3,649.57	3,048.47	898.77	95.49	68.99	68.99	62.86	45.73	40.85	31.59	37.37	55.24	10,259.52
1985-1986	1	70.50	608.68	547.22	437.62	183.32	78.74	57.37	34.20	37.54	24.69	20.87	22.46	2,123.21
	2	224.96	436.09	635.57	3,219.80	108.84	62.62	47.72	67.88	31.92	23.19	21.01	34.81	4,914.41
	3	922.98	880.49	382.59	404.23	84.53	71.79	44.41	34.47	31.55	22.18	28.13	92.59	2,999.94
TOTAL	1,218.44	1,925.26	1,565.38	4,061.65	376.69	213.15	213.15	149.50	136.55	101.01	70.06	70.01	149.86	10,037.56
1986-1987	1	656.89	970.98	414.94	458.14	187.52	91.79	88.78	57.18	29.18	18.95	46.26	80.79	3,101.40
	2	1,182.79	599.01	1,121.72	324.48	144.37	185.25	82.77	63.99	23.86	17.14	25.68	20.25	3,791.31
	3	1,253.21	896.32	605.43	228.94	102.52	151.94	76.76	39.85	22.98	15.74	18.35	17.92	3,429.96
TOTAL	3,092.89	2,466.31	2,142.09	1,011.56	434.41	376.69	428.98	248.31	161.02	76.02	51.83	90.29	118.96	10,322.67
1987-1988	1	139.82	127.00	353.50	34.03	39.30	29.49	26.54	21.04	20.98	19.91	55.66	15.48	882.75
	2	536.10	394.00	163.50	66.81	36.80	28.56	25.51	20.45	67.12	19.07	15.34	15.39	1,388.65
	3	292.50	957.00	170.00	70.32	32.38	27.38	27.02	17.83	24.99	19.25	16.54	36.66	1,691.87
TOTAL	968.42	1,478.00	687.00	171.16	108.48	85.43	85.43	79.07	59.32	113.09	58.23	87.54	67.53	3,963.27
1988-1989	1	188.54	340.50	139.42	130.78	43.22	32.75	51.12	26.65	20.41	18.22	14.82	15.66	1,022.09
	2	163.50	224.00	128.39	72.37	38.22	29.36	44.18	23.92	21.94	17.05	13.48	25.56	801.97
	3	321.00	171.00	59.95	59.95	42.03	33.10	33.11	18.20	20.38	15.82	15.35	52.16	974.91
TOTAL	673.04	735.50	460.62	263.10	123.47	95.21	95.21	128.41	68.77	62.73	51.09	43.65	93.38	2,798.97
1989-1990	1	40.13	445.00	763.00	178.30	49.37	36.79	27.14	22.89	185.34	35.79	20.21	21.10	1,825.06
	2	24.83	322.80	404.00	99.19	44.77	32.07	26.28	64.86	45.27	26.75	23.22	18.60	1,132.64
	3	591.00	563.00	286.00	75.61	43.75	32.19	26.82	61.88	43.11	20.82	26.84	30.36	1,801.38
TOTAL	655.96	1,330.80	1,453.00	353.10	137.89	101.05	101.05	80.24	149.63	273.72	83.36	70.27	70.06	4,759.08
1990-1991	1	2,208.61	1,417.00	802.00	225.00	62.35	36.16	57.25	21.36	36.11	18.31	15.38	19.02	4,918.55
	2	575.00	435.00	857.00	146.63	51.65	30.09	31.94	24.44	28.46	19.73	13.81	30.38	2,244.13
	3	404.00	1,160.00	363.00	89.42	45.41	75.12	27.98	23.87	24.28	16.73	24.96	22.89	2,277.66
TOTAL	3,187.61	3,012.00	2,022.00	461.05	159.41	141.37	141.37	117.17	69.67	88.85	54.77	54.15	72.29	9,440.34

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1991-1992	1	22.92	144.62	120.78	68.58	29.03	27.75	28.18	34.74	20.07	15.76	13.18	19.41	545.02
	2	26.13	124.18	97.94	47.66	37.93	24.07	27.86	31.46	18.32	14.91	12.37	22.41	485.24
	3	53.33	182.16	96.63	41.28	29.26	39.36	30.19	20.41	19.13	13.73	-14.13	69.35	609.02
TOTAL	102.38	450.96	315.35	157.52	96.22	96.22	91.18	86.23	86.61	57.58	44.40	39.68	111.17	1,639.28
1992-1993	1	27.49	239.90	47.84	16.44	37.51	17.68	20.81	18.40	18.40	22.74	14.67	12.63	494.51
	2	40.34	85.89	67.10	47.09	32.39	25.37	20.46	18.62	18.73	17.99	16.24	13.25	403.47
	3	265.94	107.96	25.46	54.02	31.82	25.97	20.85	15.50	40.83	15.88	14.94	16.05	635.22
TOTAL	333.77	433.75	140.40	117.55	101.72	69.02	62.12	62.12	52.52	77.96	56.61	45.85	41.93	1,533.20
1993-1994	1	32.61	280.23	1,588.86	46.46	63.01	35.29	28.76	20.27	19.02	14.96	14.96	14.67	2,169.06
	2	94.24	194.89	3,108.50	59.43	47.59	32.67	34.72	29.24	16.72	18.28	18.28	13.77	3,672.53
	3	407.97	127.83	320.10	84.70	37.61	33.70	32.71	25.83	22.74	15.59	16.44	26.35	1,151.57
TOTAL	534.82	602.95	5,017.46	190.59	148.21	101.66	101.66	96.19	75.34	70.14	51.33	49.68	54.79	6,993.16
1994-1995	1	36.35	213.76	91.46	72.47	38.35	26.37	27.91	25.99	24.98	21.14	18.50	13.61	610.89
	2	67.00	333.25	78.02	59.33	33.62	24.80	31.53	24.95	25.03	20.65	16.82	12.14	727.14
	3	94.00	193.29	59.84	47.81	29.18	26.70	27.71	21.57	24.05	19.40	16.88	63.14	623.57
TOTAL	197.35	740.30	229.32	179.61	101.15	77.87	77.87	87.15	72.51	74.06	61.19	52.20	88.89	1,961.60
1995-1996	1	53.69	60.31	1,961.25	107.97	39.78	30.48	23.83	23.80	26.64	19.99	13.74	18.49	2,379.97
	2	120.51	464.97	321.62	70.76	33.69	28.11	28.51	26.74	22.11	16.10	14.02	40.80	1,187.94
	3	62.07	139.81	155.39	68.13	31.59	28.69	27.62	27.85	24.39	14.39	13.76	140.08	733.77
TOTAL	236.27	665.09	2,438.26	246.86	105.06	87.28	87.28	79.96	78.39	73.14	50.48	41.52	199.37	4,301.68
1996-1997	1	121.32	138.83	520.97	75.80	42.23	34.91	27.82	25.26	20.90	29.07	20.38	15.58	1,073.07
	2	185.66	243.63	261.96	61.89	42.91	31.29	26.45	22.54	21.52	20.04	17.07	18.07	953.03
	3	124.16	376.57	133.54	60.05	37.82	31.86	29.94	16.77	21.44	18.62	16.57	34.60	901.94
TOTAL	431.14	759.03	916.47	197.74	122.96	84.21	98.06	84.21	64.57	63.86	67.73	54.02	68.25	2,928.04
1997-1998	1	46.29	209.20	76.60	69.00	51.31	52.08	43.75	31.00	33.23	36.62	41.94	17.85	708.87
	2	81.20	138.10	75.80	63.00	48.34	61.55	41.45	30.00	33.54	34.48	28.88	46.00	682.34
	3	79.10	82.50	71.00	67.90	42.93	45.52	36.86	24.47	35.30	29.98	22.40	102.50	640.46
TOTAL	206.59	429.80	223.40	199.90	142.58	159.15	159.15	122.06	85.47	102.07	101.08	93.22	166.35	2,031.67
1998-1999	1	2,037.57	1,088.14	659.09	473.15	466.33	141.96	74.00	56.40	62.32	24.21	14.16	10.30	5,107.63
	2	980.85	2,840.69	578.21	929.67	252.11	93.19	64.90	51.08	58.32	14.12	17.64	143.53	6,024.32
	3	1,117.36	2,233.81	917.21	731.86	154.43	77.27	77.08	50.62	46.09	14.76	15.48	147.78	5,583.74
TOTAL	4,135.78	6,162.64	2,154.50	2,134.69	872.88	312.42	312.42	215.97	158.10	166.73	53.10	47.27	301.60	16,715.69
1999-2000	1	468.07	845.16	472.79	346.81	110.44	36.09	26.02	34.32	19.07	14.80	17.13	1,436.36	3,827.06
	2	459.17	498.85	818.88	254.75	75.23	36.07	22.18	27.71	17.43	15.62	14.50	946.23	3,186.62
	3	864.68	444.66	552.23	207.27	41.35	32.78	21.50	20.52	17.63	15.68	17.31	1,451.84	3,687.45
TOTAL	1,791.92	1,788.67	1,843.90	808.83	227.02	104.94	104.94	69.70	82.55	54.13	46.10	48.94	3,834.43	10,701.13
2000-2001	1	976.41	1,001.12	2,711.28	646.92	123.24	32.59	24.71	17.92	16.16	14.19	12.21	28.03	5,504.78
	2	1,839.71	2,839.24	1,083.72	207.78	59.25	29.07	21.41	18.22	15.09	14.45	14.25	19.89	6,162.08
	3	1,563.94	2,312.50	751.29	212.44	39.99	26.09	21.54	14.40	15.94	13.88	14.88	412.60	5,399.09
TOTAL	4,380.06	6,152.86	4,546.29	1,067.14	222.48	87.75	87.75	67.66	50.54	46.79	42.52	41.34	460.52	17,165.95
2001-2002	1	524.60	622.50	263.58	153.04	50.42	22.76	18.47	19.23	56.86	18.39	18.88	20.92	1,789.65
	2	535.68	690.67	201.40	34.74	32.27	21.23	18.16	68.54	19.45	16.93	16.31	23.68	1,679.06
	3	749.21	429.60	327.97	68.37	25.61	21.67	20.95	16.72	20.60	19.85	17.45	24.67	1,742.67
TOTAL	1,809.49	1,742.77	792.95	256.15	108.30	65.66	65.66	57.58	104.49	96.91	55.17	52.64	69.27	5,211.38

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
2002-2003	1	257.84	157.33	1,115.17	283.52	53.17	26.42	23.63	28.63	232.25	18.31	14.70	13.51	2,224.48
	2	174.58	515.04	830.52	257.45	42.06	24.85	21.52	418.79	49.38	16.68	13.56	13.61	2,378.04
	3	170.36	376.91	342.22	351.66	33.17	23.66	23.50	212.92	26.46	15.35	14.05	16.48	1,606.74
TOTAL	602.78	1,049.28	2,287.91	892.63	128.40	74.93	74.93	68.65	660.34	308.09	50.34	42.31	43.60	6,209.26
2003-2004	1	21.33	279.45	246.44	330.37	155.08	44.56	22.95	24.42	18.24	15.64	16.04	24.94	1,199.46
	2	1,148.40	869.70	417.04	229.81	159.59	34.19	21.62	21.87	17.54	14.59	13.04	28.43	2,975.82
	3	236.94	749.92	1,074.73	182.76	151.80	29.52	23.70	17.87	17.99	17.31	19.15	20.11	2,541.80
TOTAL	1,406.67	1,899.07	1,738.21	742.94	466.47	108.27	108.27	68.27	64.16	53.77	47.54	48.23	73.48	6,717.08
2004-2005	1	169.25	193.58	255.37	206.91	64.97	30.32	19.19	30.84	19.23	15.15	13.29	13.93	1,032.03
	2	223.37	266.69	202.62	283.04	49.81	25.72	23.66	34.70	17.23	13.76	12.63	12.08	1,165.31
	3	177.06	2,192.08	481.59	205.84	40.39	23.39	34.75	18.12	17.10	13.42	13.86	43.76	3,261.36
TOTAL	569.68	2,652.35	939.58	695.79	155.17	79.43	79.43	77.60	83.66	53.56	42.33	39.78	69.77	5,458.70
2005-2006	1	134.18	64.92	90.00	276.05	81.56	40.58	20.90	19.12	18.10	16.83	13.77	20.34	796.35
	2	222.56	126.60	289.78	159.47	77.24	32.48	22.60	18.89	20.32	15.91	19.76	14.58	1,020.19
	3	116.64	129.52	2,401.21	114.95	65.65	24.96	23.41	14.12	20.40	14.51	17.20	19.90	2,962.47
TOTAL	473.38	321.04	2,780.99	550.47	224.45	98.02	98.02	66.91	52.13	58.82	47.25	50.73	54.82	4,779.01

Table-4.5

**ANNUAL SERIES ARRANGED IN DESCENDING ORDER
(By Linear Regression Method)**

Year	Rank	Percentage of Time (%)	Annual flow in cumecs day	Annual flow in million cubic metre (MCM)
1	2	3	4	5
1950 - 51	1	2	18,957.83	1,638.0
2000 - 01	2	3	17,165.95	1,483.1
1955 - 56	3	5	16,442.65	1,420.6
1949 - 50	4	7	15,653.86	1,352.5
1952 - 53	5	8	12,482.46	1,078.5
1998 - 99	6	10	11,550.09	997.9
1948 - 49	7	12	11,527.04	995.9
1956 - 57	8	14	11,277.25	974.4
1954 - 55	9	15	10,780.84	931.5
1999 - 2000	10	17	10,701.13	924.6
1986 - 87	11	19	10,322.67	891.9
1984 - 85	12	20	10,259.52	886.4
1961 - 62	13	22	10,050.64	868.4
1953 - 54	14	24	10,048.51	868.2
1985 - 86	15	25	10,037.56	867.2
1964 - 65	16	27	9,887.54	854.3
1990 - 91	17	29	9,440.34	815.6
1967 - 68	18	31	8,920.81	770.8
1960 - 61	19	32	8,789.58	759.4
1958 - 59	20	34	8,658.02	748.1
1951 - 52	21	36	8,563.37	739.9
1963 - 64	22	37	8,464.88	731.4
1969 - 70	23	39	8,110.96	700.8
1993 - 94	24	41	6,993.16	604.2
1982 - 83	25	42	6,937.61	599.4
1959 - 60	26	44	6,935.69	599.2
2003 - 04	27	46	6,717.08	580.4
1962 - 63	28	47	6,394.83	552.5
2002 - 03	29	49	6,209.26	536.5
1978 - 79	30	51	5,951.21	514.2
1983 - 84	31	53	5,730.03	495.1
2004 - 05	32	54	5,458.70	471.6
1957 - 58	33	56	5,429.49	469.1
1975 - 76	34	58	5,422.64	468.5
2001 - 02	35	59	5,211.38	450.3
2005 - 06	36	61	4,779.01	412.9
1989 - 90	37	63	4,759.08	411.2
1980 - 81	38	64	4,724.41	408.2
1970 - 71	39	66	4,418.30	381.7
1981 - 82	40	68	4,410.60	381.1
1995 - 96	41	69	4,301.68	371.7
1965 - 66	42	71	4,246.04	366.9

Year	Rank	Percentage of Time (%)	Annual flow in cumecs day	Annual flow in million cubic metre (MCM)
1	2	3	4	5
1968 - 69	43	73	4,122.59	356.2
1976 - 77	44	75	4,118.89	355.9
1971 - 72	45	76	4,110.47	355.1
1977 - 78	46	78	4,066.70	351.4
1966 - 67	47	80	4,059.45	350.7
1987 - 88	48	81	3,963.27	342.4
1972 - 73	49	83	3,859.60	333.5
1973 - 74	50	85	3,155.00	272.6
1979 - 80	51	86	3,008.61	259.9
1996 - 97	52	88	2,928.04	253.0
1988 - 89	53	90	2,798.97	241.8
1974 - 75	54	92	2,550.13	220.3
1997 - 98	55	93	2,031.67	175.5
1994 - 95	56	95	1,961.60	169.5
1991 - 92	57	97	1,639.28	141.6
1992 - 93	58	98	1,533.20	132.5

NOTE :

1. 75% dependable water availability year 1976 - 1977
2. The 75% dependable annual flow = 355.90 MCM
3. Irrigation year has been assumed to commence from 1st July every year to 30th June of next year.

Table-4.6

**ANNUAL SERIES ARRANGED IN DESCENDING ORDER
(By Log Deviation Regression Method)**

Year	Rank	Percentage of Time (%)	Annual flow in cumecs day	Annual flow in million cubic metre (MCM)
1	2	3	4	5
1950 - 51	1	2	27,589.32	2,383.7
1955 - 56	2	3	23,690.76	2,046.9
1949 - 50	3	5	22,572.60	1,950.3
1952 - 53	4	7	18,035.77	1,558.3
2000 - 01	5	8	17,165.95	1,483.1
1998 - 99	6	10	16,715.69	1,444.2
1956 - 57	7	12	16,371.70	1,414.5
1948 - 49	8	14	15,997.42	1,382.2
1954 - 55	9	15	15,453.90	1,335.2
1953 - 54	10	17	15,353.22	1,326.5
1964 - 65	11	19	14,653.59	1,266.1
1961 - 62	12	20	14,576.49	1,259.4
1960 - 61	13	22	13,023.05	1,125.2
1967 - 68	14	24	12,954.99	1,119.3
1958 - 59	15	25	12,865.78	1,111.6
1963 - 64	16	27	12,168.65	1,051.4
1969 - 70	17	29	11,909.12	1,028.9
1951 - 52	18	31	11,667.22	1,008.0
1999 - 2000	19	32	10,701.13	924.6
1986 - 87	20	34	10,322.67	891.9
1959 - 60	21	36	10,278.88	888.1
1984 - 85	22	37	10,259.52	886.4
1985 - 86	23	39	10,037.56	867.2
1962 - 63	24	41	9,451.93	816.6
1990 - 91	25	42	9,440.34	815.6
1975 - 76	26	44	8,352.30	721.6
1957 - 58	27	46	8,324.12	719.2
1993 - 94	28	47	6,993.16	604.2
1982 - 83	29	49	6,937.61	599.4
1970 - 71	30	51	6,928.54	598.6
2003 - 04	31	53	6,717.08	580.4
1971 - 72	32	54	6,547.64	565.7
1968 - 69	33	56	6,456.66	557.9
1965 - 66	34	58	6,377.01	551.0
1976 - 77	35	59	6,293.54	543.8
2002 - 03	36	61	6,209.26	536.5
1966 - 67	37	63	6,194.77	535.2
1972 - 73	38	64	6,164.41	532.6
1978 - 79	39	66	5,951.21	514.2
1983 - 84	40	68	5,730.03	495.1
2004 - 05	41	69	5,458.70	471.6
2001 - 02	42	71	5,211.38	450.3

Year	Rank	Percentage of Time (%)	Annual flow in cumecs day	Annual flow in million cubic metre (MCM)
1	2	3	4	5
1973 - 74	43	73	4,989.78	431.1
2005 - 06	44	75	4,779.01	412.9
1989 - 90	45	76	4,759.08	411.2
1980 - 81	46	78	4,724.41	408.2
1981 - 82	47	80	4,410.60	381.1
1995 - 96	48	81	4,301.68	371.7
1977 - 78	49	83	4,066.70	351.4
1974 - 75	50	85	4,061.78	350.9
1987 - 88	51	86	3,963.27	342.4
1979 - 80	52	88	3,008.61	259.9
1996 - 97	53	90	2,928.04	253.0
1988 - 89	54	92	2,798.97	241.8
1997 - 98	55	93	2,031.67	175.5
1994 - 95	56	95	1,961.60	169.5
1991 - 92	57	97	1,639.28	141.6
1992 - 93	58	98	1,533.20	132.5

- NOTE :
1. 75% dependable water availability year 2005 - 2006
 2. The 75% dependable annual flow = 412.90 MCM
 3. Irrigation year has been assumed to commence from 1st July every year to 30th June of next year.

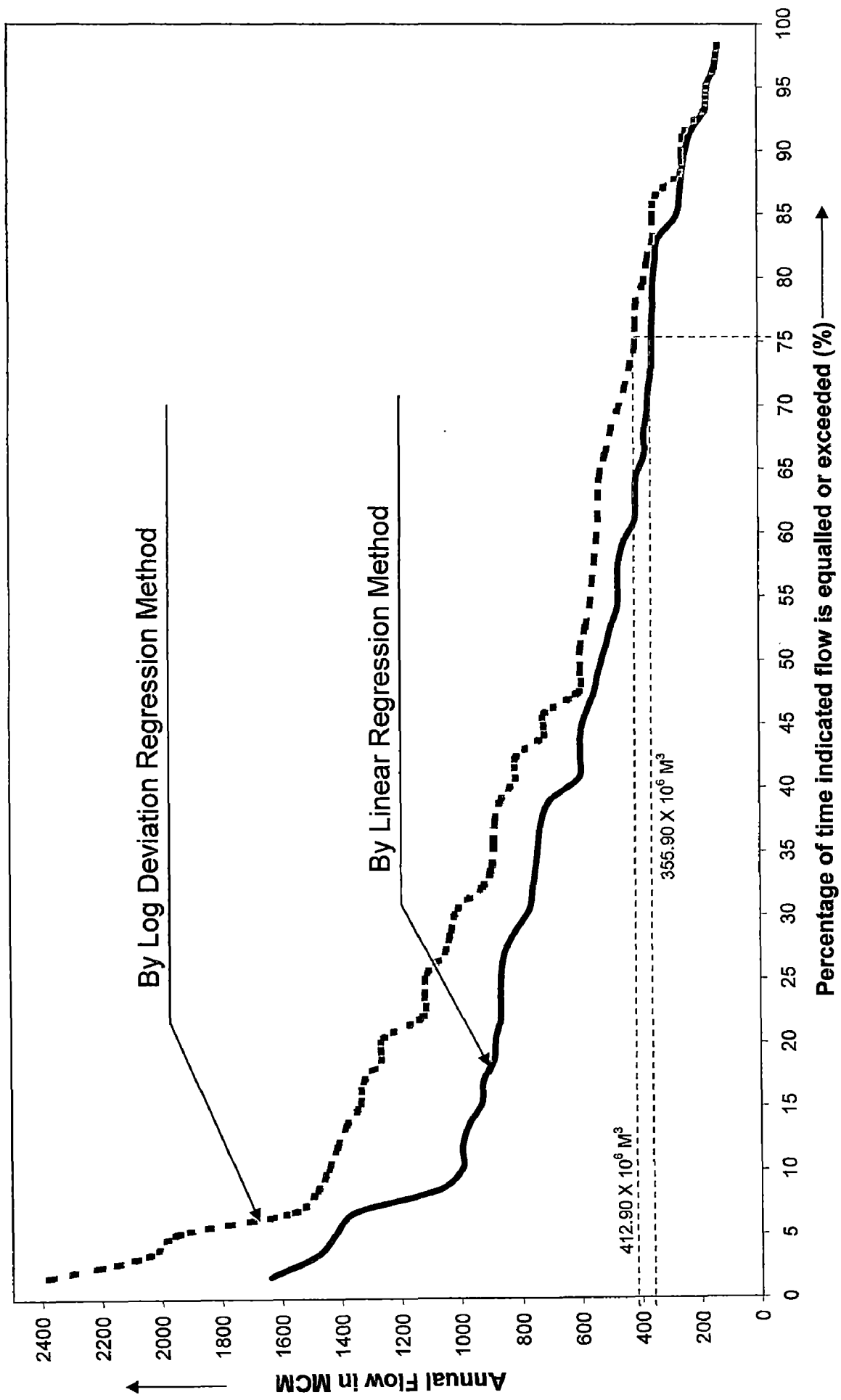


Fig. 4.3 FLOW DURATION CURVE AT JAMRANI DAM SITE



CHAPTER 5

DESIGN FLOOD ESTIMATION

5.1. DESIGN FLOOD

A Flood is an unusual high stage of a river that overflows the natural or man made banks spreading water to its flood plains that are thickly populated due to the obvious advantage of water supply and irrigation. It is possible to predict and contain a flood to reasonable extent with the forecasting models and technologies. The first information required to predict a flood at a particular place and time is the measurement of all the floods for maintaining a good record. Analysis of flood records gives an in-depth knowledge based on which flood prediction and protection measures can be carried out. However, for urban areas where catchments sizes are small, flood prediction may be carried out on empirical relations.

The Design Flood is a flood used for the design of a structure on considerations of its safety, economy, life expectancy and probable damage considerations.

The Design Flood is considered as inflow at the upstream of the structure which the structure should pass safely. The problem of computation of design flood for a structure is carried out under the following methods :

- | | |
|----------------------------|------------------------------|
| (1) For ungauged catchment | (2) For gauged catchment |
| (a) Rational approach | (a) Flood Frequency analysis |
| (b) Empirical equations | (b) Unit Hydrograph approach |
| (c) Envelope curves | |

Catchment Area for Jamrani Dam site is gauged catchment, so that methods that used to estimate Flood Design are Flood Frequency Analysis and Unit Hydrograph approach.

5.2. FLOOD FREQUENCY ANALYSIS

Flood Frequency Analysis considers the annual peak flows at a site for all the years. The method of analysis and predicting flood from the data of runoff peaks is called flood frequency analysis. It gives only the magnitude of flood peak of desired recurrence interval or return period, but doesn't provide information about the complete hydrograph or the flood volume.

In this study, methods of flood frequently analysis are used are :

1. Gumbel's extreme-value distribution,
2. Log-Pearson Type III distribution.

5.2.1. Gumbel's Method

Gumbel defined a flood as the largest of the 365 daily or 10-daily flows and the annual series of flood flows. Flows used are derived discharge data by using direct linear regression in Table-4.3. Following steps are used for estimating the flood illustrated in Table-5.1 and 5.1a.

- a). Maximum flood of every years in size N years are arranged in descending order, from the largest flood to the lowest flood.

Here the annual flood value is the variate X.

- b). Then find mean \bar{x} and standard deviation σ_{n-1} for the given data.

- c). Using Table-1.C and 2.C of Appendix C to determine \bar{y}_n and S_n appropriate to given N years.

For N = 58 years $\rightarrow \bar{y}_n = 0.5515$ and $S_n = 1.1721$

- d). See Table-5.1a to find y_T appropriate to a given T by equation

$$y_T = - \left[0.834 + 2.303 \log \log \frac{T}{T-1} \right].$$

- e). Find K appropriate to a given y_T by equation

$$K = \frac{y_T - \bar{y}_n}{S_n}$$

- f). Last, determine the value of the variate x with a return period T by

$$x_T = \bar{x} + K\sigma_{n-1}$$

TABLE-5.1

**GUMBEL'S METHOD CALCULATION OF
DERIVED DISCHARGE OF GOLA RIVER AT JAMRANI DAM SITE**

(DERIVED DISCHARGE DATA BY LINEAR REGRESSION)

Year	Max. flood (m ³ /s)	Order number m	Tp = (N+1)/m (years)
1948 - 49	5151.02	1	59.00
1951 - 52	4725.39	2	29.50
1950 - 51	3334.70	3	19.67
1985 - 86	3219.80	4	14.75
1993 - 94	3108.50	5	11.80
2000 - 01	2839.24	6	9.83
1949 - 50	2749.98	7	8.43
1963 - 64	2602.45	8	7.38
1955 - 56	2490.77	9	6.56
1953 - 54	2473.66	10	5.90
1952 - 53	2447.73	11	5.36
2005 - 06	2401.21	12	4.92
1990 - 91	2208.61	13	4.54
2004 - 05	2192.08	14	4.21
1962 - 63	2105.00	15	3.93
1967 - 68	2055.17	16	3.69
1995 - 96	1961.25	17	3.47
1956 - 57	1862.75	18	3.28
1983 - 84	1819.88	19	3.11
1998 - 99	1751.14	20	2.95
1964 - 65	1681.97	21	2.81
1969 - 70	1597.81	22	2.68
1982 - 83	1559.09	23	2.57
1954 - 55	1538.26	24	2.46
1961 - 62	1508.64	25	2.36
1999 - 2000	1451.84	26	2.27
1984 - 85	1395.48	27	2.19
1958 - 59	1328.05	28	2.11
1986 - 87	1253.21	29	2.03
1957 - 58	1170.22	30	1.97
2003 - 04	1148.40	31	1.90
2002 - 03	1115.17	32	1.84
1976 - 77	1093.25	33	1.79
1965 - 66	1058.30	34	1.74
1960 - 61	1051.26	35	1.69
1978 - 79	988.82	36	1.64
1987 - 88	957.00	37	1.59
1981 - 82	916.45	38	1.55
1975 - 76	874.07	39	1.51
1977 - 78	812.10	40	1.48
1972 - 73	787.05	41	1.44
1959 - 60	785.65	42	1.40
1966 - 67	783.76	43	1.37
1989 - 90	763.00	44	1.34
2001 - 02	749.21	45	1.31
1971 - 72	714.40	46	1.28
1970 - 71	638.50	47	1.26
1968 - 69	632.21	48	1.23

Year	Max. flood (m ³ /s)	Order number m	Tp = (N+1)/m (years)
1979 - 80	607.07	49	1.20
1996 - 97	520.97	50	1.18
1980 - 81	517.36	51	1.16
1974 - 75	389.33	52	1.13
1973 - 74	378.27	53	1.11
1988 - 89	340.50	54	1.09
1994 - 95	333.25	55	1.07
1992 - 93	265.94	56	1.05
1997 - 98	209.20	57	1.04
1991 - 92	182.16	58	1.02

$$\begin{aligned}
 \frac{N}{x} &= 58 \\
 \sigma_{n-1} &= 1510 \text{ m}^3/\text{s} \\
 \frac{Y_n}{S_n} &= 1055.2 \text{ m}^3/\text{s} \\
 &= 0.5515 \\
 &= 1.1721
 \end{aligned}$$

TABLE-5.1a
Results of Computation at Jamrani Dam Site by Gumbel's Method

T	Y _T	K	X _T
2.33			1510 m ³ /s
5	1.49994	0.81	2364 m ³ /s
10	2.25037	1.45	3040 m ³ /s
20	2.97020	2.06	3688 m ³ /s
50	3.90194	2.86	4527 m ³ /s
100	4.60015	3.45	5155 m ³ /s
150	5.00729	3.80	5522 m ³ /s
200	5.29581	4.05	5781 m ³ /s
500	6.21361	4.83	6608 m ³ /s
1000	6.90726	5.42	7232 m ³ /s

Note :

T_{2.33} = The Property of Gumbel's distribution for the average of the annual series

5.2.2. Graphical Method using Gumbel Probability Paper

The Gumbel probability paper is an aid for convenient graphical representation of Gumbel's distribution. Purpose of this method is to check the calculations of Gumbel's Method by a graphical means.

For such plottings, the recurrence interval is taken on abscissa and the event magnitudes as ordinate. The ordinate of Gumbel paper on which the value, x_T (flood discharge, maximum rainfall depth, etc) are plotted may have either an arithmetic scale or logarithmic scale whereas recurrence interval T years on arithmetic scale. Gumbel distribution will plot as a straight line on a Gumbel probability paper. This property can be used advantageously for graphical extrapolation, wherever necessary.

The following procedure may be adopted for graphical method using Gumbel probability paper.

- a). The value of x_T for some return periods $T < N = 58$ are calculated by using Gumbel's formula and plotted as x_T vs T on Gumbel probability paper. In this case, $T = 5, 10, 20,$ and 50 years are appropriate for in which $x_T = 2364, 3040, 3688, 4527$ cumecs respectively. These values are plotted on Gumbel's probability paper.
- b). Gumbel's distribution has the property which satisfied $T = 2.33$ years for the x_T is the average of the annual series when N is very large. With this rule, for $T = 2.33$ years, $x_T = \bar{x} = 1510$ cumecs. Thus, the value of a flood with $T = 2.33$ years is called the *mean annual flood*. In graphical plots this gives a mandatory point through which the line showing variation of x_T with T must pass.
- c). The use of Gumbel probability paper results in a straight line for x_T vs T plot. By extrapolation of the straight line x_T vs T , values of x_T for $T > N$ can be determined easily. In this study, values of x_T for $T > N = 58$ years are $T = 100, 150, 200, 500,$ and 1000 years. The corresponding value of x_T are $5250, 5600, 5900, 6800$ and 7500 cumecs respectively. Graphically, the results are shown in Fig.-5.1.

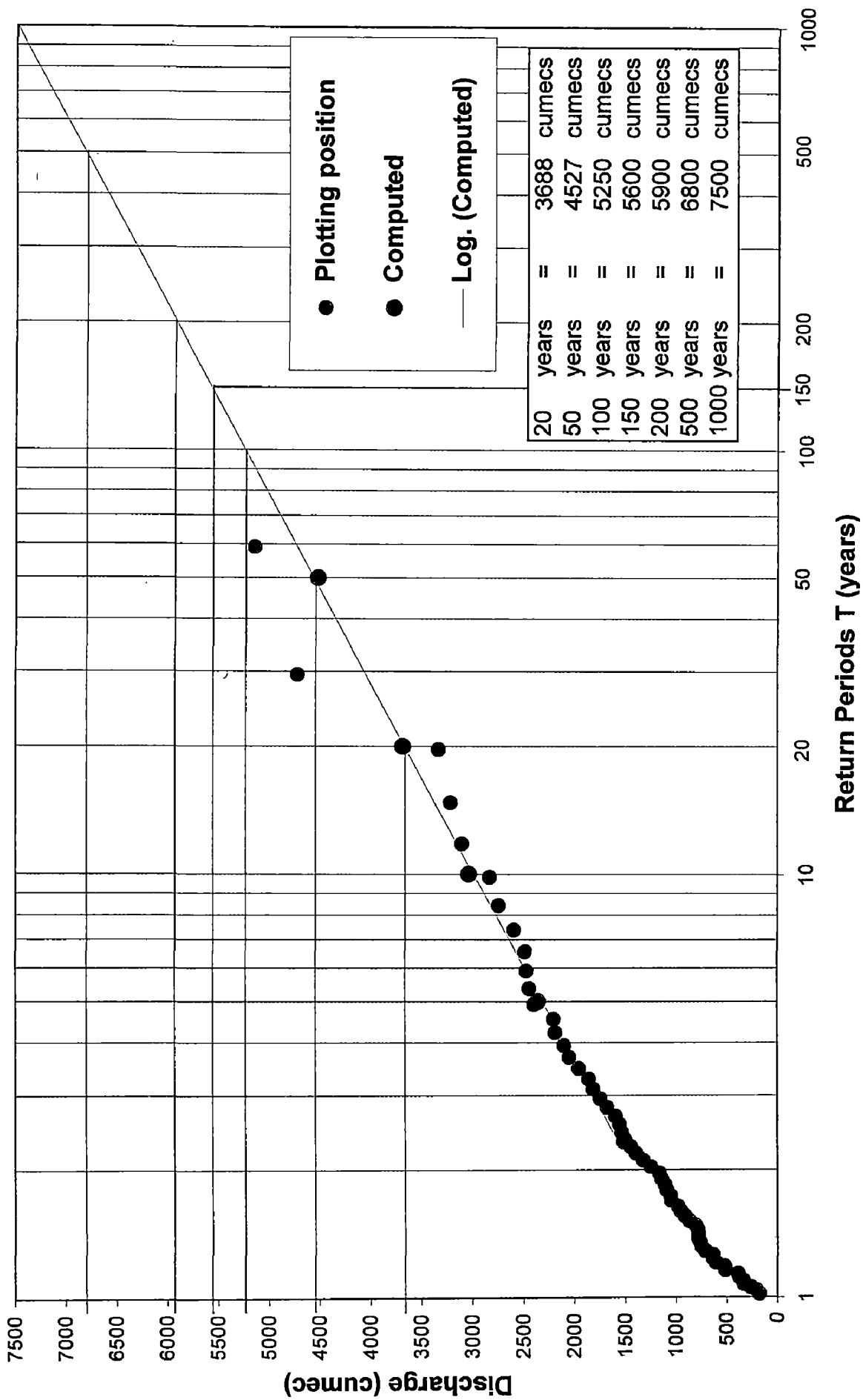


Fig. 5.1 GUMBEL GRAPHICAL METHOD FOR JAMRANI DAM

5.2.3. Log Pearson Type III Distribution

This distribution is widely used in United States, India and other countries as the standard distribution for flood frequency analysis of annual maximum floods. Stepwise procedure to fit Log-Pearson type III distribution to the observed series are outlined below.

- a). Transfer the observed data series to the logarithmic values. Generate x_i series as $z_i = \log_{10} x_i$ and the transferred data is then analysed.
- b). Find number of years of record (N), mean of the z values (\bar{z}), standard deviation (σ_z) and coefficient of skewness (C_s) for the log transferred series.

$$\sigma_z = \sqrt{\sum (z - \bar{z})^2 / (N - 1)} \quad \text{and} \quad C_s = \frac{N \sum (z - \bar{z})^3}{(N - 1)(N - 2)(\sigma_z)^3}$$

- c). To compute flood of required return period T , find frequency factor K_z from Table 3C of Appendix C corresponding to the skewness coefficient of the log transferred series.
- d). After get this K_z , find $z_T = \bar{z} + K_z \sigma_z$, for any recurrence interval T .
- e). By taking antilog of z_T , find x_T which is desired value of the event for the return period T . For more clear explanation of calculation, please refer to Table-5.2 and 5.2a.

TABLE-5.2

LOG PEARSON TYPE III DISTRIBUTION'S METHOD CALCULATION OF DERIVED DISCHARGE OF GOLA RIVER AT JAMRANI DAM SITE

(DERIVED DISCHARGE DATA BY LINEAR REGRESSION)

Year	Max. flood (cumecs) x	z = log x	(z - \bar{z})	(z - \bar{z}) ²	(z - \bar{z}) ³
1948 - 49	5151.02	3.7119	0.6416	0.4117	0.2642
1949 - 50	2749.98	3.4393	0.3691	0.1362	0.0503
1950 - 51	3334.70	3.5231	0.4528	0.2050	0.0928
1951 - 52	4725.39	3.6744	0.6042	0.3650	0.2206
1952 - 53	2447.73	3.3888	0.3185	0.1015	0.0323
1953 - 54	2473.66	3.3933	0.3231	0.1044	0.0337
1954 - 55	1538.26	3.1870	0.1168	0.0136	0.0016
1955 - 56	2490.77	3.3963	0.3261	0.1063	0.0347
1956 - 57	1862.75	3.2702	0.1999	0.0400	0.0080
1957 - 58	1170.22	3.0683	-0.0020	0.0000	0.0000
1958 - 59	1328.05	3.1232	0.0530	0.0028	0.0001
1959 - 60	785.65	2.8952	-0.1750	0.0306	-0.0054
1960 - 61	1051.26	3.0217	-0.0485	0.0024	-0.0001
1961 - 62	1508.64	3.1786	0.1083	0.0117	0.0013
1962 - 63	2105.00	3.3233	0.2530	0.0640	0.0162
1963 - 64	2602.45	3.4154	0.3451	0.1191	0.0411
1964 - 65	1681.97	3.2258	0.1556	0.0242	0.0038
1965 - 66	1058.30	3.0246	-0.0456	0.0021	-0.0001
1966 - 67	783.76	2.8942	-0.1761	0.0310	-0.0055
1967 - 68	2055.17	3.3128	0.2426	0.0589	0.0143
1968 - 69	632.21	2.8009	-0.2694	0.0726	-0.0196
1969 - 70	1597.81	3.2035	0.1333	0.0178	0.0024
1970 - 71	638.50	2.8052	-0.2651	0.0703	-0.0186
1971 - 72	714.40	2.8539	-0.2163	0.0468	-0.0101
1972 - 73	787.05	2.8960	-0.1742	0.0304	-0.0053
1973 - 74	378.27	2.5778	-0.4924	0.2425	-0.1194
1974 - 75	389.33	2.5903	-0.4799	0.2303	-0.1105
1975 - 76	874.07	2.9415	-0.1287	0.0166	-0.0021
1976 - 77	1093.25	3.0387	-0.0315	0.0010	0.0000
1977 - 78	812.10	2.9096	-0.1606	0.0258	-0.0041
1978 - 79	988.82	2.9951	-0.0751	0.0056	-0.0004
1979 - 80	607.07	2.7832	-0.2870	0.0824	-0.0236
1980 - 81	517.36	2.7138	-0.3565	0.1271	-0.0453
1981 - 82	916.45	2.9621	-0.1081	0.0117	-0.0013
1982 - 83	1559.09	3.1929	0.1226	0.0150	0.0018
1983 - 84	1819.88	3.2600	0.1898	0.0360	0.0068
1984 - 85	1395.48	3.1447	0.0745	0.0055	0.0004
1985 - 86	3219.80	3.5078	0.4376	0.1915	0.0838
1986 - 87	1253.21	3.0980	0.0278	0.0008	0.0000
1987 - 88	957.00	2.9809	-0.0893	0.0080	-0.0007
1988 - 89	340.50	2.5321	-0.5381	0.2896	-0.1558
1989 - 90	763.00	2.8825	-0.1877	0.0352	-0.0066
1990 - 91	2208.61	3.3441	0.2739	0.0750	0.0205
1991 - 92	182.16	2.2605	-0.8098	0.6558	-0.5310
1992 - 93	265.94	2.4248	-0.6455	0.4166	-0.2689
1993 - 94	3108.50	3.4926	0.4223	0.1783	0.0753
1994 - 95	333.25	2.5228	-0.5475	0.2997	-0.1641

Year	Max. flood (cumecs) x	z = log x	(z - \bar{z})	(z - \bar{z}) ²	(z - \bar{z}) ³
1995 - 96	1961.25	3.2925	0.2223	0.0494	0.0110
1996 - 97	520.97	2.7168	-0.3534	0.1249	-0.0442
1997 - 98	209.20	2.3206	-0.7497	0.5620	-0.4214
1998 - 99	1751.14	3.2433	0.1731	0.0300	0.0052
1999 - 2000	1451.84	3.1619	0.0917	0.0084	0.0008
2000 - 01	2839.24	3.4532	0.3830	0.1467	0.0562
2001 - 02	749.21	2.8746	-0.1956	0.0383	-0.0075
2002 - 03	1115.17	3.0473	-0.0229	0.0005	0.0000
2003 - 04	1148.40	3.0601	-0.0102	0.0001	0.0000
2004 - 05	2192.08	3.3409	0.2706	0.0732	0.0198
2005 - 06	2401.21	3.3804	0.3102	0.0962	0.0298
TOTAL		178.0746		6.1482	-0.8430

$$\begin{aligned}
 N &= 58 \\
 \bar{z} &= 3.0703 \\
 \sigma_z &= 0.3284 \\
 C_s &= -0.432
 \end{aligned}$$

		Cs		
		-0.4	-0.432	-0.5
Return Period (T)	10	1.231	1.226	1.216
	20	1.382	1.373	1.356
	50	1.834	1.816	1.777
	100	2.029	2.005	1.955
	150	2.115	2.088	2.0315
	200	2.201	2.171	2.108
	500	2.328	2.292	2.218
	1000	2.540	2.495	2.400

☐ = these values are from interpolated

TABLE-5.2a
Results of Computation at Jamrani Dam Site by Log Pearson type-III Distribution

Return Period T (years)	K _z from table Cs = -0.432	K _z · σ_z	Z _T = $\bar{z} + K_z \cdot \sigma_z$	x _T = antilog Z _T (cumecs)
20	1.373	0.4511	3.5213	3322
50	1.816	0.5963	3.6665	4640
100	2.005	0.6585	3.7288	5355
150	2.088	0.6857	3.7560	5701
200	2.171	0.7130	3.7832	6070
500	2.292	0.7528	3.8231	6654
1000	2.495	0.8193	3.8896	7755

5.3. Unit Hydrograph Principle

In the early years, the design flood were calculated by well known empirical formulae viz : Dickens, Ryves, Inglis and Ali Nawaz Jung etc. In these formulae, flood discharge is related to catchment area only and all other factors are included in a constant which is to be decided by the designer from his experience. Even intensity of the storm rainfall which is a prime factor responsible for the flood and which varies substantially from place to place is not indicated in the above formulae. The need to evolve a method on estimation of design flood peak of desired frequency knowing the physical characteristics of the catchments and design rainfall has been recognised and a committee of engineers under the Chairmanship of Dr. A.N. Khosla have recommended Systematic and sustained collection of hydro-meteorological data of selected catchments in different climatic zones of India for evolution of a rational approach for determination of flood discharges. The committee felt that design discharge should be maximum flood on record for a period not less than 50 years. Where adequate records are available extending over a period of not less than 50 years, the design flood should be 50 year flood determined from probability curve on the basis of recorded floods during the period. In case where the requisite data, as above are not available, the design flood should be decided based on the ground and meteorological characteristics obtained on the basis of design storm. In this study, a method has been based on unit hydrograph principle to derive the design flood.

The various steps necessary to estimate the design flood / design flood hydrograph are as under :

- 1). Preparation of catchment area plan of the catchment in question.
- 2). Determination of physiographic parameters viz; the catchment area (A), the length of the longest stream (L) and equivalent stream slope (S), see Fig. 1C of Appendix-C.
- 3). Determination of 2-hr synthetic unit hydrograph parameters (for the specified unit duration) i.e. the unit peak discharge (q_p), the peak discharge (Q_p), the basin lag (t_p), the period of U.H. (T_m), widths of the U.H. at 50% and 75% of Q_p (W_{50} and W_{75}), widths of the rising limb of

U.H. at 50% and 75% of Q_p (W_{R50} and W_{R75}) and time base of U.H. (T_B), see Fig. 2C of Appendix-C.

- 4). Preparation of a synthetic unit hydrograph.
- 5). Estimation of design storm duration (T_D).
- 6). Estimation of point rainfall and areal rainfall for design storm duration (T_D).
- 7). Distribution of areal rainfall during design storm duration (T_D) to obtain rainfall increments for unit duration intervals.
- 8). Estimation of rainfall excess units after subtraction of design loss rate from rainfall increments.
- 9). Estimation of base flow.
- 10). Computation of design flood peak.
- 11). Computation of design flood hydrograph.

The details of the calculation are as follows :

FLOOD ESTIMATION BY USING UNIT HYDROGRAPH METHOD FOR 50-YEARS RETURN PERIOD

Step 1 Preparation of Cathment Area Plan

A Cathment Area Plan (Fig. 3.4) showing main rivers was prepared.

Step 2 Determination of Physiographic Parameters

The following physiographic parameters were determined from the catchment area plan & long-section of Gola river (Fig. 3.5).

1. $A =$.450 Km² (known)
2. $L =$ 23.5 Kms (known)
3. $S =$ 13.16 m/Km (get from the below computation)
4. $L/\sqrt{S} =$ 6.48

TABLE-5.3 CALCULATION OF EQUIVALENT STREAM SLOPE (S)

Segment No.	Distance from gauging site (Kms)	Bed Level of River (m)	Length of each segment L_i (Km)	Height above datum D_i (m)	$(D_{i-1} + D_i)$ (m)	$L_i \cdot (D_{i-1} + D_i)$
1	0.0	509.605	Datum 0.00	0.00	0.00	0.00
2	0.5	513.735	0.50	4.13	4.13	2.07
3	1.0	519.915	0.50	10.31	14.44	7.22
4	1.5	526.215	0.50	16.61	26.92	13.46
5	2.0	531.395	0.50	21.79	38.40	19.20
6	2.5	537.475	0.50	27.87	49.66	24.83
7	3.0	542.110	0.50	32.51	60.38	30.19
8	3.5	548.460	0.50	38.86	71.36	35.68
9	4.0	555.970	0.50	46.37	85.22	42.61
10	4.5	560.975	0.50	51.37	97.74	48.87
11	5.0	567.675	0.50	58.07	109.44	54.72
12	5.5	574.035	0.50	64.43	122.50	61.25
13	6.0	578.730	0.50	69.13	133.56	66.78
14	6.5	583.320	0.50	73.72	142.84	71.42
15	7.0	589.885	0.50	80.28	154.00	77.00
16	7.5	598.145	0.50	88.54	168.82	84.41
17	8.0	604.840	0.50	95.24	183.78	91.89
18	8.5	611.965	0.50	102.36	197.60	98.80
19	9.0	619.875	0.50	110.27	212.63	106.32
20	9.5	624.135	0.50	114.53	224.80	112.40
21	10.0	631.335	0.50	121.73	236.26	118.13
22	10.5	637.955	0.50	128.35	250.08	125.04
23	11.0	643.830	0.50	134.23	262.58	131.29
24	11.5	650.820	0.50	141.22	275.44	137.72
25	12.0	659.530	0.50	149.93	291.14	145.57
26	12.5	669.660	0.50	160.06	309.98	154.99
27	13.0	680.400	0.50	170.80	330.85	165.43
28	13.5	685.030	0.50	175.43	346.22	173.11
29	14.0	692.840	0.50	183.24	358.66	179.33
30	14.5	700.800	0.50	191.20	374.43	187.22
31	15.0	707.905	0.50	198.30	389.50	194.75
32	15.5	714.855	0.50	205.25	403.55	201.78
33	16.0	721.470	0.50	211.87	417.12	208.56
34	16.5	728.110	0.50	218.51	430.37	215.19
35	17.0	740.030	0.50	230.43	448.93	224.47
36	17.5	748.815	0.50	239.21	469.64	234.82
37	18.0	757.150	0.50	247.55	486.76	243.38
38	18.5	762.995	0.50	253.39	500.94	250.47
39	19.0	770.935	0.50	261.33	514.72	257.36

Segment No.	Distance from gauging site (Kms)	Bed Level of River (m)	Length of each segment L_i (Km)	Height above datum D_i (m)	$(D_{i-1} + D_i)$ (m)	$L_i \cdot (D_{i-1} + D_i)$
40	19.5	778.680	0.50	269.08	530.41	265.20
41	20.0	782.620	0.50	273.02	542.09	271.05
42	20.5	789.160	0.50	279.56	552.57	276.29
43	21.0	797.270	0.50	287.67	567.22	283.61
44	21.5	806.520	0.50	296.92	584.58	292.29
45	22.0	816.000	0.50	306.40	603.31	301.66
46	22.5	827.710	0.50	318.11	624.50	312.25
47	23.0	841.330	0.50	331.73	649.83	324.92
48	23.5	858.070	0.50	348.47	680.19	340.10
total			23.50			7265.01

$$S = \frac{\sum L_i (D_{i-1} + D_i)}{L^2} = \frac{7265.01}{552.25} = 13.16 \text{ m/Km}$$

Step 3 Determination of Synthetic Unit Hydrograph (SUH) parameters

2-hr UH is UH produced 1 cm RE in 2 hour duration

2-hr UH parameters may be found out by using one of the following approaches :

- by using the synthetic relations
- by using coaxial diagram

i. By using the synthetic relations

q_p	$= 2.03 / (L / \sqrt{S})^{0.646}$	=	0.604	≈	0.60 cumec/Km ²
Q_p	$= q_p \times CA$	=	270.00	≈	270.00 cumecs
t_p	$= 1.858 / (q_p)^{1.038}$	=	3.16	≈	3.00 hours
T_m	$= t_p + (t_r / 2)$	=	4	≈	4.00 hours
W_{50}	$= 2.217 / (q_p)^{0.99}$	=	3.68	≈	3.70 hours
W_{75}	$= 1.477 / (q_p)^{0.876}$	=	2.31	≈	2.30 hours
WR_{50}	$= 0.812 / (q_p)^{0.907}$	=	1.29	≈	1.30 hours
WR_{75}	$= 0.606 / (q_p)^{0.791}$	=	0.91	≈	0.90 hours
T_B	$= 7.744 \times (t_p)^{0.779}$	=	18.22	≈	18.00 hours

t_r (unit duration) 2 hours

ii. By using coaxial diagram

The synthetic relations have been transferred on one graph sheet in Fig.-3C of Appendix C in such a way that with the known values of L , S , L / \sqrt{S} , the values of q_p can be found out.

Similarly with the known value of q_p , the other parameters like t_p , W_{50} , W_{75} , WR_{50} , WR_{75} can be directly read from the respective relation (curves).

Again with the known value of t_p , the value T_B is read from the respective relation.

The values so obtained are similar to the calculated are as shown above.

Step 4 Preparation of 2-hr Synthetic Unit Hydrograph

The parameters got in Step-3 were plotted to scale on a graph paper as shown in Fig.-5.2.

The points were joined to fit a trial synthetic Unit Hydrograph.

By definition, the volume of the unit hydrograph must be equivalent to 1.0 cm depth of direct runoff over the entire catchment (A) in Km².

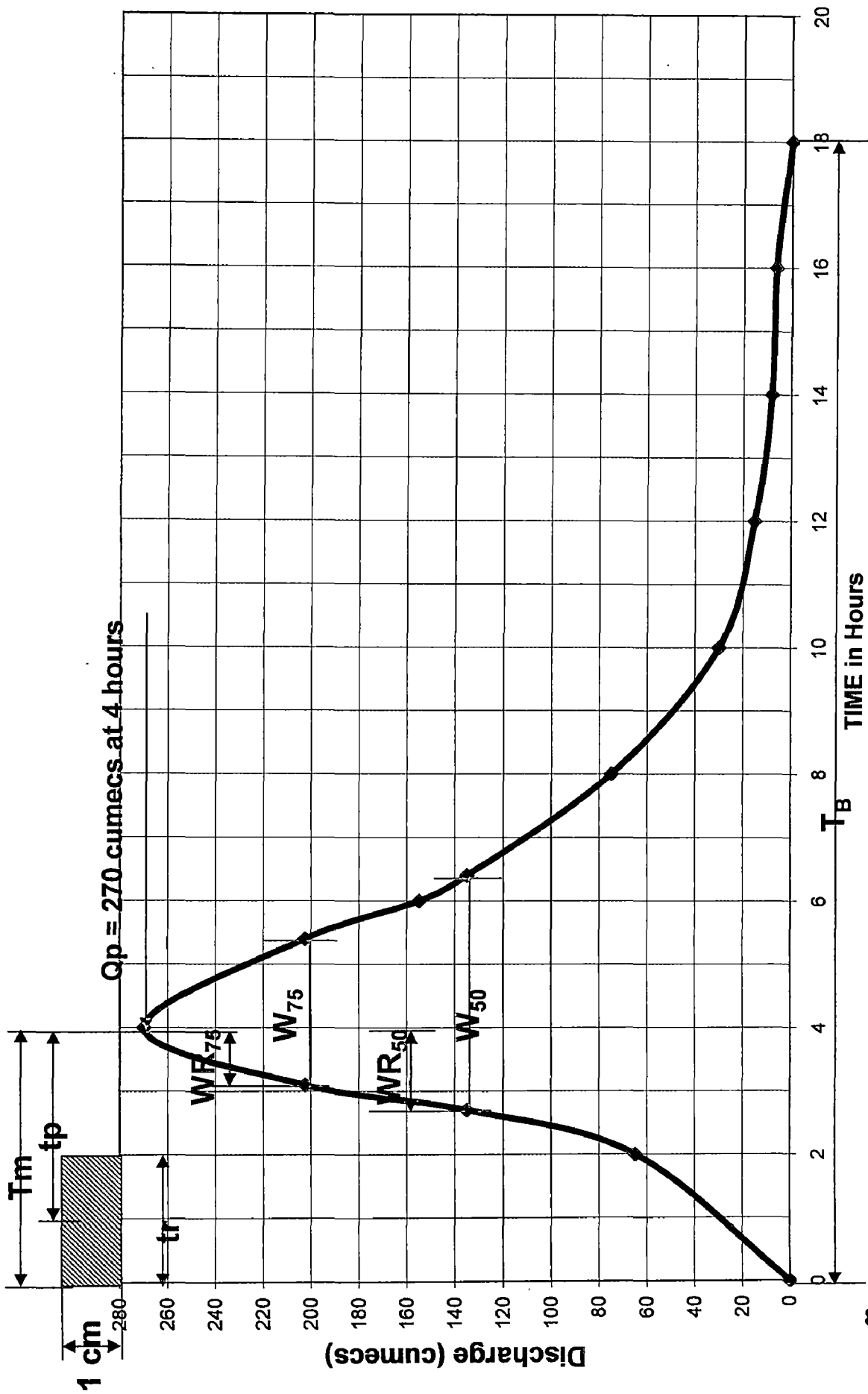


Fig.-5.2 2 HRS UNIT HYDROGRAPH

Step 5 Estimation of Design Storm duration

The design storm duration is

$$T_D = 1.1 \times t_p = 1.1 \times 3 = 3.30 \text{ hrs} \approx 4.00 \text{ hours}$$

Adjusting the design storm duration to nearest even hour, adopted design storm duration is 4 hrs

Step 6 Estimation of Point Rainfall and Areal Rainfall

Point rainfall estimated for 50-year return period for duration of 24-hr = 44 cm from isopluvial map reading in Fig.-4C of Appendix C

Design storm duration (T_D) for the catchment = 4 hrs. get value of ratio = 0.607 from Fig-6C of Appendix C

50-yr, 4-hr point rainfall = $44 \times 0.607 = 26.708 \text{ cm}$

Area = 450 Km^2 and 4-hr point rainfall, we get areal reduction factor = 0.76 from Table-4C of Appendix C

So, above point rainfall = $0.76 \times 26.708 = 20.30 \text{ cm}$

Step 7 Time distribution of Areal Rainfall

Areal rainfall estimated for 50-yr Return Period for a Design storm duration of 4-hrs got in Step 6 above was distributed to give 2-hrs gross rainfall units as shown in the following table by using Fig. 7C of Appendix C.

TABLE-5.4

Hr	%age of storm duration	cumulative % of total rainfall	cumulative rainfall	2-hr gross rainfall increments
1	2	3	4	5
0	0.00	0	0.00	0.00
2	50.00	82	16.64	16.64
4	100.00	100	20.30	3.65
total				20.30

Step 8 Estimation of Rainfall Excess Units

Col.5 of the Table-5.4 in Step-7 gives the 2 hourly gross rainfall units.

Design value of loss rate of 0.3 cm/hr is subtracted from each of the units to give the rainfall excess units.

For 2-hrs, the loss rate is to be subtracted = $2 \times 0.3 = 0.6 \text{ cm}$

The table below illustrates the procedure for calculation of rainfall excess units :

TABLE-5.5

Hr	Gross rainfall (col.5) of Step 7	Loss / 2-hrs cm	2-hr Rainfall excess (cm)
0	-	-	-
2	16.64	0.60	16.04
4	3.65	0.60	3.05

Step 9 Estimation of Base Flow

Model value of base flow for this subzone = 0.05 cumecs/Km²

So, total base flow for the catchment of 450 Km² = 0.05 x 450 = 22.5 cumecs
 However, from flood of 1986, the base flow is 35.48 cumec, hence the adopted base flow is **35.48** cumec.

Step 10 Estimation of Design Flood (peak only)

For estimation of the peak discharge, the rainfall excess units have to be re-arranged against the unit hydrograph ordinates such that maximum rainfall excess is placed against the maximum unit hydrograph ordinate the next lower value of rainfall excess comes against the next lower value of unit hydrograph ordinate and so on.

In this case, the maximum peak discharge ordinate of UH was occurring at 4th hour. The UH ordinates at 2-hr interval from the peak hour of 4th were read from Fig.5.2 of 2-hr UH near the neighbourhood of the peak and tabulated as under. The maximum 2-hourly rainfall excess unit was placed against the peak discharge of UH. Likewise, the next lower rainfall excess unit was placed against the next lower UH ordinate in the following table and so on. Summation of the products of cols (2) and (3) gives the total direct runoff to which base flow is added to get the total peak discharge.

TABLE-5.6

Time Hr	SUH ordinates (cumecs)	2-hr Rainfall excess (cm) (after rearranged)	Direct Runoff (cumecs)
1	2	3	4 = (2) x (3)
2	65.0	3.05	198.49
4	270.0	16.04	4331.99
Total			4530.48
Add Base Flow from Step-9			35.48
Total Peak Discharge			4565.96

Step 11 2-hr Rainfall Excess sequence

The 2-hour rainfall excess sequence shown in col.3 of Table-5.6 in Step 10 was reversed to obtain critical sequence as shown below :

TABLE-5.7

Time Hr	Critical 2-hr rainfall excess sequence
2	16.04
4	3.05

**Table-5.8 COMPUTATION OF DESIGN FLOOD HYDROGRAPH
FOR 50 YEARS RETURN PERIOD**

Time (hours)	SUH ordinates (cumecs)	2-hourly rainfall excess (cm)		Total (cumecs)	Base Flow (cumecs)	Flood Hydrograph (cumecs)	Remark
		16.04	3.05				
0	0.00	0.00		0.00	35.48	35.48	Peak Flood
2	65.00	1042.89	0.00	1042.89	35.48	1078.37	
4	270.00	4331.99	198.49	4530.48	35.48	4565.96	
6	155.00	2486.89	824.49	3311.37	35.48	3346.85	
8	75.00	1203.33	473.32	1676.65	35.48	1712.13	
10	30.00	481.33	229.02	710.36	35.48	745.84	
12	15.25	244.68	91.61	336.29	35.48	371.77	
14	8.25	132.37	46.57	178.93	35.48	214.41	
16	6.50	104.29	25.19	129.48	35.48	164.96	
18	0.00	0.00	19.85	19.85	35.48	55.33	
20			0.00	0.00	35.48	35.48	

Result : Design Flood for 50 Years is 4566 cumecs

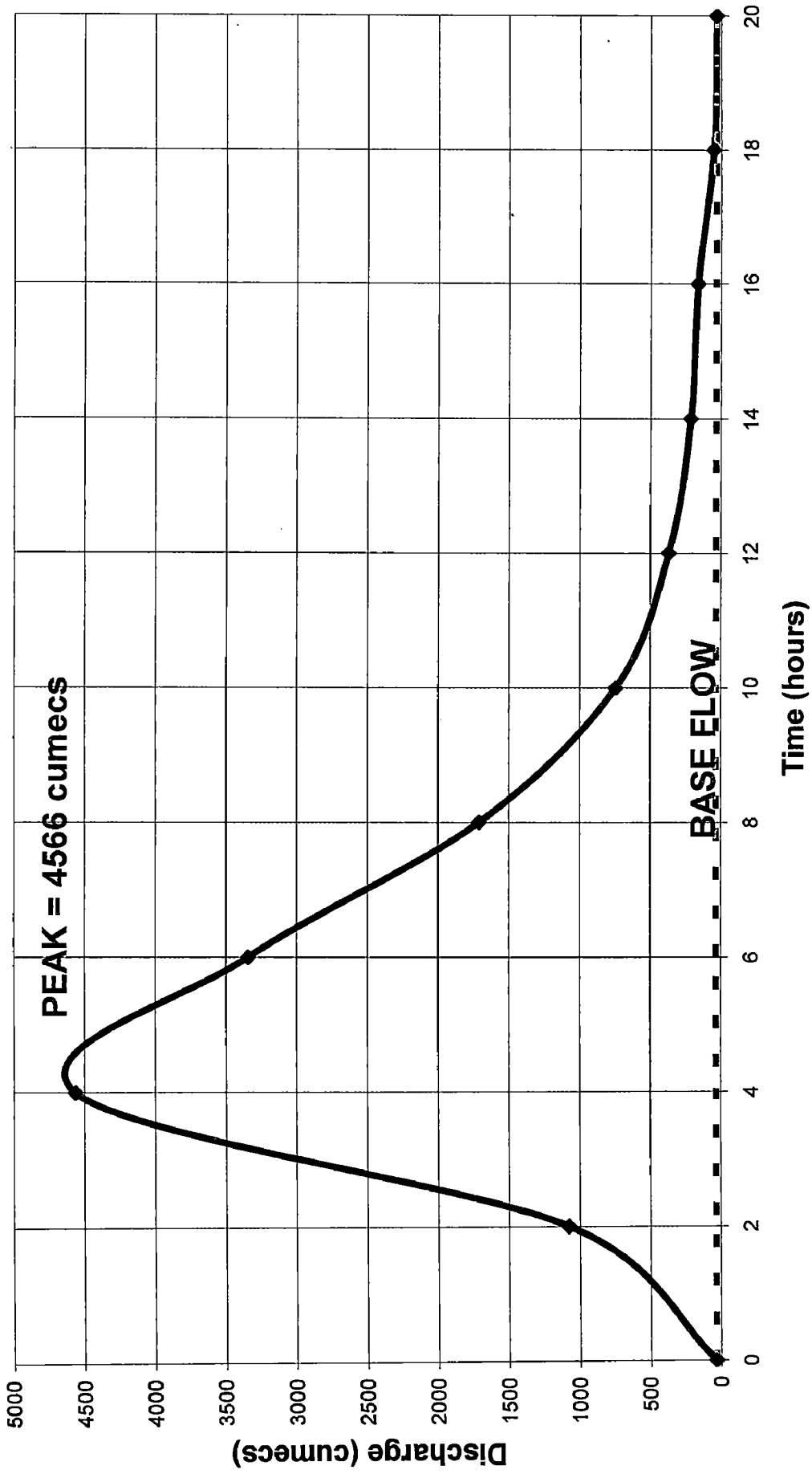


Fig.-5.3 DESIGN FLOOD HYDROGRAPH - 50 years

FLOOD ESTIMATION BY USING UNIT HYDROGRAPH METHOD FOR 100-YEARS RETURN PERIOD

Step 1 to Step 5 are same with Flood Estimation for 50 Years Return Period.

Step 6 Estimation of Point Rainfall and Areal Rainfall

Point rainfall estimated for 100-year return period for duration of 24-hr = 48 cm from isopluvial map reading in Fig.-5C of Appendix C

Design storm duration (T_D) for the catchment = 4 hrs. get value of ratio = 0.607
 from Conversion ratio = (100-yr 4-hr rainfall / 100-yr 24-hr rainfall) in section 4.2 page 61 of Flood Estimation Report for Upper Indo-Ganga Plains (sub zone-1e).

100-yr, 4-hr point rainfall = $48 \times 0.607 = 29.136$ cm

Area = 450 Km^2 and 4-hr point rainfall, we get areal reduction factor = 0.76 from Table-4C of Appendix C

So, above point rainfall = $0.76 \times 29.136 = 22.14$ cm

Step 7 Time distribution of Areal Rainfall

Areal rainfall estimated for 100-yr Return Period for a Design storm duration of 4-hrs got in Step 6 above was distributed to give 2-hrs gross rainfall units as shown in the following table by using Fig. 7C of Appendix C.

TABLE-5.9

Hr	%age of storm duration	cumulative % of total rainfall	cumulative rainfall	2-hr gross rainfall increments
1	2	3	4	5
0	0.00	0	0.00	0.00
2	50.00	82	18.16	18.16
4	100.00	100	22.14	3.99
total				22.14

Step 8 Estimation of Rainfall Excess Units

Col.5 of the Table-5.4 in Step-7 gives the 2 hourly gross rainfall units.

Design value of loss rate of 0.3 cm/hr is subtracted from each of the units to give the rainfall excess units.

For 2-hrs, the loss rate is to be subtracted = $2 \times 0.3 = 0.6$ cm

The table below illustrates the procedure for calculation of rainfall excess units :

TABLE-5.10

Hr	Gross rainfall (col.5) of Step 7	Loss / 2-hrs cm	2-hr Rainfall excess (cm)
0	-	-	-
2	18.16	0.60	17.56
4	3.99	0.60	3.39

Step 9 Estimation of Base Flow

Model value of base flow for this subzone = 0.05 cumecs/Km²

So, total base flow for the catchment of 450 Km² = 0.05 x 450 = 22.5 cumecs
 However, from flood of 1986, the base flow is 35.48 cumec, hence the adopted base flow is **35.48** cumec.

Step 10 Estimation of Design Flood (peak only)

For estimation of the peak discharge, the rainfall excess units have to be re-arranged against the unit hydrograph ordinates such that maximum rainfall excess is placed against the maximum unit hydrograph ordinate the next lower value of rainfall excess comes against the next lower value of unit hydrograph ordinate and so on.

In this case, the maximum peak discharge ordinate of UH was occurring at 4th hour. The UH ordinates at 2-hr interval from the peak hour of 4th were read from Fig.5.2 of 2-hr UH near the neighbourhood of the peak and tabulated as under. The maximum 2-hourly rainfall excess unit was placed against the peak discharge of UH. Likewise, the next lower rainfall excess unit was placed against the next lower UH ordinate in the following table and so on. Summation of the products of cols (2) and (3) gives the total direct runoff to which base flow is added to get the total peak discharge.

TABLE-5.11

Time Hr	SUH ordinates (cumecs)	2-hr Rainfall excess (cm) (after rearranged)	Direct Runoff (cumecs)
1	2	3	4 = (2) x (3)
2	65.0	3.39	220.08
4	270.0	17.56	4740.54
Total			4960.62
Add Base Flow from Step-9			35.48
Total Peak Discharge			4996.10

Step 11 2-hr Rainfall Excess sequence

The 2-hour rainfall excess sequence shown in col.3 of Table-5.6 in Step 10 was reversed to obtain critical sequence as shown below :

TABLE-5.12

Time Hr	Critical 2-hr rainfall excess sequence
2	17.56
4	3.39

**Table-5.13 COMPUTATION OF DESIGN FLOOD HYDROGRAPH
FOR 100 YEARS RETURN PERIOD**

Time (hours)	SUH ordinates (cumecs)	2-hourly rainfall excess (cm)		Total (cumecs)	Base Flow (cumecs)	Flood Hydrograph (cumecs)	Remark
		17.56	3.39				
0	0.00	0.00		0.00	35.48	35.48	Peak Flood
2	65.00	1141.24	0.00	1141.24	35.48	1176.72	
4	270.00	4740.54	220.08	4960.62	35.48	4996.10	
6	155.00	2721.42	914.17	3635.59	35.48	3671.07	
8	75.00	1316.82	524.80	1841.62	35.48	1877.10	
10	30.00	526.73	253.94	780.66	35.48	816.14	
12	15.25	267.75	101.57	369.33	35.48	404.81	
14	8.25	144.85	51.63	196.48	35.48	231.96	
16	6.50	114.12	27.93	142.06	35.48	177.54	
18	0.00	0.00	22.01	22.01	35.48	57.49	
20			0.00	0.00	35.48	35.48	

Result : Design Flood for 100 Years is 4996 cumecs

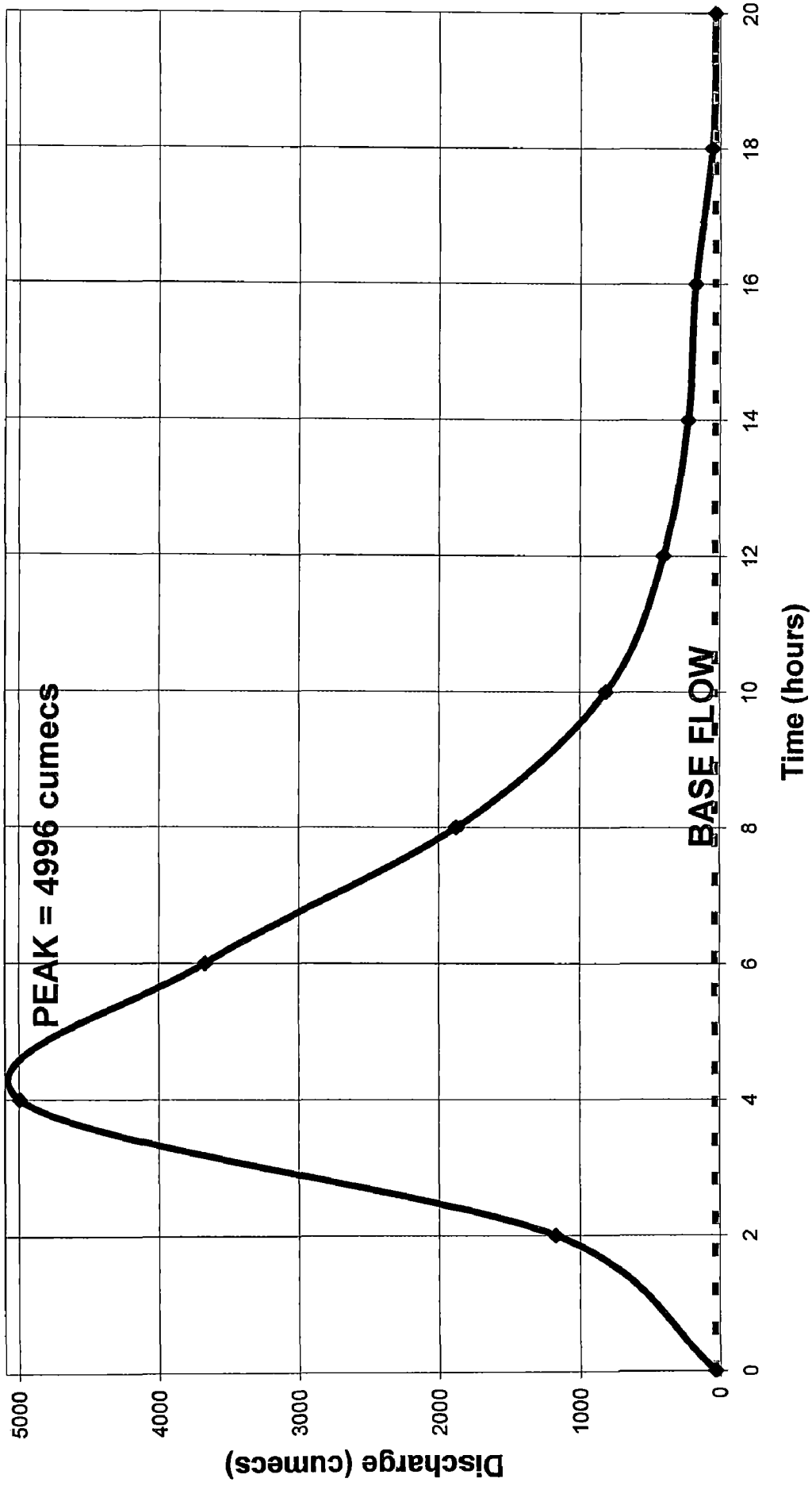


Fig.-5.4 DESIGN FLOOD HYDROGRAPH - 100 Years



CHAPTER 6

SEDIMENTATION OF RESERVOIR

6.1. INTRODUCTION

The development of water resources involves the construction of storage reservoir for storing water during the floods so that it may be used during periods of low supply for various purposes such as power generation, irrigation, domestic supply etc. The ultimate destiny of all reservoirs is to get filled with sediment. If the sediment inflow is large in comparison to the reservoir capacity, the useful life of the reservoir may be very short.

Sediments are the fragmented materials, originated either due to chemical or physical disintegration of rocks. Sediments are transported by the water and air. The size of sediment may vary from big boulders to colloidal. Amongst various agents causing transportation of sediments, the flowing water is one of the main. Therefore, sediment transported by water is generally considered in general view.

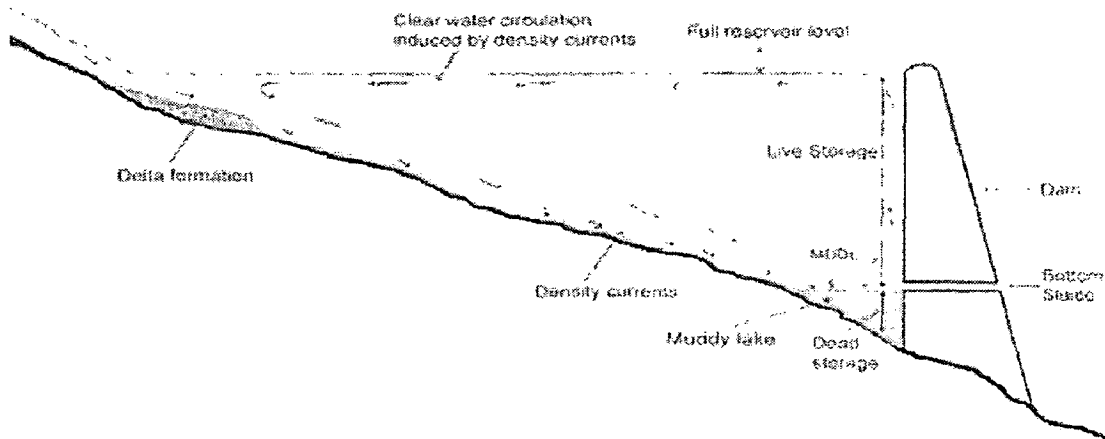


Fig. 6.1 Sediment deposit in a typical section of reservoir

Every river carries certain amount of sediment load. The sediment particles try to settle down to the river bottom due to the gravitational force, but may be kept in suspension due to the upward currents in the turbulent flow which may overcome the gravity force. Due to these reasons, the river carries

fine sediment in suspension as suspended load, and larger solids along the river bed as bed load. When the silt laden water reaches a reservoir in the vicinity of a dam, the velocity and the turbulence are considerably reduced. The bigger suspended particles and most of the bed load, therefore, gets deposited in the head reaches of the reservoir. Fine particles may travel some more distance and may finally deposit farther down in the reservoir, as shown in Fig. 6.1. Some very fine particles may remain in suspension for much longer period, and may finally escape from the dam along with the water discharged through the sluiceways, turbines, spillway, etc.

The deposition of sediment in the reservoir is known as Reservoir Sedimentation. The deposition of the sediment will automatically reduce the water storing capacity of the reservoir, and if this process of deposition continues longer, a stage is likely to reach when the whole reservoir may get silted up and become useless.

6.2. RESERVOIR AND CATCHMENT CHARACTERISTICS

The reservoir of the Jamrani dam will spread between elevation 635 and 761.3 m for a distance of 9 km along Gola river and for approximately 4.5 km in the Lugar-gad. In the reservoir area, there are only three slides. These occur on the left bank of Gola river varying in width from 120 m to 168 m at the river level and in a height from 50 to 61 m. Two of these are in rock slides while the third is the debris slide. All the three slides are in granite prophyey. Since these slides are of small dimension and do not appear to be deep there would be no stability problem. The lithology of the entire catchment area has been compiled from the geological maps. The catchment is well dissected by numerous streams and the main drainages are Kalsa river, Gola river and Lugargad. The relief varies between elevation 635 and 2296 m.

6.3. SEDIMENTATION DATA

Available Sediment data observed at a proposed Jamrani Dam site from 1985 to 2006, observed from 15 June until 15 October during Monsoon season. Those sediment data observed as shown in Table-6.1.

Average sediment at Jamrani Dam site is shown in Fig.6.2.

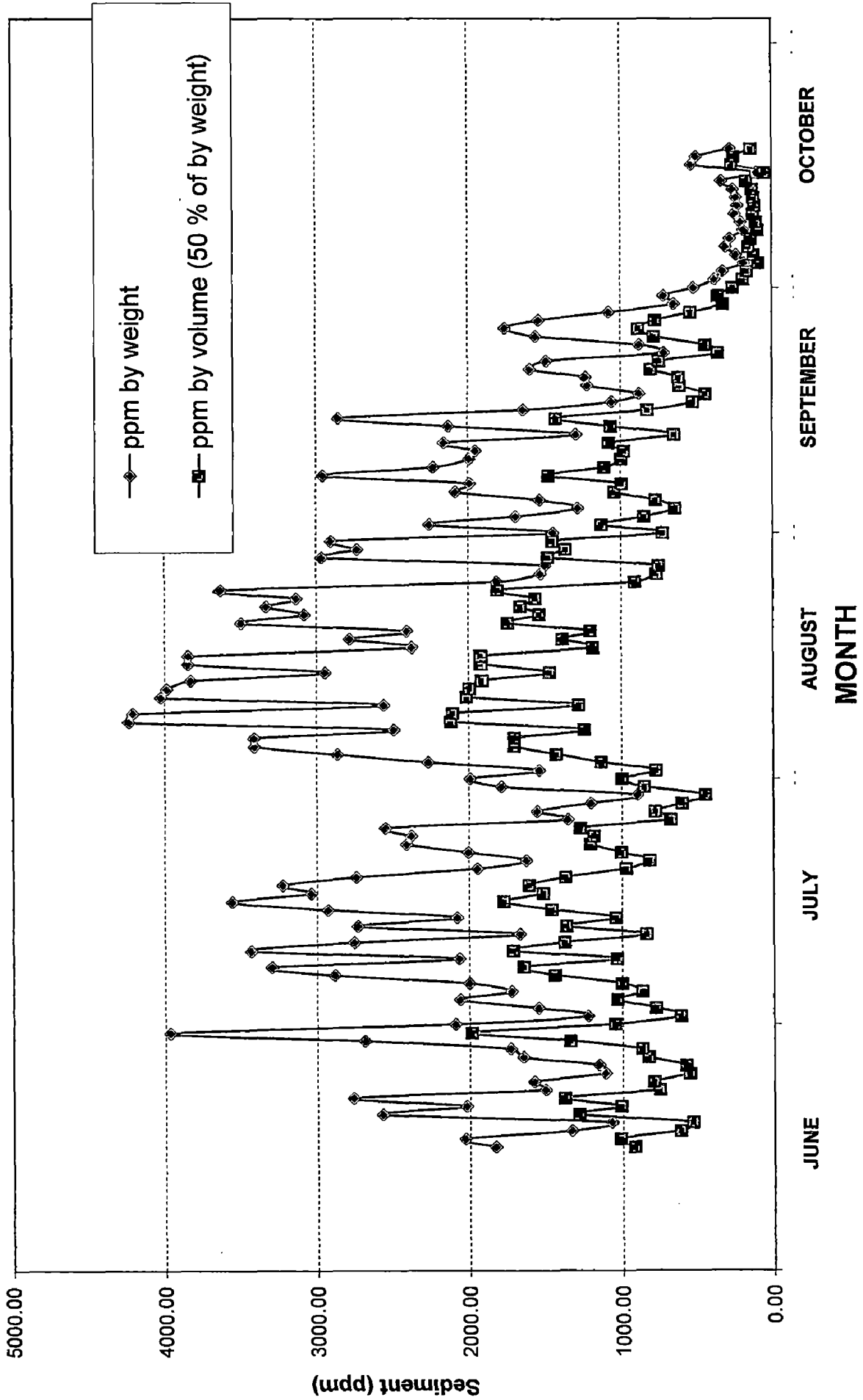
In estimating sediment rate occurred at Jamrani Dam site, relationships were made between 10-daily observed discharge at Dam site and 10-daily observed sediment at Dam site as shown in Table-6.2 and its calculation of Sediment Rate is indicated in Table-6.3

Then, also calculation of sediment rate is divided in 5 years basis are :

- Sed. Rate during 1985 to 1990 : 49.64 Hect.m/100 Km²/year (Table-6-4a).
- Sed. Rate during 1991 to 1995 : 31.89 Hect.m/100 Km²/year (Table-6-4b).
- Sed. Rate during 1996 to 2000 : 226.02 Hect.m/100 Km²/year (Table-6-4c).
- Sed. Rate during 2001 to 2006 : 100.19 Hect.m/100 Km²/year (Table-6-4d).

Average Sediment at Jamrani Dam site for different years are shown in Fig. 6.3.

From above results of calculation, annual sediment rate at Jamrani Dam site is more high than average of some reservoirs in India because of several factors namely a landslide ever taken place in 1993 and mining practices at that site.



98 Fig. 6.2 AVERAGE SEDIMENT AT JAMRANI DAM SITE (1985-2006)

TABLE-6.2
10-DAILY OBSERVED DISCHARGE AND 10-DAILY OBSERVED SEDIMENT AT JAMFRANI DAM SITE

MONTH	10-DAILY	1985	1986	1987	1988	1989	1990	1991	1994	1995	1996	1997	1998	1999	2000	2001	2002	2003	2004	2005	2006	Average
June	1	9.52	22.46	80.79	15.48	15.66	21.10	19.02	14.67	13.61	18.49	15.58	17.85	17.08	1,435.36	28.03	20.92	13.51	24.94	13.93	20.34	91.97
	2	14.65	34.81	20.25	15.39	25.56	18.60	30.38	13.77	12.14	40.80	18.07	46.00	91.37	946.23	19.89	23.66	13.61	28.43	12.08	14.58	72.01
	3	31.07	92.59	17.92	36.66	52.16	30.36	22.89	26.35	63.14	140.08	34.60	102.50	116.77	1,451.84	412.60	24.67	16.48	20.11	43.76	19.90	137.82
July	1	70.50	656.89	139.82	188.54	40.13	2,208.61	22.92	36.35	53.69	221.32	46.29	1,206.85	468.07	976.41	524.60	257.84	21.33	169.25	134.18		386.50
	2	224.96	1,182.79	536.10	163.50	24.83	575.00	26.13	67.00	120.51	185.66	81.20	531.02	459.17	1,839.71	535.68	174.58	1,148.40	223.37	222.56		438.01
	3	922.98	1,253.21	292.50	321.00	591.00	404.00	53.33	94.00	62.07	124.16	79.10	773.48	864.68	1,563.94	749.21	170.36	236.94	177.06	116.64		465.77
August	1	608.68	970.98	127.00	340.50	445.00	1,412.00	144.62	213.76	60.31	138.83	209.20	774.29	845.16	1,001.12	622.50	157.33	279.45	193.98	64.92		453.38
	2	436.09	599.01	394.00	224.00	322.80	435.00	124.18	333.29	464.97	243.63	138.10	1751.14	498.85	2,839.24	690.67	515.04	869.70	266.69	126.60		593.31
	3	880.49	896.32	957.00	171.00	563.00	1,160.00	182.16	193.29	199.81	376.57	82.50	1668.05	444.66	2,312.50	429.60	376.91	749.92	2,192.08	129.52		731.86
September	1	547.22	414.94	353.50	139.42	763.00	802.00	120.78	91.46	1,961.25	520.97	76.60	482.86	472.79	2,711.28	263.58	1,115.17	246.44	255.37	90.00		601.51
	2	635.57	1,121.72	163.50	128.39	404.00	857.00	97.94	78.02	321.62	261.96	75.80	379.47	818.88	1,083.72	201.40	830.52	417.04	202.62	289.78		440.47
	3	382.59	605.43	170.00	192.81	286.00	363.00	96.63	59.84	155.39	131.54	71.00	645.82	552.23	751.29	327.97	342.22	1,074.73	481.59	2,401.21		478.59
October	1	437.62	458.14	34.03	130.78	178.30	225.00	68.58	72.47	107.97	75.80	69.00	309.71	346.81	153.04	153.04	283.52	330.37	206.91	276.05		232.16
	2	3,219.80	324.48	66.81	72.37	99.19	146.63	47.66	59.33	70.76	61.89	63.00	854.94	254.75	207.78	34.74	257.45	229.81	283.04	159.47		342.84
	3	404.23	228.94	70.32	59.95	75.61	89.42	41.28	47.81	68.13	60.05	67.90	577.88	207.27	212.44	68.37	351.66	182.76	205.84	114.95		164.99

MONTH	10-DAILY	1985	1986	1987	1988	1989	1990	1991	1994	1995	1996	1997	1998	1999	2000	2001	2002	2003	2004	2005	2006	Average	
June	1																						
	2		120.84	0.00	0.00	11.67		25.39			3,734.20	0.00	703.96	541.43	2,875.63	2,600.33	327.22	299.86	458.79			835.88	
	3		329.61	0.49	55.83	498.37	18.64				3,434.17	699.22	853.78	888.52	2,167.72	2,333.97	1,055.77	370.59	686.98	1,841.38			1,030.40
July	1		252.41	212.88	1,371.34	69.26	832.57			1,521.63	1,626.55	1,991.19	3,023.20	795.24	1,065.98	1,824.75	2,564.15	883.10	2,862.23	499.23			1,150.87
	2		1,800.52	300.97	830.72	373.87	341.93	60.95		2,650.17	4,152.89	1,048.39	1,755.32	563.85	1,121.14	3,764.47	2,119.86	417.91	328.84	248.48			1,215.57
	3		1,919.17	172.13	341.00	696.34	233.52	2.54		2,705.40	392.09	409.98	343.77	1,564.04	1,062.53	2,924.28	1,526.27	273.17	269.39	307.09			890.75
August	1		3,648.00	5.08	404.87	40.19	144.19	12.07		815.55	1,954.45	4,781.58	1,166.26	6,214.84	3,711.79	761.99	2,369.87	368.91	1,577.27	482.32			1,674.06
	2		1,330.61	57.91	403.50	3.77	997.46	170.43		3,080.16	2,368.97	1,372.12	5651.00	601.03	3,733.79	1,539.71	2,837.27	1,190.33	1,615.12	497.79			1,530.74
	3		920.85	75.53	305.48	915.39	230.45	252.10		1,707.92	1,419.50	280.61	2,485.46	866.33	883.79	1,005.89	1,144.61	3,303.55	3,070.40	148.05			1,157.33
September	1		132.27	39.06	184.76	270.82	119.86	17.35	101.68	2,364.99	5,121.07	310.82	80.26	2,380.93	927.53	1,308.40	3,845.59	620.73	224.01	123.24			973.56
	2		253.77	1.60	119.00	0.65	229.69	195.61	6.85	281.90	970.90	765.24	75.41	3,166.86	1,306.97	878.65	2,577.50	2,308.82	340.74	350.46			733.19
	3		499.96	2.78	481.87	19.44	84.20	0.82	0.00	45.12	3.76	284.22	462.76	828.87	655.45	384.18	639.17	2,122.86	1,674.53	0.00			493.66
October	1		188.84	0.83	41.40	0.00	5.33		0.00	0.00	1.03	650.89	17.65	68.05	0.00	394.49	32.03	446.94	22.22	0.00			117.31
	2		2,160.53	20.71	0.00	0.00	241.63		0.00	0.00	0.00	174.04	0.88	0.00	0.00	74.39	0.00	0.00	413.89	0.00			172.52
	3																						

TABLE-6.3
CALCULATION OF SEDIMENT RATE

MONTH	10-DAILY	Average Discharge (1985 - 2006) (cumec)	Average Sediment (1985 - 2006) (ppm)	Sediment load (m ³ /day)	Sediment load m ³
June	2	72.01	836	5,201	26,004
	3	137.82	1,030	12,270	122,699
July	1	386.50	1,151	38,432	384,320
	2	438.01	1,216	46,002	460,020
	3	465.77	891	35,846	358,460
August	1	453.38	1,674	65,577	655,771
	2	593.31	1,531	78,469	784,690
	3	731.86	1,157	73,181	731,811
September	1	601.51	974	50,596	505,961
	2	440.47	733	27,903	279,030
	3	478.59	434	17,932	179,320
October	1	232.16	117	2,353	23,530
	2	342.84	173	5,110	25,551

Annual Sediment Load 4,537,167.82 m³/year

Sediment Rate 100.83 Hect.m/100 Km²/y

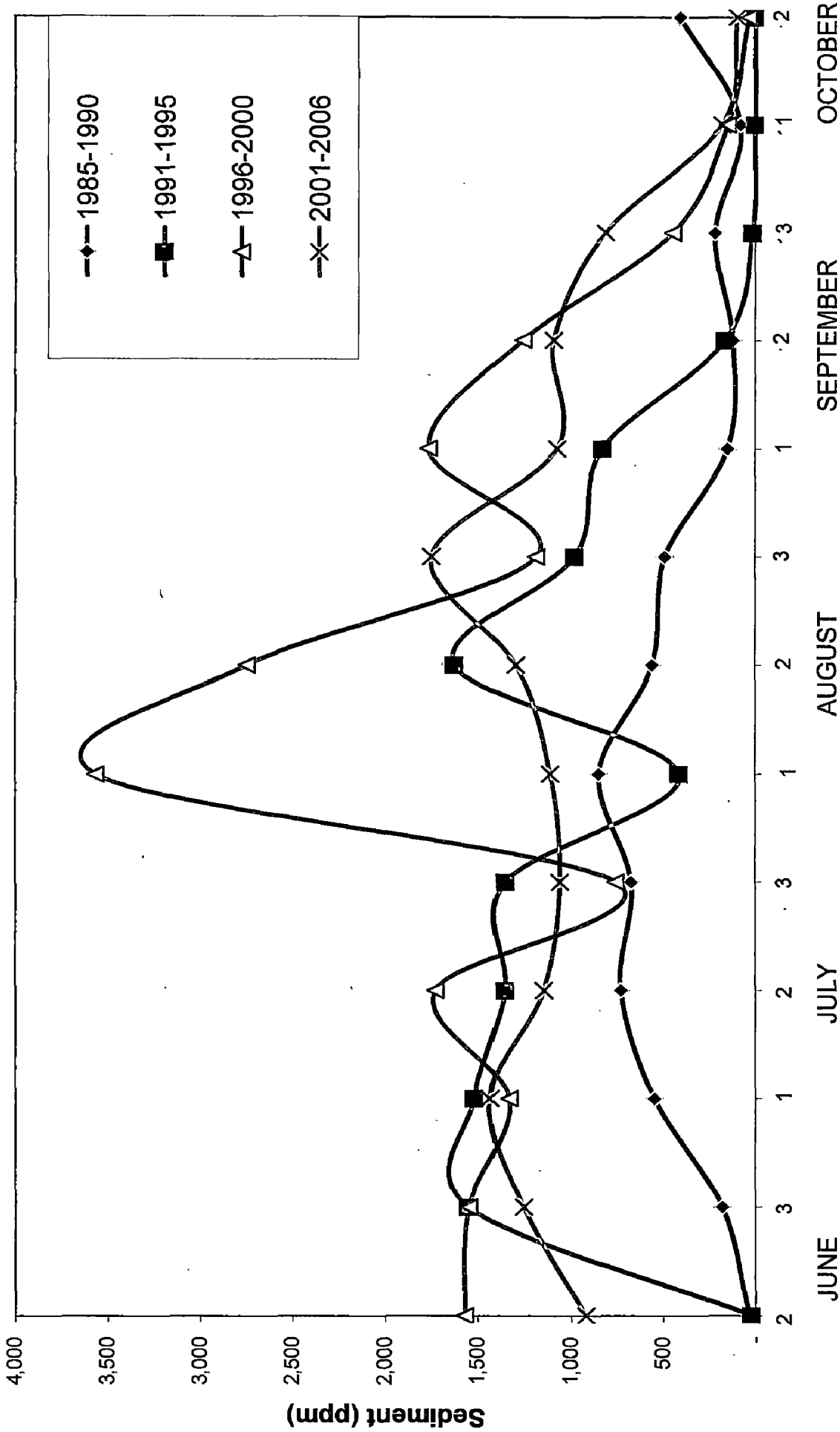


Fig. 6.3 AVERAGE SEDIMENT AT JAMRANI DAM SITE FOR DIFFERENT YEARS

TABLE-6-4a
CALCULATION OF SEDIMENT RATE AT JAMRANI DAM SITE (1985-1990)

MONTH	10-DAILY	Average Discharge (1985 - 1990) (cumec)	Average Sediment (1985 - 1990) (ppm)	Sediment load (m ³ /day)	Sediment load m ³
June	2	21.54	33.13	62	308
	3	43.46	180.59	678	6,781
July	1	550.75	547.69	26,062	260,618
	2	451.20	729.60	28,442	284,423
	3	630.78	672.43	36,647	366,473
August	1	651.53	848.47	47,762	477,617
	2	401.82	558.65	19,395	193,946
	3	771.30	489.54	32,623	326,230
September	1	503.35	149.35	6,495	64,953
	2	551.70	120.98	5,767	57,668
	3	333.31	217.65	6,268	62,677
October	1	243.98	79.71	1,680	16,802
	2	654.88	407.03	23,030	115,152

Annual Sediment Load **2,233,649.50** m³/year

Sediment Rate **49.64** Hect.m/100 Km²/year

TABLE-6-4b
CALCULATION OF SEDIMENT RATE AT JAMRANI DAM SITE (1991-1995)

MONTH	10-DAILY	Average Discharge (1991 - 1995) (cumec)	Average Sediment (1991 - 1995) (ppm)	Sediment load (m ³ /day)	Sediment load m ³
June	2	18.76	25.39	41	206
	3	37.46	1,551.38	5,021	50,211
July	1	37.65	1,521.63	4,950	49,503
	2	71.21	1,355.56	8,341	83,405
	3	69.80	1,353.97	8,165	81,654
August	1	139.56	413.81	4,990	49,898
	2	307.47	1,625.29	43,176	431,761
	3	171.75	980.01	14,543	145,429
September	1	724.50	828.01	51,830	518,305
	2	165.86	162.79	2,333	23,328
	3	103.95	15.31	138	1,375
October	1	83.01	-	-	-
	2	59.25	-	-	-

Annual Sediment Load **1,435,074.66** m³/year

Sediment Rate **31.89** Hect.m/100 Km²/year

TABLE-6-4c
CALCULATION OF SEDIMENT RATE AT JAMRANI DAM SITE (1996-2000)

MONTH	10-DAILY	Average Discharge (1996 - 2000) (cumec)	Average Sediment (1996 - 2000) (ppm)	Sediment load (m ³ /day)	Sediment load m ³
June	2	228.49	1,572	31,027	155,135
	3	369.16	1,549	49,396	493,956
July	1	563.79	1,334	64,992	649,921
	2	619.35	1,728	92,486	924,856
	3	681.07	754	44,397	443,972
August	1	593.72	3,566	182,916	1,829,164
	2	1,094.19	2,745	259,543	2,595,430
	3	976.86	1,187	100,195	1,001,951
September	1	852.90	1,764	129,999	1,299,990
	2	523.97	1,257	56,909	569,088
	3	430.78	447	16,637	166,373
October	1	289.65	148	3,692	36,918
	2	288.47	35	872	4,360

Annual Sediment Load 10,171,113.41 m³/year

Sediment Rate 226.02 Hect.m/100 Km²/year

TABLE-6-4d
CALCULATION OF SEDIMENT RATE AT JAMRANI DAM SITE (2001-2006)

MONTH	10-DAILY	Average Discharge (2001 - 2006) (cumec)	Average Sediment (2001 - 2006) (ppm)	Sediment load (m ³ /day)	Sediment load m ³
June	2	18.71	921.55	1,490	7,449
	3	89.59	1,257.74	9,735	97,353
July	1	221.44	1,438.91	27,530	275,298
	2	460.92	1,146.59	45,661	456,611
	3	290.04	1,060.04	26,564	265,642
August	1	263.56	1,112.07	25,323	253,233
	2	493.74	1,297.08	55,332	553,324
	3	775.61	1,748.08	117,143	1,171,428
September	1	394.11	1,074.37	36,584	365,838
	2	388.27	1,092.01	36,633	366,333
	3	925.54	811.71	64,910	649,102
October	1	249.98	179.14	3,869	38,690
	2	192.90	97.66	1,628	8,138

Annual Sediment Load 4,508,439.07 m³/year

Sediment Rate 100.19 Hect.m/100 Km²/year



CHAPTER 7

SUMMARY AND CONCLUSION

7.1. SUMMARY

1. The average precipitation for Jamrani Dam site based on rainfall data recorded from five and seven rain gauge stations since 1979-1980 to 2005-2006 is **1541.83 mm**.

2. 10-daily observed discharge data available at dam site only from 1977 to 2006 while at Gola Barrage has long term data 10-daily observed discharge since 1948 till to date. Based on above condition, derived 10-daily discharge data for dam site from 1948 to 1977 has been carried out by regression analysis either by direct linear regression method or log deviation regression method.

Direct Linear Regression method is more appropriate than Log Deviation Regression method because the coefficient of correlation in each period of the first method are most higher than of the last method. Therefore, for dependable flow and design flood estimation, derived data by using Direct Linear Regression is adopted.

Result of direct linear regression are tabulated as follows :

TABLE-7.1

RESULTS OF DIRECT LINEAR REGRESSION

FOR 10-DAILY DERIVED DISCHARGE AT JAMRANI DAM SITE

1 st July			2 nd July			3 rd July		
a = 6.95	b = 0.53	r = 0.97	a = -24.04	b = 0.73	r = 0.95	a = -54.19	b = 0.80	r = 0.98
correlation : $Q_J = 0.53 Q_G + 6.95$			correlation : $Q_J = 0.73 Q_G - 24.04$			correlation : $Q_J = 0.80 Q_G - 54.19$		
1 st August			2 nd August			3 rd August		
a = -59.61	b = 0.82	r = 0.93	a = -135.26	b = 0.85	r = 0.94	a = -137.61	b = 0.97	r = 0.98
correlation : $Q_J = 0.82 Q_G - 59.61$			correlation : $Q_J = 0.85 Q_G - 135.26$			correlation : $Q_J = 0.97 Q_G - 137.61$		
1 st September			2 nd September			3 rd September		
a = 18.81	b = 0.69	r = 0.98	a = 17.42	b = 0.67	r = 1.00	a = -6.81	b = 0.74	r = 0.99
correlation : $Q_J = 0.69 Q_G + 18.81$			correlation : $Q_J = 0.67 Q_G + 17.42$			correlation : $Q_J = 0.74 Q_G - 6.81$		

1 st October			2 nd October			3 rd October		
a = -0.94	b = 0.65	r = 0.95	a = -56.33	b = 0.99	r = 1.00	a = -10.53	b = 0.82	r = 0.98
correlation : $Q_J = 0.65 Q_G - 0.94$			correlation : $Q_J = 0.99 Q_G - 56.33$			correlation : $Q_J = 0.82 Q_G - 10.53$		
1 st November			2 nd November			3 rd November		
a = -22.86	b = 1.01	r = 0.95	a = -0.95	b = 0.71	r = 0.90	a = -2.28	b = 0.65	r = 0.87
correlation : $Q_J = 1.01 Q_G - 22.86$			correlation : $Q_J = 0.71 Q_G - 0.95$			correlation : $Q_J = 0.65 Q_G - 2.28$		
1 st December			2 nd December			3 rd December		
a = 0.33	b = 0.63	r = 0.80	a = 2.59	b = 0.56	r = 0.87	a = 3.37	b = 0.61	r = 0.81
correlation : $Q_J = 0.63 Q_G + 0.33$			correlation : $Q_J = 0.56 Q_G + 2.59$			correlation : $Q_J = 0.61 Q_G + 3.37$		
1 st January			2 nd January			3 rd January		
a = 0.98	b = 0.66	r = 0.90	a = 0.27	b = 0.68	r = 0.77	a = -5.83	b = 0.80	r = 0.89
correlation : $Q_J = 0.66 Q_G + 0.98$			correlation : $Q_J = 0.68 Q_G + 0.27$			correlation : $Q_J = 0.80 Q_G - 5.83$		
1 st February			2 nd February			3 rd February		
a = 1.66	b = 0.62	r = 0.88	a = 6.50	b = 0.48	r = 0.92	a = -5.20	b = 0.81	r = 0.96
correlation : $Q_J = 0.62 Q_G + 1.66$			correlation : $Q_J = 0.48 Q_G + 6.50$			correlation : $Q_J = 0.81 Q_G - 5.20$		
1 st March			2 nd March			3 rd March		
a = -12.57	b = 0.95	r = 0.99	a = -4.44	b = 0.83	r = 0.93	a = 2.82	b = 0.59	r = 0.85
correlation : $Q_J = 0.95 Q_G - 12.57$			correlation : $Q_J = 0.83 Q_G - 4.44$			correlation : $Q_J = 0.59 Q_G + 2.82$		
1 st April			2 nd April			3 rd April		
a = 3.14	b = 0.62	r = 0.87	a = 4.99	b = 0.52	r = 0.77	a = 4.66	b = 0.48	r = 0.68
correlation : $Q_J = 0.62 Q_G + 3.14$			correlation : $Q_J = 0.52 Q_G + 4.99$			correlation : $Q_J = 0.48 Q_G + 4.66$		
1 st May			2 nd May			3 rd May		
a = 0.84	b = 0.63	r = 0.86	a = 2.33	b = 0.58	r = 0.90	a = 0.63	b = 0.61	r = 0.90
correlation : $Q_J = 0.63 Q_G + 0.84$			correlation : $Q_J = 0.58 Q_G + 2.33$			correlation : $Q_J = 0.61 Q_G + 0.63$		
1 st June			2 nd June			3 rd June		
a = 13.22	b = 0.18	r = 0.69	a = 4.28	b = 0.54	r = 0.78	a = -7.17	b = 0.78	r = 0.91
correlation : $Q_J = 0.18 Q_G + 13.22$			correlation : $Q_J = 0.54 Q_G + 4.28$			correlation : $Q_J = 0.78 Q_G - 7.17$		

3. 75% dependable annual flow at Jamrani Dam site is estimated as **355.9 MCM**.

4. Design Flood Estimation is carried out by the following methods :

- Gumbel's Method and then checked by Gumbel Probability paper
- Log Pearson type III Distribution
- Unit Hydrograph principle

Comparison of result obtained by four methods are tabulated in Table-7.2.

TABLE-7.2
CALCULATION RESULTS OF FLOOD ESTIMATION
AT JAMRANI DAM SITE

(cumecs)

Return Period (T years)	M E T H O D S			
	Gumbel's	Graphical by prob. paper	Log Pearson type III	Unit Hydrograph
20	3688	3688	3322	
50	4527	4527	4640	4566
100	5155	5250	5355	4996
150	5522	5600	5701	
200	5781	5900	6070	
500	6608	6800	6654	
1000	7232	7500	7755	

5. Sediment data at Jamrani Dam site is observed from 1985 to 2006, between 15 June and 15 October for each year.

Sediment Rate during 1985 to 2006 is 100.83 Hect.m/100 Km²/year.

Then, also calculation of sediment rate is divided in 5 years basis are :

- Sed. Rate during 1985 to 1990 : 49.64 Hect.m/100 Km²/year.
- Sed. Rate during 1991 to 1995 : 31.89 Hect.m/100 Km²/year.
- Sed. Rate during 1996 to 2000 : 226.02 Hect.m/100 Km²/year.
- Sed. Rate during 2001 to 2006 : 100.19 Hect.m/100 Km²/year.

From above results of calculation, annual sediment rate at Jamrani Dam site is more high than average of some reservoirs in India because of several factors namely a landslide ever taken place in 1993 and mining practices at that site.

7.2. CONCLUSION

Hydrology finds its greatest application in the design and operation of water resources engineering projects. Many important projects in the past have failed due to improper assessment of the hydrological factor.

Therefore the Hydrological Analysis of a project should of necessity precede structural and other detailed design studies.

It involves the collection of relevant data and analysis of the data by applying the principles and the theories of hydrology to seek solutions to practical problems.

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APPENDIX – A

AVAILABLE DATA

Table-1.A**10 - Daily observed Rainfall data within the Catchment Area = 450 Km²**

The weight	Jamrani (J)	=	39.00	Km2
	Lugar (L)	=	87.00	Km2
	Okhalkanda (O)	=	151.00	Km2
	Paharpani (P)	=	66.00	Km2
	Champi (C)	=	107.00	Km2
	Total	=	450.00	Km2

1979 - 80		RAINFALL					MAP
Month.	Ten-daily	J (mm)	L (mm)	O (mm)	P (mm)	C (mm)	(mm)
JUL	1	119.00	92.71	2.00	25.50	34.80	40.92
	2	230.57	257.81	182.90	101.60	137.20	178.72
	3	326.00	250.19	115.60	147.30	143.90	171.23
AUG	1	21.60	40.64	41.20	59.70	62.60	47.19
	2	725.60	69.80	159.00	128.30	139.00	181.60
	3	21.60	93.90	6.40	-	-	22.17
SEP	1	1.60	13.90	-	11.40	-	4.50
	2	31.20	42.40	27.70	16.50	18.00	26.90
	3	-	-	-	-	-	-
OCT	1	-	-	4.30	-	4.00	2.39
	2	8.00	-	1.50	-	-	1.20
	3	-	-	-	-	-	-
NOV	1	-	-	-	-	-	-
	2	-	-	-	-	-	-
	3	7.60	11.43	11.40	-	-	6.69
DEC	1	2.40	-	1.50	-	-	0.71
	2	-	-	-	-	-	-
	3	14.40	36.60	27.90	19.70	55.90	33.87
JAN	1	16.80	7.62	8.90	10.10	20.30	12.22
	2	-	-	-	-	-	-
	3	-	-	-	-	-	-
FEB	1	30.40	18.30	33.53	7.60	14.40	21.96
	2	12.80	18.80	19.00	5.55	6.30	13.43
	3	1.30	2.80	8.90	2.90	2.90	4.76
MAR	1	56.00	8.90	104.10	13.50	84.40	63.55
	2	-	-	10.30	1.30	4.30	4.67
	3	8.80	10.90	16.20	8.90	15.20	13.23
APR	1	7.20	8.26	7.40	-	12.70	7.72
	2	-	-	3.80	-	-	1.28
	3	-	-	-	-	-	-
MAY	1	0.40	15.72	6.80	-	-	5.36
	2	24.00	-	6.90	12.70	48.30	17.74
	3	-	16.80	5.80	41.90	-	11.34
JUN	1	184.80	124.70	55.60	44.50	91.19	86.99
	2	93.48	185.90	78.50	95.30	139.90	117.63
	3	74.56	65.90	76.70	73.70	45.70	66.62

TOTAL OF MAP = 1,166.60

SOURCE: FROM 1979-80 TO 1986-87 RECORDS OF EXISTING 5 RAIN GAUGE STATIONS
INSTALLED IN THE CATCHMENT AREA

Table-2.A

10 - Daily observed Rainfall data within the Catchment Area = 450 Km²

Jamrani (J)	=	39.00	Km2
Lugar (L)	=	87.00	Km2
Okhalkanda (O)	=	151.00	Km2
Paharpani (P)	=	66.00	Km2
Champi (C)	=	107.00	Km2
Total =		450.00	Km2

1986 - 87		RAINFALL					MAP (mm)
Month.	Ten-daily	J (mm)	L (mm)	O (mm)	P (mm)	C (mm)	
JUL	1	405.00	27.50	278.75	193.50	236.50	218.57
	2	111.75	170.12	322.00	146.25	43.75	182.48
	3	173.68	175.12	160.00	93.50	118.50	144.49
AUG	1	160.96	44.00	37.50	31.75	29.75	46.77
	2	129.92	221.37	27.50	62.25	107.00	97.86
	3	89.96	117.75	11.25	46.75	31.50	48.68
SEP	1	-	7.50	-	1.00	-	1.60
	2	134.75	-	162.50	206.25	121.25	125.29
	3	49.75	90.50	-	62.50	65.75	46.61
OCT	1	10.35	-	-	15.50	17.50	7.33
	2	18.40	20.00	7.50	13.75	26.50	16.30
	3	11.20	-	-	-	-	0.97
NOV	1	-	-	-	-	-	-
	2	-	-	-	-	-	-
	3	11.20	10.12	11.20	11.20	10.25	10.77
DEC	1	-	-	-	-	-	-
	2	108.00	42.62	46.25	108.00	99.25	72.56
	3	3.20	-	-	3.20	-	0.75
JAN	1	-	-	-	-	-	-
	2	20.00	17.62	-	26.30	17.70	13.21
	3	-	-	-	-	-	-
FEB	1	40.00	-	-	3.25	-	3.94
	2	-	5.12	37.50	70.25	87.25	44.62
	3	-	-	-	-	0.50	0.12
MAR	1	15.00	-	8.75	7.00	-	5.26
	2	-	-	12.50	7.75	-	5.33
	3	12.00	-	3.75	13.75	13.00	7.41
APR	1	-	10.00	13.75	36.75	10.75	14.49
	2	19.20	-	-	-	-	1.66
	3	59.20	43.75	28.75	46.37	19.80	34.75
MAY	1	14.24	40.50	48.76	76.50	102.00	60.90
	2	-	2.87	31.25	15.25	11.75	16.07
	3	132.00	-	-	9.00	2.00	13.24
JUN	1	2.40	53.98	25.90	54.86	59.30	41.48
	2	-	-	-	-	-	-
	3	-	-	-	-	-	-

TOTAL OF MAP = 1,283.48

SOURCE: FROM 1979-80 TO 1986-87 RECORDS OF EXISTING 5 RAIN GAUGE STATIONS
INSTALLED IN THE CATCHMENT AREA

Table-3.A**10 - Daily observed Rainfall data within the Catchment Area = 450 Km²**

Jamrani (J)	=	47.00	Km2
Lugar (L)	=	73.50	Km2
Okhalkanda (O)	=	77.00	Km2
Paharpani (P)	=	49.50	Km2
Champi (C)	=	60.00	Km2
Gunialekh (G)	=	69.00	Km2
Khansual (K)	=	74.00	Km2

Total = **450.00** Km2

1987 - 88		RAINFALL							(MAP)
Month.	Ten-daily	J (mm)	L (mm)	O (mm)	P (mm)	C (mm)	G (mm)	K (mm)	(mm)
JUL	1	146.00	86.98	45.72	32.51	61.22	68.50	108.20	77.31
	2	277.20	214.76	144.78	111.25	96.01	184.75	265.43	185.82
	3	121.14	153.03	33.02	54.35	55.37	38.00	77.47	75.22
AUG	1	39.11	20.45	18.54	10.15	17.78	12.00	29.72	20.81
	2	171.70	168.12	213.36	89.92	143.51	77.72	143.51	146.44
	3	296.42	181.73	138.43	95.48	169.60	34.29	362.29	182.28
SEP	1	66.04	73.15	24.84	44.45	22.86	33.13	20.06	39.41
	2	12.44	-	-	10.92	2.54	12.70	1.27	5.00
	3	12.70	26.92	-	8.12	-	-	-	6.62
OCT	1	-	-	-	12.19	12.19	-	7.62	4.22
	2	12.70	50.16	-	28.45	11.43	-	6.35	15.22
	3	-	-	-	-	-	-	-	-
NOV	1	-	-	-	-	-	-	-	-
	2	-	-	-	-	15.24	-	-	2.03
	3	-	-	-	-	-	-	-	-
DEC	1	-	-	-	-	-	-	-	-
	2	-	15.24	17.70	22.60	-	-	20.32	11.35
	3	-	-	-	-	-	-	-	-
JAN	1	-	-	-	1.27	-	-	-	0.14
	2	2.54	-	8.89	20.07	1.52	-	8.64	5.62
	3	-	-	-	0.76	-	-	-	0.08
FEB	1	-	-	-	-	-	-	-	-
	2	8.13	-	6.35	13.21	7.37	1.26	10.67	6.32
	3	9.75	7.62	69.85	63.50	74.93	6.10	70.61	43.74
MAR	1	39.88	30.48	21.59	71.88	-	29.97	21.84	28.93
	2	45.47	35.56	69.85	56.39	-	5.08	83.06	43.15
	3	2.54	-	8.89	6.10	-	-	-	2.46
APR	1	-	-	-	-	-	-	-	-
	2	7.11	27.94	38.10	25.40	40.89	4.32	36.83	26.79
	3	2.03	6.98	-	15.74	13.72	2.79	11.94	7.30
MAY	1	12.23	8.89	64.77	60.96	-	6.35	35.56	27.34
	2	25.15	4.32	8.89	8.13	-	-	33.57	11.27
	3	11.18	13.20	-	10.67	-	3.05	29.97	9.89
JUN	1	11.43	40.51	36.58	38.86	40.64	17.78	29.67	31.37
	2	148.94	21.08	8.89	36.83	6.86	53.38	19.81	36.93
	3	221.12	101.09	-	73.91	95.76	148.59	78.23	96.15

TOTAL OF MAP = 1,149.21

SOURCE :

FROM 1987-88 TO 2005-06 RECORDS OF EXISTING 7 RAIN GAUGE STATIONS INSTALLED IN THE CATCHMENT AREA

Table-4.A**10 - Daily observed Rainfall data within the Catchment Area = 450 Km²**

Jamrani (J)	=	47.00	Km2
Lugar (L)	=	73.50	Km2
Okhalkanda (O)	=	77.00	Km2
Paharpani (P)	=	49.50	Km2
Champi (C)	=	60.00	Km2
Gunialekh (G)	=	69.00	Km2
Khansual (K)	=	74.00	Km2

Total = **450.00** Km2

2005 - 06		RAINFALL							(MAP) (mm)
Month.	Ten-daily	J (mm)	L (mm)	O (mm)	P (mm)	C (mm)	G (mm)	K (mm)	
JUL	1	172.72	83.40	240.03	10.16	119.89	184.66	99.06	136.94
	2	100.86	116.33	154.94	107.19	71.88	138.94	76.21	114.74
	3	119.38	127.00	114.30	55.12	98.55	137.32	20.32	100.17
AUG	1	38.10	76.81	113.03	34.37	43.18	72.90	12.70	60.97
	2	218.44	218.68	88.90	88.90	39.70	85.09	-	108.41
	3	97.79	59.92	30.48	7.62	15.24	29.21	-	34.36
SEP	1	64.77	92.44	-	30.64	6.86	-	-	28.92
	2	123.17	166.11	204.47	41.87	179.83	309.88	10.41	157.77
	3	299.72	381.76	257.05	328.00	298.45	437.38	320.04	344.65
OCT	1	-	-	-	-	1.52	-	-	0.20
	2	-	-	13.97	-	3.81	-	-	2.90
	3	-	-	-	-	-	-	-	-
NOV	1	-	-	-	-	-	-	-	-
	2	-	-	-	-	-	-	-	-
	3	-	-	-	-	-	-	-	-
DEC	1	-	-	-	-	-	-	-	-
	2	-	-	-	-	-	-	-	-
	3	1.27	1.78	15.24	2.54	5.08	11.43	5.08	6.63
JAN	1	-	-	-	-	5.08	-	-	0.68
	2	-	-	-	-	-	-	-	-
	3	-	-	-	-	-	-	-	-
FEB	1	-	-	-	-	-	-	-	-
	2	-	-	-	-	-	-	-	-
	3	-	-	-	-	-	-	-	-
MAR	1	5.08	-	22.86	-	33.02	-	-	8.84
	2	29.72	41.65	77.47	38.10	52.07	130.05	39.37	61.96
	3	-	-	-	-	1.27	-	-	0.17
APR	1	-	-	-	2.29	-	-	-	0.25
	2	-	-	13.97	2.54	-	5.08	2.50	3.86
	3	-	6.60	-	-	-	3.81	3.81	2.49
MAY	1	33.02	-	-	22.86	3.81	-	6.35	7.52
	2	-	16.76	-	-	50.80	-	12.70	12.10
	3	38.10	47.61	-	12.70	27.94	-	-	18.30
JUN	1	88.90	54.35	100.33	11.43	72.39	90.17	-	61.69
	2	10.16	8.38	24.13	3.81	8.13	-	-	8.31
	3	42.16	138.17	123.19	76.20	81.28	87.63	24.13	88.81

TOTAL OF MAP = 1,371.65

SOURCE :

FROM 1987-88 TO 2005-06 RECORDS OF EXISTING 7 RAIN GAUGE STATIONS INSTALLED IN THE CATCHMENT AREA

Table-5.A
10 - Daily Discharge observed at Dam Site (Cumecs)

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1976-1977	1									31.86	22.88	23.67	23.68	
	2									25.09	22.67	36.39	31.68	
	3									24.70	22.42	34.51	92.15	
TOTAL										81.65	67.97	94.57	147.51	
1977-1978	1	75.20	184.62	206.93	128.05	85.72	63.10	44.68	38.51	57.92	27.50	16.43	14.34	943.000
	2	156.83	812.10	331.99	95.67	74.24	44.44	44.36	53.58	83.33	28.64	16.48	16.87	1,778.530
	3	463.58	218.68	176.20	99.54	61.05	53.74	45.83	28.78	54.47	24.53	19.35	99.42	1,345.170
TOTAL	695.61	1,215.40	735.12	323.26	221.01	161.28	134.87	120.87	80.67	195.72	80.67	52.26	130.63	4,066.700
1978-1979	1	450.00	926.66	988.82	170.77	80.02	61.43	42.83	48.89	61.82	34.78	21.80	18.85	2,906.670
	2	553.00	339.00	345.50	116.69	70.22	54.02	43.77	51.47	47.74	30.09	37.77	31.68	1,720.950
	3	209.00	357.00	230.00	102.23	65.67	49.21	55.76	53.64	45.22	27.33	32.53	96.00	1,323.590
TOTAL	1,212.00	1,622.66	1,564.32	389.69	215.91	164.66	142.36	154.00	154.00	154.78	92.20	18.04	146.53	5,951.210
1979-1980	1	82.77	126.82	138.84	83.22	43.37	27.37	28.01	27.84	32.57	20.76	18.04	19.39	649.000
	2	251.35	607.07	117.35	55.43	39.25	25.06	24.43	23.78	26.85	20.07	16.58	30.85	1,238.070
	3	396.05	370.06	97.64	54.12	33.44	28.26	26.04	19.81	21.75	19.86	15.58	38.93	1,121.540
TOTAL	730.17	1,103.95	353.83	192.77	116.06	80.69	78.48	71.43	71.43	81.17	60.69	50.20	89.17	3,008.610
1980-1981	1	134.07	501.54	517.36	215.47	65.56	35.38	30.64	34.57	25.98	22.14	18.64	17.05	1,618.400
	2	272.24	324.88	433.43	155.91	44.89	33.17	27.80	28.91	23.00	22.12	22.99	16.10	1,405.440
	3	504.37	178.07	393.12	115.62	36.99	35.05	68.51	21.26	25.34	21.06	19.56	281.62	1,700.570
TOTAL	910.68	1,004.49	1,343.91	487.00	147.44	103.60	126.95	84.74	84.74	74.32	65.32	61.19	314.77	4,724.410
1981-1982	1	192.02	472.00	194.36	77.00	40.00	46.00	36.30	40.00	54.00	34.50	40.00	45.23	1,261.410
	2	916.45	231.00	152.56	70.00	43.00	31.00	33.99	36.00	42.00	34.50	46.00	60.00	1,696.500
	3	816.57	198.00	51.56	78.00	50.00	35.50	39.86	24.00	45.50	32.00	35.70	46.00	1,452.690
TOTAL	1,925.04	901.00	388.48	225.00	133.00	112.50	110.15	100.00	100.00	141.50	101.00	121.70	151.23	4,410.600
1982-1983	1	77.11	291.28	1,027.02	442.66	50.98	33.42	32.78	25.31	33.44	18.78	31.47	21.96	2,086.210
	2	56.46	276.82	802.37	249.02	44.19	30.64	27.88	28.01	20.66	43.80	35.19	22.14	1,637.180
	3	370.04	1,559.09	838.82	68.87	37.21	35.62	33.59	20.92	22.78	44.93	51.49	130.86	3,214.220
TOTAL	503.61	2,127.19	2,668.21	760.55	132.38	99.68	94.25	74.24	74.24	76.88	107.51	118.15	174.96	6,937.610
1983-1984	1	144.08	242.00	298.11	354.76	68.91	57.30	55.09	67.57	13.15	13.36	13.15	29.16	1,356.640
	2	50.29	76.84	308.10	188.56	64.20	57.21	55.09	75.06	13.36	13.17	14.78	35.98	952.640
	3	655.74	121.90	1,819.88	139.16	60.21	57.24	62.05	88.30	14.75	13.14	19.67	368.71	3,420.750
TOTAL	850.11	440.74	2,426.09	682.48	193.32	171.75	172.23	230.93	230.93	41.26	39.67	47.60	433.85	5,730.030
1984-1985	1	523.76	1,190.47	1,062.50	685.33	38.41	23.62	23.52	16.97	14.82	11.16	12.53	9.52	3,612.610
	2	348.56	1,063.62	1,024.98	158.67	29.84	22.46	20.22	16.33	13.21	10.52	12.43	14.65	2,735.490
	3	1,352.27	1,395.48	960.99	54.77	27.24	22.91	19.12	12.82	9.91	9.91	12.41	31.07	3,911.420
TOTAL	2,224.59	3,649.57	3,048.47	898.77	95.49	68.99	45.73	45.73	45.73	40.85	31.59	37.37	55.24	10,259.520
1985-1986	1	70.50	608.68	547.22	437.62	183.32	78.74	57.37	34.20	37.54	24.69	20.87	22.46	2,123.210
	2	224.96	436.09	635.57	3,219.80	108.84	62.62	47.72	67.88	31.92	23.19	21.01	34.81	4,914.410
	3	922.98	880.49	382.59	404.23	84.53	71.79	44.41	36.47	31.55	22.18	28.13	92.59	2,999.940
TOTAL	1,218.44	1,925.26	1,565.38	4,061.65	376.69	213.15	149.50	136.55	136.55	101.01	70.06	70.01	149.86	10,037.560

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1986-1987	1	656.89	970.98	414.94	458.14	187.52	91.79	88.78	57.18	29.18	18.95	46.26	80.79	3,101.400
	2	1,182.79	599.01	1,171.72	324.48	144.37	185.25	82.77	63.99	23.86	17.14	25.68	20.25	3,791.310
	3	1,253.21	896.32	605.43	228.94	102.52	151.94	76.76	39.85	22.98	15.74	18.35	17.92	3,429.960
TOTAL		3,092.89	2,466.31	2,142.09	1,011.56	434.41	428.98	248.31	161.02	76.02	51.83	90.29	118.96	10,322.670
1987-1988	1	139.82	353.50	34.03	34.03	39.30	29.49	26.54	21.04	20.98	19.91	55.66	15.48	882.750
	2	536.10	394.00	163.50	66.81	36.80	28.56	25.51	20.45	67.12	19.07	15.34	15.39	1,388.650
	3	292.50	957.00	170.00	70.32	32.38	27.38	27.02	17.83	24.99	19.25	16.54	36.66	1,691.870
TOTAL		968.42	1,478.00	687.00	171.16	108.48	85.43	79.07	59.32	113.09	58.23	87.54	67.53	3,963.270
1988-1989	1	188.54	340.50	139.42	130.78	43.22	32.75	51.12	26.65	20.41	18.22	14.82	15.66	1,022.090
	2	163.50	224.00	128.39	72.37	38.22	29.36	44.18	23.92	21.94	17.05	13.48	25.56	801.970
	3	321.00	171.00	192.81	59.95	42.03	33.10	33.11	18.20	20.38	15.82	15.35	52.16	974.910
TOTAL		673.04	735.50	460.62	263.10	123.47	95.21	128.41	68.77	62.73	51.09	43.65	93.38	2,798.970
1989-1990	1	40.13	445.00	763.00	178.30	49.37	36.79	27.14	22.89	185.34	35.79	20.21	21.10	1,825.060
	2	24.83	322.80	404.00	99.19	44.77	32.07	26.28	64.86	45.27	26.75	23.22	18.60	1,132.640
	3	591.00	563.00	286.00	75.61	43.75	32.19	26.82	61.88	43.11	20.82	26.84	30.36	1,801.380
TOTAL		655.96	1,330.80	1,453.00	353.10	137.89	101.05	80.24	149.63	273.72	83.36	70.27	70.06	4,759.080
1990-1991	1	2,208.61	1,417.00	802.00	225.00	62.35	36.16	57.25	21.36	36.11	18.31	15.38	19.02	4,918.550
	2	575.00	435.00	837.00	146.63	51.65	30.09	31.94	24.44	28.46	19.73	13.81	30.38	2,244.130
	3	404.00	1,160.00	363.00	89.42	45.41	75.12	27.98	23.87	24.28	16.73	24.96	22.89	2,277.660
TOTAL		3,187.61	3,012.00	2,022.00	461.05	159.41	141.37	117.17	69.67	88.85	54.77	54.15	72.29	9,440.340
1991-1992	1	22.92	144.62	120.78	68.58	29.03	27.75	28.18	34.74	20.87	15.76	13.18	19.41	545.020
	2	26.13	124.18	97.94	47.66	37.93	24.07	27.86	31.46	18.32	14.91	12.37	22.41	485.240
	3	53.33	182.16	96.63	41.28	29.26	39.36	30.19	20.41	19.19	13.73	14.13	69.35	609.020
TOTAL		102.38	450.96	315.35	157.52	96.22	91.18	86.23	86.61	57.58	44.40	39.68	111.17	1,639.280
1992-1993	1	27.49	239.90	47.84	16.44	37.51	17.68	20.81	18.40	18.40	22.74	14.67	12.63	494.510
	2	40.34	85.89	67.10	47.09	32.39	25.37	20.46	18.62	18.73	17.99	16.24	13.25	403.470
	3	265.94	107.96	25.46	54.02	31.82	25.97	20.85	15.50	40.83	15.88	14.94	16.05	635.220
TOTAL		333.77	433.75	140.40	117.55	101.72	69.02	62.12	52.52	77.96	56.61	45.85	41.93	1,533.200
1993-1994	1	32.61	280.23	1,588.86	46.46	63.01	35.29	28.76	20.27	24.92	19.02	14.96	14.67	2,169.060
	2	94.24	194.89	3,108.50	59.43	47.59	32.67	34.72	29.24	22.48	16.72	18.28	13.77	3,672.530
	3	407.97	127.83	320.10	84.70	37.61	33.70	32.71	25.83	22.74	15.59	16.44	26.35	1,151.570
TOTAL		534.82	602.95	5,017.46	190.59	148.21	101.66	96.19	75.34	70.14	51.33	49.68	54.79	6,993.160
1994-1995	1	36.35	213.76	91.46	72.47	38.35	26.37	27.91	25.99	24.98	21.14	18.50	13.61	610.890
	2	67.00	333.25	78.02	59.33	33.62	24.80	31.53	24.95	25.03	20.65	16.82	12.14	727.140
	3	94.00	193.29	59.84	47.81	29.18	26.70	27.71	21.57	24.05	19.40	16.88	63.14	623.570
TOTAL		197.35	740.30	229.32	179.61	101.15	77.87	87.15	72.51	74.06	61.19	52.20	88.89	1,961.600
1995-1996	1	53.69	60.31	1,961.25	107.97	39.78	30.48	28.83	23.80	26.64	19.99	13.74	18.49	2,379.970
	2	120.51	464.97	321.62	70.76	33.69	28.11	28.51	26.74	22.11	16.10	14.02	40.80	1,187.940
	3	62.07	139.81	155.39	68.13	31.59	28.69	27.62	27.85	24.39	14.39	13.76	140.08	733.770
TOTAL		236.27	665.09	2,438.26	246.86	105.06	87.28	79.96	78.39	73.14	50.48	41.52	199.37	4,301.680
1996-1997	1	121.32	138.83	520.97	75.80	42.23	34.91	27.82	25.26	20.90	29.07	20.38	15.58	1,073.070
	2	185.66	243.63	251.96	61.89	42.91	31.29	26.45	22.54	21.52	20.04	17.07	18.07	953.030
	3	124.16	376.57	133.54	60.05	37.82	31.86	29.94	16.77	21.44	18.62	16.57	34.60	901.940
TOTAL		431.14	759.03	916.47	197.74	122.96	98.06	84.21	64.57	63.86	67.73	54.02	68.25	2,928.040

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1997-1998	1	46.29	209.20	76.60	69.00	51.31	52.08	43.75	31.00	33.23	36.62	41.94	17.85	708.870
	2	81.20	138.10	75.80	63.00	48.34	61.55	41.45	30.00	33.54	34.48	28.88	46.00	682.340
	3	79.10	82.50	71.00	67.90	42.93	45.52	36.86	24.47	35.30	29.98	22.40	102.50	640.460
TOTAL		206.59	429.80	223.40	199.90	142.58	159.15	122.06	85.47	102.07	101.08	93.22	166.35	2,031.670
1998-1999	1													
	2													
	3													
TOTAL														
1999-2000	1	468.07	845.16	472.79	346.81	110.44	36.09	26.02	34.32	19.07	14.80	17.13	1,436.36	3,827.060
	2	459.17	498.85	818.88	254.75	75.23	36.07	22.18	27.71	17.43	15.62	14.50	946.23	3,186.620
	3	864.68	444.66	552.23	207.27	41.35	32.78	21.50	20.52	17.63	15.68	17.31	1,451.84	3,687.450
TOTAL		1,791.92	1,788.67	1,843.90	808.83	227.02	104.94	69.70	82.55	54.13	46.10	48.94	3,834.43	10,701.130
2000-2001	1	976.41	1,001.12	2,711.28	646.92	123.24	32.59	24.71	17.92	16.16	14.19	12.21	28.03	5,604.780
	2	1,839.71	2,839.24	1,083.72	207.78	59.25	29.07	21.41	18.22	15.09	14.45	14.25	19.89	6,162.080
	3	1,563.94	2,312.50	751.29	212.44	39.99	26.09	21.54	14.40	15.54	13.88	14.88	412.60	5,399.090
TOTAL		4,380.06	6,152.86	4,546.29	1,067.14	222.48	87.75	67.66	50.54	46.79	42.52	41.34	460.52	17,165.950
2001-2002	1	524.60	622.50	263.58	153.04	50.42	22.76	18.47	19.23	56.86	18.39	18.88	20.92	1,789.650
	2	535.68	690.67	201.40	34.74	32.27	21.23	18.16	68.54	19.45	16.93	16.31	23.68	1,679.060
	3	749.21	429.60	327.97	68.37	25.61	21.67	20.95	16.72	20.60	19.85	17.45	24.67	1,742.670
TOTAL		1,809.49	1,742.77	792.95	256.15	108.30	65.66	57.58	104.49	96.91	55.17	52.64	69.27	5,211.380
2002-2003	1	257.84	157.33	1,115.17	283.52	53.17	26.42	23.63	28.63	232.25	18.31	14.70	13.51	2,224.480
	2	174.58	515.04	830.52	257.45	42.06	24.85	21.52	418.79	49.38	16.68	13.56	13.61	2,378.040
	3	170.36	376.91	342.22	351.66	33.17	23.66	23.50	212.92	26.46	15.35	14.05	16.48	1,606.740
TOTAL		602.78	1,049.28	2,287.91	892.63	128.40	74.93	68.65	660.34	308.09	50.34	42.31	43.60	6,209.260
2003-2004	1	21.33	279.45	246.44	330.37	155.08	44.56	22.95	24.42	18.24	15.64	16.04	24.94	1,199.460
	2	1,148.40	869.70	417.04	229.81	159.59	34.19	21.62	21.87	17.54	14.59	13.04	28.43	2,975.820
	3	236.94	749.92	1,074.73	182.76	151.80	29.52	23.70	17.87	17.99	17.31	19.15	20.11	2,541.800
TOTAL		1,406.67	1,899.07	1,738.21	742.94	466.47	108.27	68.27	64.16	53.77	47.54	48.23	73.48	6,717.080
2004-2005	1	169.25	193.58	255.37	206.91	64.97	30.32	19.19	30.84	19.23	15.15	13.29	13.93	1,032.030
	2	223.37	266.69	202.62	283.04	49.81	25.72	23.66	34.70	17.23	13.76	12.63	12.08	1,165.310
	3	177.06	2,192.08	481.59	205.84	40.39	23.39	34.75	18.12	17.10	13.42	13.86	43.76	3,261.360
TOTAL		569.68	2,652.35	939.58	695.79	155.17	79.43	77.60	83.66	53.56	42.33	39.78	69.77	5,458.700
2005-2006	1	134.18	64.92	90.00	276.05	81.56	40.58	20.90	19.12	18.10	16.83	13.77	20.34	796.350
	2	222.56	126.60	289.78	159.47	77.24	32.48	22.60	18.89	20.32	15.91	19.76	14.58	1,020.190
	3	116.64	129.52	2,401.21	114.95	65.65	24.96	23.41	14.12	20.40	14.51	17.20	19.90	2,962.470
TOTAL		473.38	321.04	2,780.99	550.47	224.45	98.02	66.91	52.13	58.82	47.25	50.73	54.82	4,779.010

Table-6.A

10 - Daily Discharge observed at Gola Barrage (Cumecs)

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1948-1949	1	201.30	505.30	1,316.90	369.20	159.80	131.10	63.40	421.70	77.80	45.00	58.90	283.90	3,634.30
	2	243.90	1,631.20	814.90	520.80	176.90	105.30	55.50	57.90	45.30	60.50	70.50	46.20	3,828.90
	3	288.90	5,452.20	568.90	255.40	152.90	75.70	55.00	56.00	48.20	39.60	108.30	76.30	7,177.40
TOTAL		734.10	7,588.70	2,700.70	1,145.40	489.60	312.10	173.90	535.60	171.30	145.10	237.70	406.40	14,640.60
1949-1950	1	321.80	1,450.90	1,327.60	861.80	124.70	88.90	69.00	83.60	52.60	49.70	39.70	50.90	4,521.20
	2	1,322.20	2,500.30	1,348.50	314.20	124.60	79.80	63.00	83.80	50.00	42.60	55.30	1,096.50	7,080.80
	3	1,681.40	2,976.90	2,381.30	277.90	108.50	84.10	72.20	46.80	642.80	40.30	83.00	992.60	9,387.80
TOTAL		3,325.40	6,928.10	5,057.40	1,453.90	357.80	252.80	204.20	214.20	745.40	132.60	178.00	2,140.00	20,989.80
1950-1951	1	1,757.60	3,538.70	1,628.40	427.20	152.20	125.80	80.80	87.50	48.70	50.10	71.00	71.30	8,039.30
	2	1,945.40	4,082.30	1,742.20	225.10	138.50	110.40	80.60	67.80	48.20	47.30	86.20	39.80	8,613.80
	3	3,312.90	2,936.90	1,309.80	183.00	121.00	102.30	73.80	39.30	60.60	41.90	41.00	131.50	8,354.00
TOTAL		7,015.90	10,557.90	4,680.40	835.30	411.70	338.50	235.20	194.60	157.50	139.30	198.20	242.60	25,007.10
1951-1952	1	67.10	659.00	41.20	292.60	111.90	91.20	80.40	58.40	74.30	60.50	65.30	71.30	1,673.20
	2	467.60	273.00	44.80	187.40	95.20	87.60	78.40	65.90	72.50	60.50	69.70	129.50	1,632.10
	3	609.00	5,013.40	43.00	158.50	96.60	85.90	81.10	64.10	79.10	58.30	73.20	949.30	7,311.50
TOTAL		1,143.70	5,945.40	129.00	638.50	303.70	264.70	239.90	188.40	225.90	179.30	208.20	1,150.10	10,616.80
1952-1953	1	161.60	2,200.00	1,431.70	158.50	112.10	52.90	41.70	40.00	35.70	31.20	35.70	33.90	4,335.00
	2	1,240.50	2,805.00	1,687.80	143.20	106.60	49.10	72.00	39.90	35.70	29.60	44.40	221.20	6,475.00
	3	612.10	2,665.30	288.00	142.90	82.30	46.00	64.50	28.90	37.30	29.20	46.90	1,436.90	5,480.30
TOTAL		2,014.20	7,670.30	3,407.50	444.60	301.00	148.00	178.20	108.80	108.70	90.00	127.00	1,692.00	16,290.30
1953-1954	1	437.50	634.60	1,637.40	194.70	107.70	67.80	172.80	106.80	85.80	64.70	43.40	26.80	3,580.00
	2	3,421.50	416.40	563.60	175.40	102.90	60.40	60.50	102.30	74.00	47.10	49.50	108.70	5,182.30
	3	2,988.80	1,012.40	369.60	147.40	90.20	60.00	70.50	66.50	86.30	42.20	55.60	194.90	5,184.40
TOTAL		6,847.80	2,063.40	2,570.60	517.50	300.80	188.20	303.80	275.60	246.10	154.00	148.50	330.40	13,946.70
1954-1955	1	94.40	1,226.30	692.60	2,368.00	257.90	131.60	91.50	60.70	65.90	32.00	34.90	48.10	5,103.90
	2	603.20	1,599.20	646.70	806.60	197.10	117.20	90.40	73.80	62.60	32.50	39.30	46.60	4,315.20
	3	1,746.00	1,530.10	511.10	361.80	153.40	108.60	123.00	52.80	56.70	32.50	62.80	258.90	4,997.70
TOTAL		2,443.60	4,355.60	1,850.40	3,536.40	608.40	357.40	304.90	187.30	185.20	97.00	137.00	353.60	14,416.80
1955-1956	1	1,171.50	1,333.80	883.40	3,833.40	417.00	210.70	121.90	79.60	62.10	60.10	37.90	2,179.70	10,391.10
	2	894.30	1,911.90	623.30	2,416.90	275.00	188.40	99.70	67.90	71.90	56.20	76.60	1,281.20	7,963.30
	3	912.50	1,021.50	854.70	767.50	223.90	163.20	96.20	56.90	72.40	43.30	826.60	829.40	5,868.10
TOTAL		2,978.30	4,267.20	2,361.40	7,017.80	915.90	562.30	317.80	204.40	206.40	159.60	941.10	4,290.30	24,222.50
1956-1957	1	3,501.50	1,013.40	1,142.90	1,100.70	529.60	168.90	114.10	76.50	59.00	53.50	35.10	42.80	7,838.00
	2	844.90	925.20	722.90	1,864.90	291.40	120.80	126.40	56.20	60.40	106.10	44.00	41.70	5,204.90
	3	955.20	128.40	530.30	803.80	204.80	115.50	110.50	39.00	16.30	46.00	50.00	50.30	3,050.10
TOTAL		5,301.60	2,067.00	2,396.10	3,769.40	1,025.80	405.20	351.00	171.70	135.70	205.60	129.10	134.80	16,093.00
1957-1958	1	141.60	566.40	472.20	211.60	191.50	91.60	86.90	75.50	57.70	26.10	49.70	48.20	2,019.00
	2	402.90	297.80	1,720.60	194.10	111.70	139.50	72.50	68.70	50.80	24.60	121.40	41.00	3,245.60
	3	1,258.70	483.30	257.90	136.30	103.20	110.40	86.60	51.20	51.50	21.90	57.20	40.00	2,658.20
TOTAL		1,803.20	1,347.50	2,450.70	542.00	406.40	341.50	246.00	195.40	160.00	72.60	228.30	129.20	7,922.80

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1958-1959	1	1,357.80	422.90	571.30	1,062.60	250.50	142.50	91.30	74.00	77.50	53.20	42.40	52.20	4,198.20
	2	1,100.70	280.70	478.90	431.90	224.30	103.40	80.00	87.40	59.90	46.00	51.70	57.00	3,001.90
	3	1,727.80	1,163.00	1,123.50	373.30	181.10	127.70	96.50	67.10	59.50	44.90	63.70	70.90	5,099.00
TOTAL		4,186.30	1,866.60	2,173.70	1,867.80	655.90	373.60	267.80	228.50	196.90	144.10	157.80	180.10	12,299.10
1959-1960	1	669.10	1,030.80	435.20	501.00	242.90	128.20	76.90	71.40	55.30	54.00	41.20	44.50	3,350.50
	2	375.90	1,002.90	714.10	377.30	223.80	97.30	76.60	56.10	56.90	49.20	48.60	43.40	3,132.10
	3	983.60	747.00	615.30	310.90	163.90	93.70	84.90	44.70	60.00	45.60	50.70	51.50	3,251.80
TOTAL		2,028.60	2,780.70	1,764.60	1,189.20	630.60	319.20	238.40	182.20	172.20	148.80	140.50	139.40	9,734.40
1960-1961	1	181.40	912.70	1,496.30	960.60	199.10	65.80	139.10	118.90	64.00	45.20	37.20	79.30	4,299.60
	2	1,123.00	880.20	1,179.30	494.30	167.00	83.10	95.90	114.80	71.00	37.80	51.80	205.40	4,503.60
	3	536.70	1,148.80	616.00	339.90	146.90	76.70	87.40	79.80	65.70	35.70	62.20	258.60	3,454.40
TOTAL		1,841.10	2,941.70	3,291.60	1,794.80	513.00	225.60	322.40	313.50	200.70	118.70	151.20	543.30	12,257.60
1961-1962	1	962.60	1,390.00	628.90	236.30	252.40	122.70	100.70	124.90	255.40	62.90	45.50	54.80	4,237.10
	2	1,068.20	1,934.00	419.70	593.60	184.50	149.10	95.00	96.80	79.10	54.80	46.70	215.00	4,936.50
	3	952.90	1,606.10	310.20	358.80	157.80	143.20	226.10	105.60	78.30	50.00	59.10	257.00	4,305.10
TOTAL		2,983.70	4,930.10	1,358.80	1,188.70	594.70	415.00	421.80	327.30	412.80	167.70	151.30	526.80	13,478.70
1962-1963	1	117.00	351.90	289.50	348.00	149.20	108.50	78.00	70.90	64.10	64.10	60.40	69.30	1,770.90
	2	191.50	799.00	235.80	277.30	140.90	99.50	78.90	67.50	75.90	58.40	67.30	82.20	2,174.20
	3	780.50	650.60	2,853.80	198.00	116.60	90.00	84.90	49.90	76.60	52.10	58.40	73.80	5,085.20
TOTAL		1,089.00	1,801.50	3,379.10	823.30	406.70	298.00	241.80	188.30	216.60	174.60	186.10	225.30	9,030.30
1963-1964	1	276.60	477.50	1,215.00	361.30	192.00	134.20	93.30	93.50	79.80	51.60	56.30	62.30	3,093.40
	2	380.20	739.50	1,224.30	293.70	163.10	136.40	91.80	35.20	73.80	47.00	61.80	66.90	3,313.70
	3	500.30	2,824.80	586.70	267.80	150.20	105.10	95.80	76.40	58.70	50.80	79.00	131.20	4,926.80
TOTAL		1,157.10	4,041.80	3,026.00	922.80	505.30	375.50	280.90	205.10	212.30	149.40	197.10	260.40	11,333.90
1964-1965	1	754.20	1,153.90	1,400.30	622.90	162.20	108.20	78.00	66.10	61.00	78.30	39.80	52.00	4,576.90
	2	1,129.50	768.10	967.40	257.50	137.60	110.20	95.50	78.80	56.40	60.00	52.20	47.40	3,760.60
	3	2,170.20	932.50	1,519.10	216.90	122.50	84.50	119.60	67.90	81.80	51.10	61.10	55.80	5,483.00
TOTAL		4,053.90	2,854.50	3,886.80	1,097.30	422.30	302.90	293.10	212.80	192.20	189.40	153.10	155.20	13,820.50
1965-1966	1	176.90	758.70	554.00	157.80	99.90	68.60	50.50	41.80	41.00	36.60	30.20	44.20	2,060.20
	2	313.70	313.30	329.10	130.50	78.20	62.50	50.60	45.80	38.70	33.00	35.30	44.90	1,475.60
	3	287.10	1,232.90	211.20	124.90	72.00	57.70	49.80	29.10	40.40	29.60	51.40	279.80	2,465.90
TOTAL		777.70	2,304.90	1,094.30	413.20	250.10	188.80	150.90	116.70	120.10	99.20	116.90	368.90	6,001.70
1966-1967	1	169.90	1,028.50	329.80	155.80	106.60	70.90	65.70	57.10	48.90	61.90	53.80	63.20	2,212.10
	2	148.10	559.50	292.10	130.70	93.60	61.40	61.60	50.80	51.90	59.30	57.80	59.70	1,626.50
	3	807.60	439.30	183.00	122.70	79.10	73.30	64.70	40.70	93.70	61.10	65.10	72.80	2,103.10
TOTAL		1,125.60	2,027.30	804.90	409.20	279.30	205.60	192.00	148.60	194.50	182.30	176.70	195.70	5,941.70
1967-1968	1	495.70	840.30	988.30	284.10	142.70	99.50	81.00	91.50	78.10	55.30	45.70	69.40	3,271.60
	2	909.30	873.50	908.90	210.50	110.20	88.40	76.20	81.30	61.90	46.50	54.00	237.60	3,658.30
	3	803.80	2,260.60	461.80	204.30	103.50	93.70	97.30	90.80	71.00	48.60	65.50	873.60	5,174.50
TOTAL		2,208.80	3,974.40	2,359.00	698.90	356.40	281.60	254.50	263.60	211.00	150.40	165.20	1,180.60	12,104.40
1968-1969	1	422.10	315.80	251.80	250.30	109.50	81.40	69.40	56.10	50.80	42.00	45.80	44.10	1,739.10
	2	412.00	902.90	277.90	140.10	99.10	77.20	76.60	57.10	42.50	42.00	72.40	40.50	2,240.30
	3	386.60	719.20	362.60	101.80	89.90	85.70	70.60	43.30	45.50	38.50	68.40	57.00	2,069.10
TOTAL		1,220.70	1,937.90	892.30	492.20	298.50	244.30	216.60	156.50	138.80	122.50	186.60	141.60	6,048.50

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1969-1970	1	71.80	850.50	755.50	492.60	150.40	100.50	72.10	64.80	43.80	33.00	30.30	184.30	2,849.60
	2	800.70	485.30	684.40	271.60	119.90	90.30	109.20	58.60	37.10	30.80	33.80	424.30	3,146.00
	3	641.70	968.10	2,168.40	250.50	111.20	90.70	87.20	43.40	40.10	29.30	50.30	955.30	5,436.20
TOTAL	1,514.20	2,303.90	3,608.30	1,014.70	381.50	281.50	268.50	268.50	166.80	121.00	93.10	114.40	1,563.90	11,431.80
1970-1971	1	536.80	467.80	518.70	202.10	103.70	68.90	36.70	35.30	46.50	25.20	24.40	182.00	2,248.10
	2	179.70	910.30	359.70	141.00	94.10	58.30	34.80	29.80	32.10	24.30	94.00	570.40	2,528.50
	3	439.20	551.70	268.30	114.10	70.40	48.30	42.30	24.80	32.90	22.50	151.60	196.60	1,962.70
TOTAL	1,155.70	1,929.80	1,146.70	457.20	268.20	175.50	175.50	113.80	89.90	111.50	72.00	270.00	949.00	6,739.30
1971-1972	1	972.20	410.50	1,008.10	152.70	124.70	80.20	47.60	51.80	40.70	26.00	22.20	29.60	2,966.30
	2	312.40	615.60	473.30	143.00	86.30	77.60	47.00	47.10	34.40	24.80	32.50	29.20	1,923.20
	3	321.80	375.00	200.40	135.10	93.70	64.80	87.60	26.60	32.20	24.00	28.10	43.90	1,433.20
TOTAL	1,606.40	1,401.10	1,681.80	430.80	304.70	222.60	182.20	125.50	125.50	107.30	74.80	82.80	102.70	6,322.70
1972-1973	1	268.70	251.70	303.80	197.60	110.50	72.80	48.50	36.60	35.00	27.80	28.20	44.30	1,425.50
	2	458.00	435.00	1,148.70	181.90	94.60	62.80	47.80	36.60	45.50	28.60	31.00	142.50	2,713.00
	3	325.50	294.30	261.20	224.80	69.10	52.40	60.30	24.40	35.90	27.80	47.10	319.20	1,742.00
TOTAL	1,052.20	981.00	1,713.70	604.30	274.20	188.00	156.60	156.60	97.60	116.40	84.20	106.30	506.00	5,880.50
1973-1974	1	182.00	534.00	272.90	291.10	160.60	94.10	64.40	48.50	32.70	31.40	26.20	33.50	1,771.40
	2	192.20	345.40	380.20	210.80	123.10	85.80	47.40	51.70	31.20	29.20	27.60	58.80	1,583.40
	3	286.00	310.00	252.60	201.10	105.10	81.30	52.70	31.10	35.50	27.40	30.50	36.10	1,449.40
TOTAL	660.20	1,189.40	905.70	703.00	388.80	261.50	164.50	164.50	131.30	99.40	88.00	84.30	128.40	4,804.20
1974-1975	1	69.30	363.10	208.00	95.00	59.20	45.80	69.30	72.70	51.40	32.20	24.20	36.40	1,126.60
	2	77.90	291.10	174.20	87.00	53.00	50.30	55.20	75.70	46.30	28.80	25.90	50.50	1,015.90
	3	554.40	263.60	124.60	79.40	48.90	57.50	89.00	57.00	44.80	27.60	33.20	444.70	1,824.70
TOTAL	701.60	917.80	506.80	261.40	161.10	153.60	213.50	213.50	205.40	142.50	88.60	83.30	531.60	3,967.20
1975-1976	1	607.80	905.80	1,239.50	291.00	129.00	82.00	61.00	50.70	51.70	34.90	37.40	44.20	3,535.00
	2	546.40	432.40	856.90	236.00	116.50	83.20	57.80	69.30	41.80	40.90	56.20	76.20	2,613.60
	3	373.40	427.90	422.40	203.20	97.10	74.40	59.50	57.70	39.30	33.20	40.90	53.20	1,882.20
TOTAL	1,527.60	1,766.10	2,518.80	730.20	342.60	239.60	178.30	178.30	177.70	132.80	109.00	134.50	173.60	8,030.80
1976-1977	1	70.90	466.70	296.00	204.80	97.90	75.20	48.00	48.50	35.60	27.70	39.80	30.10	1,441.20
	2	240.80	1,445.30	299.00	146.00	92.20	68.50	42.50	43.70	30.30	28.90	47.20	32.10	2,516.50
	3	433.00	542.20	250.90	128.20	79.20	73.80	46.20	31.60	32.70	27.10	34.30	84.50	1,763.70
TOTAL	744.70	2,454.20	845.90	479.00	269.30	217.50	136.70	136.70	123.80	98.60	83.70	121.30	146.70	5,721.40
1977-1978	1	136.10	235.70	257.30	194.40	108.20	67.10	55.90	37.20	46.70	38.70	32.90	50.70	1,260.90
	2	241.50	1,146.70	407.20	166.00	96.90	57.00	91.30	68.10	87.90	37.00	29.90	71.80	2,501.30
	3	516.00	309.10	241.60	142.70	82.60	67.80	45.70	27.90	70.90	35.70	37.30	102.70	1,680.00
TOTAL	893.60	1,691.50	906.10	503.10	287.70	191.90	192.90	192.90	133.20	205.50	111.40	100.10	225.20	5,442.20
1978-1979	1	706.40	1,235.30	1,335.40	342.70	119.30	93.60	50.40	67.50	78.70	41.20	33.70	36.30	4,140.50
	2	388.20	545.50	454.70	325.90	109.60	79.60	56.50	70.30	63.80	37.30	69.70	90.30	2,291.40
	3	378.30	530.20	292.90	150.30	105.00	71.50	70.20	63.70	56.00	36.00	45.70	216.80	2,016.60
TOTAL	1,472.90	2,311.00	2,083.00	818.90	333.90	244.70	177.10	177.10	201.50	198.50	114.50	149.10	343.40	8,448.50
1979-1980	1	189.30	185.50	199.80	97.70	55.80	41.50	39.60	38.90	47.60	25.50	14.90	34.10	970.20
	2	396.80	765.50	151.20	85.00	40.60	39.30	38.80	31.50	33.20	21.40	14.40	55.10	1,672.80
	3	415.40	363.40	127.60	73.90	66.80	48.20	36.60	26.30	29.10	18.20	16.80	54.70	1,277.00
TOTAL	1,001.50	1,314.40	478.60	256.60	163.20	129.00	115.00	115.00	96.70	109.90	65.10	46.10	143.90	3,920.00

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1980-1981	1	376.90	565.80	695.80	194.60	92.60	69.30	53.10	57.10	41.20	42.00	18.40	33.50	2,240.30
	2	400.90	421.50	333.40	134.50	80.90	53.60	52.50	49.30	36.60	36.90	26.50	35.20	1,661.80
	3	376.80	354.40	245.50	122.60	77.70	64.10	80.60	80.60	33.10	48.90	34.90	1,044.80	2,513.50
TOTAL		1,154.60	1,341.70	1,274.70	451.70	251.20	187.00	186.20	139.50	126.70	113.80	75.00	1,113.50	6,415.60
1981-1982	1	392.50	650.90	213.40	103.60	70.40	47.50	30.30	28.30	49.60	26.90	23.30	25.60	1,662.30
	2	1,143.60	350.80	184.80	68.80	60.20	43.20	25.70	22.00	48.00	23.50	64.90	28.70	2,064.20
	3	604.40	264.20	103.90	65.30	46.40	41.20	29.20	20.10	44.90	31.30	33.00	25.80	1,309.70
TOTAL		2,140.50	1,265.90	502.10	237.70	177.00	131.90	85.20	70.40	142.50	81.70	121.20	80.10	5,036.20
1982-1983	1	21.60	226.50	553.40	120.40	56.50	27.60	33.30	34.80	14.40	10.80	30.60	28.50	1,158.40
	2	33.90	208.90	347.30	96.30	47.40	29.50	28.90	30.00	19.30	18.80	43.60	28.10	932.00
	3	57.50	1,209.70	180.10	75.40	36.90	35.70	52.60	19.60	11.90	43.90	70.90	63.00	1,857.20
TOTAL		113.00	1,645.10	1,080.80	292.10	140.80	92.80	114.80	84.40	45.60	73.50	145.10	119.60	3,947.60
1983-1984	1	248.50	147.90	172.40	315.70	98.60	37.80	54.50	42.10	54.50	33.10	23.50	332.70	1,561.30
	2	183.80	186.40	181.80	141.40	81.50	53.50	57.10	215.20	41.10	30.90	31.90	84.80	1,289.40
	3	459.20	203.40	166.80	143.10	78.10	55.30	56.90	87.49	44.90	30.10	41.00	401.30	1,767.59
TOTAL		891.50	537.70	521.00	600.20	258.20	146.60	168.50	344.79	140.50	94.10	96.40	818.80	4,618.29
1984-1985	1	508.41	345.63	357.00	129.05	66.72	49.60	64.32	36.57	30.03	219.17	35.95	25.08	1,867.53
	2	164.00	188.71	229.33	93.45	57.63	46.71	46.76	34.99	26.98	24.97	24.23	75.33	1,013.09
	3	1,673.38	412.21	153.96	80.92	55.66	49.23	44.50	26.21	27.54	21.26	45.97	245.27	2,836.11
TOTAL		2,345.79	946.55	740.29	303.42	180.01	145.54	155.58	97.77	84.55	265.40	106.15	345.68	5,716.73
1985-1986	1	399.73	832.62	1,037.70	589.83	197.23	108.48	79.24	49.23	50.19	41.70	36.94	38.50	3,461.39
	2	514.70	792.54	768.19	3,159.02	129.74	99.34	65.45	135.15	45.55	39.20	69.55	139.31	5,957.74
	3	1,306.56	2,370.67	546.49	493.49	116.44	118.78	65.39	45.69	48.12	39.52	45.94	1,093.27	6,290.36
TOTAL		2,220.99	3,995.83	2,352.38	4,242.34	443.41	326.60	210.08	230.07	143.86	120.42	152.43	1,271.08	15,709.49
1986-1987	1	1,277.71	672.78	238.28	255.16	109.70	77.76	53.74	42.35	35.24	28.87	36.94	40.00	2,868.53
	2	1,139.22	341.73	758.42	193.24	98.91	150.35	56.29	56.91	32.15	27.76	33.22	32.27	2,920.47
	3	695.49	904.13	348.38	144.97	88.83	77.99	53.89	34.67	35.18	26.94	28.24	30.51	2,469.22
TOTAL		3,112.42	1,918.64	1,345.08	593.37	297.44	306.10	163.92	133.93	102.57	83.57	98.40	102.78	8,258.22
1987-1988	1	97.91	94.28	338.09	81.83	54.16	40.07	34.12	26.57	29.71	23.59	18.62	23.94	862.89
	2	351.33	271.36	208.91	70.29	45.64	37.75	29.37	27.28	58.57	22.41	18.76	23.66	1,165.33
	3	192.93	595.67	122.19	70.27	42.64	38.58	30.08	24.70	30.67	27.93	22.27	94.25	1,292.18
TOTAL		642.17	961.31	669.19	222.39	142.44	116.40	93.57	78.55	118.95	73.93	59.65	141.85	3,320.40
1988-1989	1	1,100.71	698.56	288.51	154.98	61.29	45.30	83.64	34.63	25.61	24.05	19.07	42.33	2,578.68
	2	222.27	719.38	217.46	101.84	54.16	42.05	51.89	32.00	29.09	22.58	19.98	60.78	1,573.48
	3	462.32	570.62	300.08	84.28	48.47	48.82	45.05	23.97	26.83	20.17	22.49	126.14	1,779.24
TOTAL		1,785.30	1,988.56	806.05	341.10	163.92	136.17	180.58	90.60	81.53	66.80	61.54	229.25	5,931.40
1989-1990	1	58.63	319.41	846.38	139.22	63.18	48.19	35.82	27.62	199.04	49.32	31.04	27.39	1,845.24
	2	92.73	157.04	354.78	102.63	55.63	43.21	33.76	57.41	59.19	36.42	46.86	37.66	1,077.32
	3	285.35	533.78	224.17	85.10	54.83	42.68	33.80	83.96	58.99	30.53	50.32	71.69	1,555.20
TOTAL		436.71	1,010.23	1,425.33	326.95	173.64	134.08	103.38	168.99	317.22	116.27	128.22	136.74	4,477.76
1990-1991	1	1,813.41	591.85	589.93	256.08	88.82	65.42	82.48	40.21	60.53	26.65	22.21	46.29	3,683.88
	2	1,030.16	1,542.78	722.66	167.71	76.57	60.69	52.72	40.43	39.67	27.31	22.07	59.37	3,842.14
	3	501.29	765.83	400.92	130.14	74.09	77.74	51.23	32.91	34.22	24.01	31.92	26.84	2,151.14
TOTAL		3,344.86	2,900.46	1,713.51	553.93	239.48	203.85	186.43	113.55	134.42	77.97	76.20	132.50	9,677.16

YEAR	10-DAY	JUL	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	TOTAL
1991-1992	1	29.71	148.33	301.07	90.75	48.56	39.84	41.31	53.17	28.21	20.57	17.29	70.03	888.84
	2	462.34	288.57	235.71	70.26	50.79	35.37	37.12	37.97	24.22	19.13	17.97	27.56	1,307.01
	3	60.58	464.05	138.35	64.30	45.36	76.52	39.02	28.08	24.31	18.78	25.36	454.61	1,439.32
TOTAL		552.63	900.95	675.13	225.31	144.71	151.73	117.45	119.22	76.74	58.48	60.62	552.20	3,635.17
1992-1993	1	35.54	591.88	406.17	170.06	62.65	46.15	34.41	29.85	27.28	29.26	23.09	24.84	1,481.18
	2	325.97	365.02	437.80	110.89	55.04	42.39	35.82	28.55	26.40	25.41	19.49	22.98	1,495.76
	3	576.94	511.36	217.86	89.20	53.51	41.06	35.30	22.41	77.03	23.66	23.09	39.93	1,711.35
TOTAL		938.45	1,468.26	1,061.83	370.15	171.20	129.60	105.53	80.81	130.71	78.33	65.67	87.75	4,688.29
1993-1994	1	35.37	296.77	2,302.00	241.42	94.85	60.02	42.22	40.75	79.09	23.65	21.22	26.77	3,264.13
	2	289.22	278.72	4,632.70	159.99	74.90	53.25	49.38	58.99	25.18	23.26	28.64	29.43	5,703.66
	3	623.21	240.09	502.62	141.14	66.21	60.41	43.47	60.37	26.73	22.35	25.98	140.38	1,952.96
TOTAL		947.80	815.58	7,437.32	542.55	235.96	173.68	135.07	160.11	131.00	69.26	75.84	196.58	10,920.75
1994-1995	1	273.20	546.66	286.81	111.15	59.85	43.09	52.77	34.18	37.01	24.02	18.98	19.52	1,507.24
	2	223.97	591.17	223.06	89.70	45.81	37.35	55.12	39.67	33.76	21.59	18.68	23.71	1,403.59
	3	233.91	611.42	138.36	80.56	46.14	36.00	50.26	34.86	31.40	21.31	19.77	30.00	1,333.99
TOTAL		731.08	1,749.25	648.23	281.41	151.80	116.44	158.15	108.71	102.17	66.92	57.43	73.23	4,244.82
1995-1996	1	264.18	228.75	1,707.13	167.66	67.49	51.05	44.54	39.56	38.93	28.75	19.61	48.22	2,705.87
	2	448.47	1,105.35	450.45	115.48	61.46	55.23	51.58	42.08	36.78	26.59	21.84	65.99	2,481.30
	3	394.37	624.57	270.79	91.38	59.56	56.17	48.04	45.31	35.74	25.25	93.68	148.03	1,892.89
TOTAL		1,107.02	1,958.67	2,428.37	374.52	188.51	162.45	144.16	126.95	111.45	80.59	135.13	262.24	7,080.06
1996-1997	1	101.58	305.97	697.68	177.16	80.39	58.80	45.30	42.10	35.00	41.08	36.95	43.15	1,665.16
	2	228.92	471.25	358.49	131.86	61.46	53.39	42.19	38.34	33.16	39.67	34.97	56.42	1,550.12
	3	248.21	598.61	181.25	105.40	59.57	55.30	50.02	28.81	40.75	40.06	31.44	122.09	1,561.51
TOTAL		578.709	1,375.83	1,237.42	414.42	201.42	167.49	137.51	109.25	108.91	120.81	103.36	221.66	4,776.79
1997-1998	1	84.46	5,705.22	232.09	148.61	88.85	65.14	61.32	50.56	49.80	47.74	51.27	36.16	6,621.22
	2	408.01	410.86	231.69	119.66	76.26	60.69	64.18	44.08	57.92	49.04	48.39	49.69	1,620.47
	3	225.54	281.17	183.65	108.14	74.07	50.70	62.01	40.40	57.05	38.83	40.62	266.86	1,429.04
TOTAL		718.01	6,397.25	647.43	376.41	239.18	176.53	187.51	135.04	164.77	135.61	140.28	352.71	9,670.73
1998-1999	1	2,263.97	1,016.95	672.54	477.93	385.40	152.64	77.08	68.78	54.67	29.53	18.39	21.45	5,239.33
	2	760.35	2,219.29	540.38	920.47	283.27	109.64	72.92	62.29	55.02	20.77	20.75	161.27	5,226.42
	3	1,034.59	1,861.51	881.93	717.51	162.56	88.82	79.46	45.20	57.61	20.22	20.10	158.90	5,128.41
TOTAL		4,058.91	5,097.75	2,094.85	2,115.91	831.23	351.10	229.46	176.27	167.30	70.52	59.24	341.62	15,594.16
1999-2000	1	786.98	666.35	311.97	239.42	112.36	77.76	63.72	97.73	44.26	31.13	40.07	50.19	2,972.94
	2	325.83	433.45	623.94	186.56	96.37	76.20	59.48	71.53	37.92	29.68	32.62	238.99	2,233.17
	3	573.08	257.43	318.78	177.76	87.59	76.40	58.74	46.37	37.72	32.58	34.78	317.73	2,018.96
TOTAL		1,685.89	1,357.23	1,254.69	603.74	296.92	230.36	181.94	215.63	119.90	93.39	107.47	1,077.91	7,225.07
2000-2001	1	494.14	1,028.75	1,112.93	226.97	126.26	83.13	67.37	45.95	41.37	29.71	33.45	92.13	3,382.16
	2	456.04	1,150.90	473.34	198.33	104.87	74.05	64.63	45.30	37.32	45.50	41.96	49.91	2,742.15
	3	479.73	923.99	331.76	162.28	94.13	74.76	64.00	35.82	37.45	33.45	36.98	163.73	2,438.08
TOTAL		1,429.91	3,103.64	1,918.03	587.58	325.26	231.94	196.00	127.07	116.14	108.66	112.39	305.77	8,562.39
2001-2002	1	128.18	305.40	287.60	105.71	62.67	51.84	33.73	37.63	151.75	32.17	26.79	30.98	1,254.45
	2	593.72	324.36	179.06	90.01	56.85	44.93	36.70	133.36	47.17	26.14	25.98	27.31	1,585.59
	3	538.77	447.54	126.16	84.23	53.77	41.54	42.67	37.29	45.44	41.89	29.65	33.76	1,522.71
TOTAL		1,260.67	1,077.30	592.82	279.95	173.29	138.31	113.10	208.28	244.36	100.20	82.42	92.05	4,362.75

Table-7.A

PANEVAPORATION DATA AT JAMRANI DAM SITE (in cm.)

YEAR	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
1974						3.60	2.80	1.70	2.90	4.90	2.20	2.40
1975	7.07	8.70	8.50	24.70	28.10	9.30	1.60	0.90	0.90	23.44		
1976					21.70					15.35	4.25	5.10
1977	6.00	1.50	23.00	29.20	35.60	27.00	1.50	3.70	3.40	8.10	6.40	2.30
1978	3.00	5.15	10.90	29.50	68.80	38.20	3.70	2.70	4.30	16.09	10.10	6.10
1979	5.40	7.00	16.70	27.60	48.80	49.70	28.40	40.00	53.60	33.70	13.40	7.30
1980	6.40	16.60	14.40	39.40	59.20	27.30	7.80	11.30	18.10	24.00	14.10	1.70
1981	1.30	7.30	13.70	26.50	37.70	30.10	7.50	11.90	35.80	31.80	9.60	4.30
1982	2.70	3.80	10.60	22.10	30.20	32.10	28.50	7.40	9.90	7.70	3.80	1.80
1983	9.00	4.90	15.50	19.80	22.40	24.30	14.70	2.10	16.50	10.50	10.30	2.50
1984	4.20	26.10	8.30	15.10	20.70	8.20	7.34	8.10	9.45	10.10	4.56	1.04
TOTAL	45.07	81.05	121.60	233.90	373.20	249.80	103.84	89.80	154.85	185.68	78.71	34.54
Average Jamrani	5.01	9.01	13.51	25.99	37.32	24.98	10.38	8.98	15.49	16.88	7.87	3.45
Average Pantnagar	10.30	13.70	22.70	47.00	45.30	31.00	27.10	16.10	16.50	19.30	11.20	8.10

PANEVAPORATION DATA AT PANTNAGAR (in cm.)

YEAR	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
2002	6.27	9.26	12.99	20.62	21.78	27.48	23.07	17.14	12.31	11.62	9.02	7.37
2003	3.49	7.17	10.90	20.15	26.22	20.10	16.03	11.31	8.95	9.50	6.52	4.47
2004	3.39	6.48	15.02	22.44	25.69	18.76	15.00	12.96	11.16	9.02	5.43	4.64
2005	4.55	6.57	11.98	23.12	22.66		16.48	14.97	12.86	9.02	7.09	5.94
2006	4.92	7.44	15.69	23.74	25.77	20.42	15.29	14.99	12.23	9.67	6.92	5.25
TOTAL	22.62	36.92	66.58	110.07	122.12	86.76	85.87	71.37	57.51	48.83	34.98	27.67
Average Pantnagar	4.52	7.38	13.32	22.01	24.42	21.69	17.17	14.27	11.50	9.77	7.00	5.53



APPENDIX – B

COMPUTATION SHEET

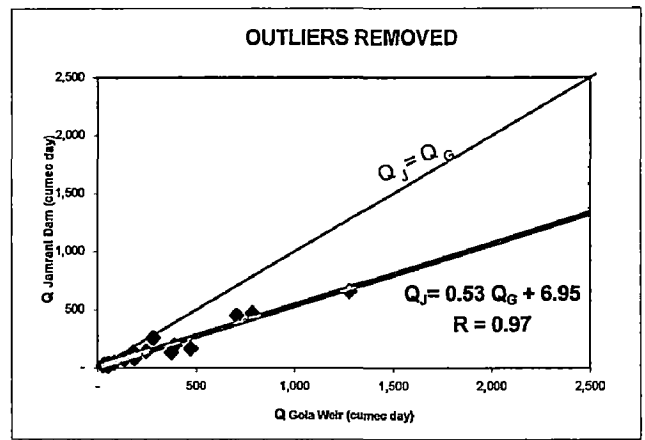
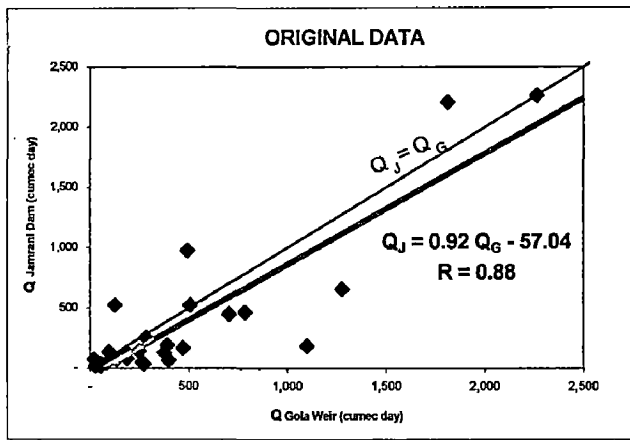
TABLE-1B
THE RELATIONSHIP BETWEEN JAMRANI DAM (QJ) TO GOLA BARRAGE (QG) DISCHARGE
FOR THE PERIOD 1976 - 2006 WITH EACH 10-DAILY BY DIRECT LINEAR REGRESSION
1st 10-DAILY IN MONTH OF JULY

MONTH	10 - DAILY	YEAR	X	Y	X ²	Y ²	X.Y
			GOLA BARRAGE (QG)	JAMRANI DAM (QJ)			
J U L Y	1	1976-77			0.00	0.00	0.00
		1977-78	136.10	75.20	18,523.21	5,655.04	10,234.72
		1978-79	706.40	450.00	499,000.96	202,500.00	317,880.00
		1979-80	189.30	82.77	35,834.49	6,850.87	15,668.36
		1980-81	376.90	134.07	142,053.61	17,974.76	50,530.98
		1981-82	392.50	192.02	154,056.25	36,871.68	75,367.85
		1982-83			0.00	0.00	0.00
		1983-84	248.50	144.08	61,752.25	20,759.05	35,803.88
		1984-85			0.00	0.00	0.00
		1985-86			0.00	0.00	0.00
		1986-87	1,277.71	656.89	1,632,542.84	431,504.47	839,314.92
		1987-88			0.00	0.00	0.00
		1988-89			0.00	0.00	0.00
		1989-90	58.63	40.13	3,437.48	1,610.42	2,352.82
		1990-91			0.00	0.00	0.00
		1991-92	29.71	22.92	882.68	525.33	680.95
		1992-93	35.54	27.49	1,263.09	755.70	976.99
		1993-94	35.37	32.61	1,251.04	1,063.41	1,153.42
		1994-95			0.00	0.00	0.00
		1995-96			0.00	0.00	0.00
		1996-97			0.00	0.00	0.00
		1997-98	84.46	46.29	7,133.49	2,142.76	3,909.65
		1998-99			0.00	0.00	0.00
		1999-2000	786.98	468.07	619,337.52	219,089.52	368,361.73
		2000-01			0.00	0.00	0.00
		2001-02			0.00	0.00	0.00
2002-03	284.64	257.84	81,019.93	66,481.47	73,391.58		
2003-04	53.99	21.33	2,914.92	454.97	1,151.61		
2004-05	473.00	169.25	223,729.00	28,645.56	80,055.25		
2005-06	184.69	134.18	34,110.40	18,004.27	24,781.70		
TOTAL			5,354.42	2,955.14	3,518,843.16	1,060,889.29	1,901,616.42
N			17	17			
MEAN			314.97	173.83			

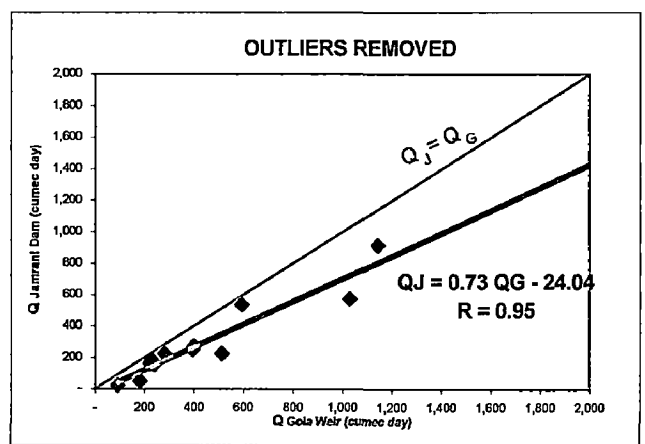
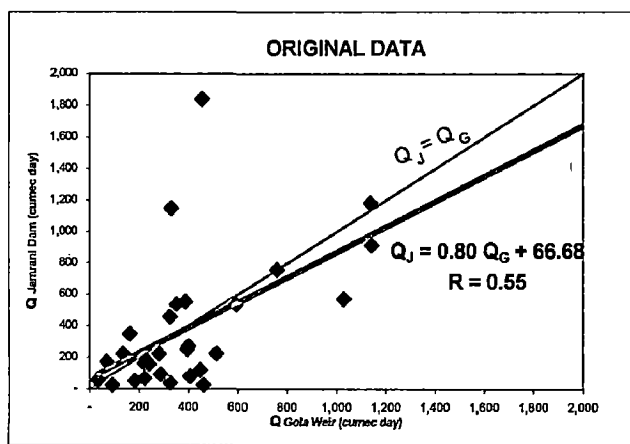
a = 6.95

b = 0.53

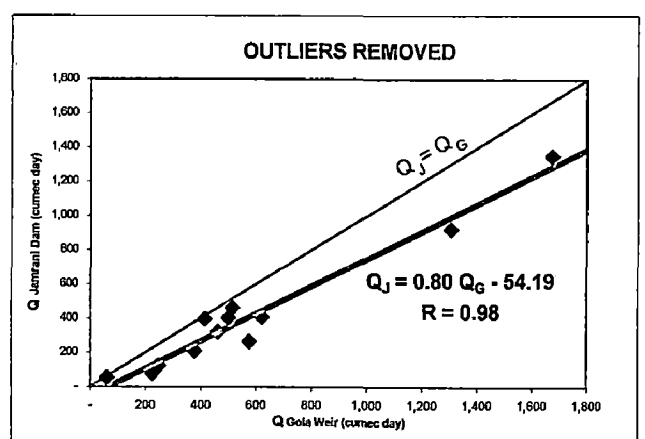
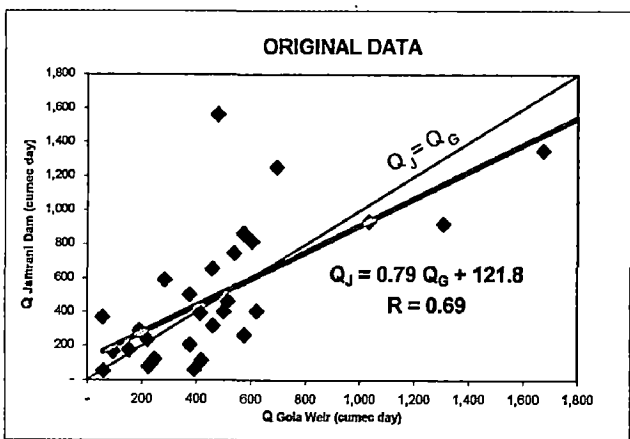
r = 0.97



1st TEN-DAILY DISCHARGE FOR JULY (1976 - 2006)



2nd TEN-DAILY DISCHARGE FOR JULY (1976 - 2006)



3rd TEN-DAILY DISCHARGE FOR JULY (1976 - 2006)

Fig. 1B

PLOT OF 10-DAILY OBSERVED DISCHARGE OF GOLA RIVER AT JAMRANI DAM & GOLA WEIR (BY DIRECT LINEAR REGRESSION) FOR MONTH OF JULY

TABLE-2B
THE RELATIONSHIP BETWEEN JAMRANI DAM (QJ) TO GOLA BARRAGE (QG) DISCHARGE
FOR THE PERIOD 1976 - 2006 WITH EACH 10-DAILY BY LOG DEVIATION REGRESSION
1st 10-DAILY IN MONTH OF JULY

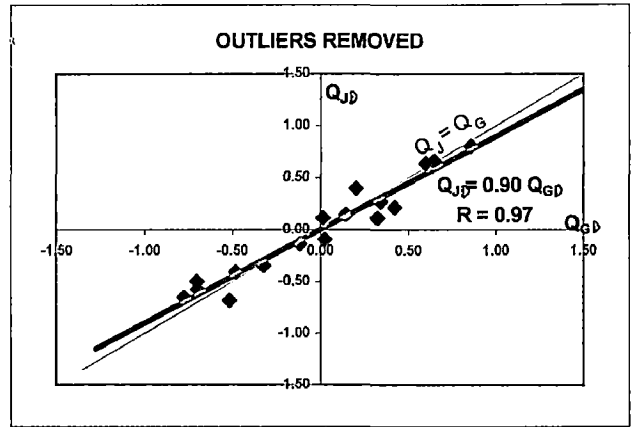
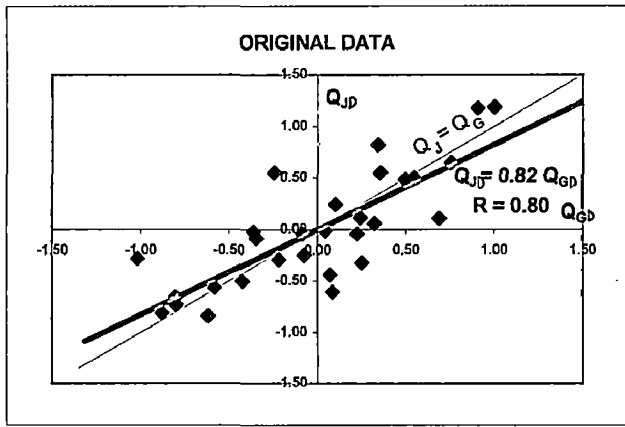
MONTH	10 - DAILY	YEAR	X GOLA BAR. (QG)	Log x	X (Log x - Log \bar{x})	Y JAMRANI DAM (QJ)	Log y	Y (Log y - Log \bar{y})	X ²	Y ²	X.Y	
J U L Y	1	1976-77							0.00	0.00	0.00	
		1977-78	136.10	2.13	-0.11	75.20	1.88	-0.13	0.01	0.02	0.02	
		1978-79	706.40	2.85	0.60	450.00	2.65	0.64	0.36	0.41	0.39	
		1979-80	189.30	2.28	0.03	82.77	1.92	-0.09	0.00	0.01	0.00	
		1980-81	376.90	2.58	0.33	134.07	2.13	0.12	0.11	0.01	0.04	
		1981-82	392.50	2.59	0.35	192.02	2.28	0.27	0.12	0.07	0.09	
		1982-83										
		1983-84	248.50	2.40	0.15	144.08	2.16	0.15	0.02	0.02	0.02	
		1984-85										
		1985-86										
		1986-87	1,277.71	3.11	0.86	656.89	2.82	0.81	0.74	0.65	0.69	
		1987-88										
		1988-89										
		1989-90	58.63	1.77	-0.48	40.13	1.60	-0.41	0.23	0.17	0.20	
		1990-91										
		1991-92	29.71	1.47	-0.78	22.92	1.36	-0.65	0.60	0.42	0.50	
		1992-93	35.54	1.55	-0.70	27.49	1.44	-0.57	0.49	0.33	0.40	
		1993-94	35.37	1.55	-0.70	32.61	1.51	-0.50	0.49	0.25	0.35	
		1994-95										
		1995-96										
1996-97												
1997-98	84.46	1.93	-0.32	46.29	1.67	-0.35	0.10	0.12	0.11			
1998-99												
1999-2000	786.98	2.90	0.65	468.07	2.67	0.66	0.42	0.44	0.43			
2000-01												
2001-02												
2002-03	284.64	2.45	0.21	257.84	2.41	0.40	0.04	0.16	0.08			
2003-04	53.99	1.73	-0.52	21.33	1.33	-0.68	0.27	0.46	0.35			
2004-05	473.00	2.67	0.43	169.25	2.23	0.22	0.18	0.05	0.09			
2005-06	184.69	2.27	0.02	134.18	2.13	0.12	0.00	0.01	0.00			
TOTAL			5,354.42	38.22	0.00	2,955.14	34.18	0.00	4.18	3.60	3.76	
N			17									
MEAN			314.97	2.25	0.00	173.83	2.01	0.00				

a = 0.00

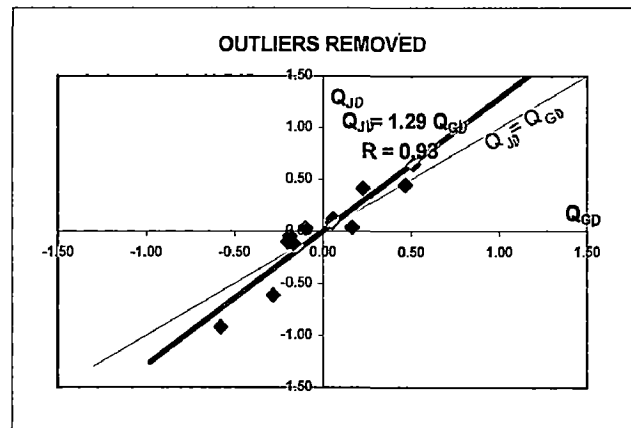
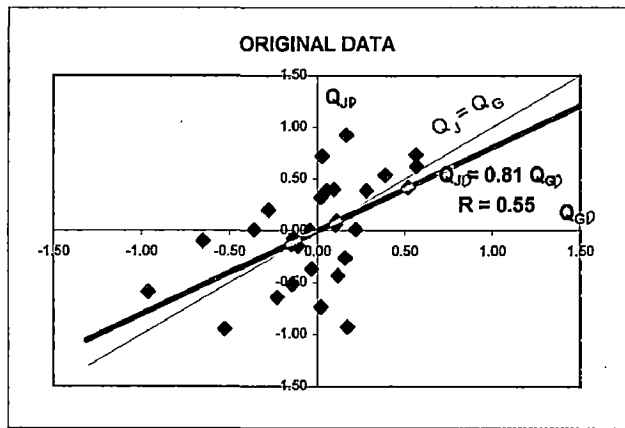
b = 0.90

r = 0.97

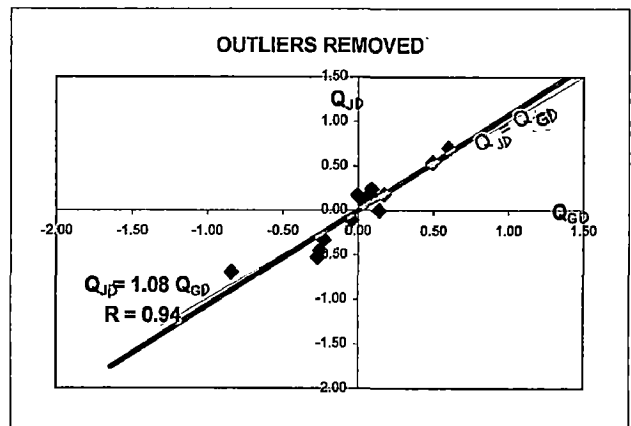
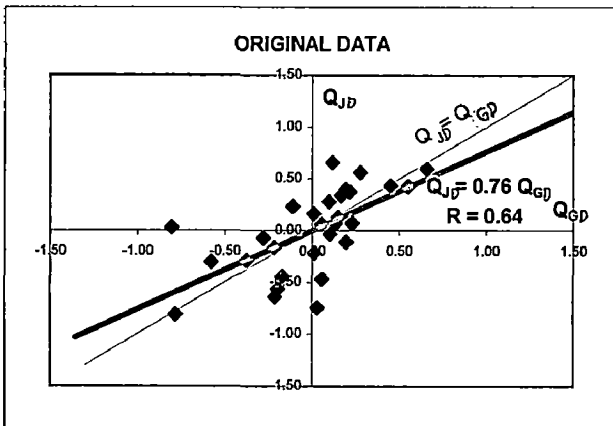
Y = 0.90 X



1st TEN-DAILY DISCHARGE FOR JULY (1976 - 2006)



2nd TEN-DAILY DISCHARGE FOR JULY (1976 - 2006)



3rd TEN-DAILY DISCHARGE FOR JULY (1976 - 2006)

Fig. 2B

PLOT OF 10-DAILY OBSERVED DISCHARGE OF GOLA RIVER AT JAMRANI DAM & GOLA WEIR (BY LOG DEVIATION REGRESSION) FOR MONTH OF JULY



APPENDIX – C

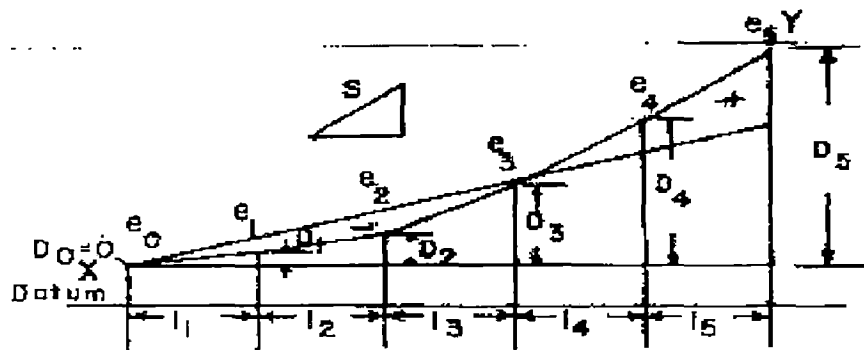
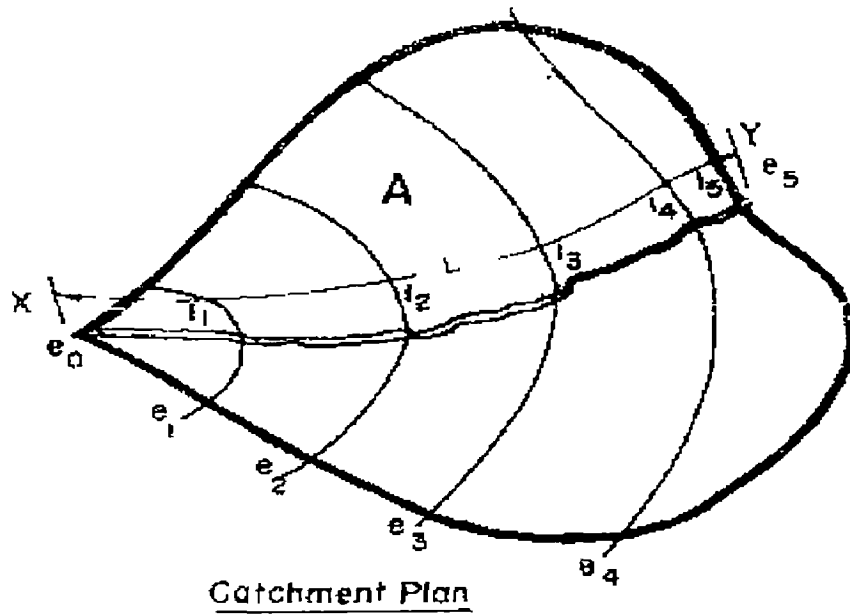
SUPPORTING TABLES & GRAPHS

TABLE-3C $K_z = F(C_s, T)$ FOR USE IN LOG-PEARSON TYPE III DISTRIBUTION

Coefficient of skew, C_s	Recurrence Interval T in years						
	2	10	25	50	100	200	1000
3.0	-0.396	1.180	2.278	3.152	4.051	4.970	7.250
2.5	-0.360	1.250	2.262	3.048	3.845	4.652	6.600
2.2	-0.330	1.284	2.240	2.970	3.705	4.444	6.200
2.0	-0.307	1.302	2.219	2.912	3.605	4.298	5.910
1.8	-0.282	1.318	2.193	2.848	3.499	4.147	5.660
1.6	-0.254	1.329	2.163	2.780	3.388	3.990	5.390
1.4	-0.225	1.337	2.128	2.706	3.271	3.828	5.110
1.2	-0.195	1.340	2.087	2.626	3.149	3.661	4.820
1.0	-0.164	1.340	2.043	2.542	3.022	3.489	4.540
0.9	-0.148	1.339	2.018	2.498	2.957	3.401	4.395
0.8	-0.132	1.336	1.998	2.453	2.891	3.312	4.250
0.7	-0.116	1.333	1.967	2.407	2.824	3.223	4.105
0.6	-0.099	1.328	1.939	2.359	2.755	3.132	3.960
0.5	-0.083	1.323	1.910	2.311	2.686	3.041	3.815
0.4	-0.066	1.317	1.880	2.261	2.615	2.949	3.670
0.3	-0.050	1.309	1.849	2.211	2.544	2.856	3.525
0.2	-0.033	1.301	1.818	2.159	2.472	2.763	3.380
0.1	-0.017	1.292	1.785	2.107	2.400	2.670	3.235
0.0	0.000	1.282	1.751	2.054	2.326	2.576	3.090
-0.1	0.017	1.270	1.716	2.000	2.252	2.482	2.950
-0.2	0.033	1.258	1.680	1.945	2.178	2.388	2.810
-0.3	0.050	1.245	1.643	1.890	2.104	2.294	2.675
-0.4	0.066	1.231	1.606	1.834	2.029	2.201	2.540
-0.5	0.083	1.216	1.567	1.777	1.955	2.108	2.400
-0.6	0.099	1.200	1.528	1.720	1.880	2.016	2.275
-0.7	0.116	1.183	1.488	1.663	1.806	1.926	2.150
-0.8	0.132	1.166	1.448	1.606	1.733	1.837	2.035
-0.9	0.148	1.147	1.407	1.549	1.660	1.749	1.910
-1.0	0.164	1.128	1.366	1.492	1.588	1.664	1.880
-1.4	0.225	1.041	1.198	1.270	1.318	1.351	1.465
-1.8	0.282	0.945	1.035	1.069	1.087	1.097	1.130
-2.2	0.330	0.844	0.888	0.900	0.905	0.907	0.910
-3.0	0.396	0.660	0.666	0.666	0.667	0.667	0.668

TABLE-4C AREAL TO POINT RAINFALL RATIOS (%)

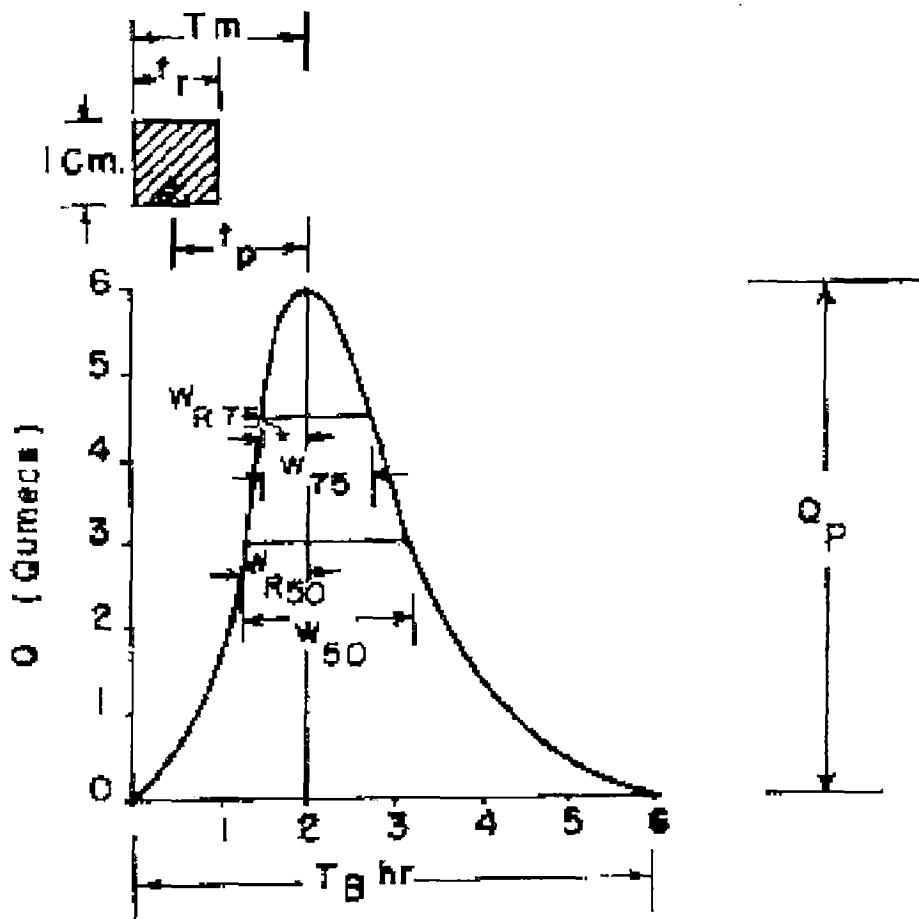
Area (Km ²)	1 hr.	3 hr.	6 hr.	12 hr.	24 hr.
50	88	92	95	96	98
100	80	88	91	94	96
150	73	84	88	92	94
200	68	81	86	90	93
250	63	78	85	89	91
300	60	76	84	88	90
350	-	74	83	87	89
400	-	73	82	86	88
450	-	73	76	85	87
500	-	72	80	84	86
600	-	-	-	82	85
700	-	-	-	80	83
800	-	-	-	79	82
900	-	-	-	78	81
1000	-	-	-	77	81
1200	-	-	-	76	80
1400	-	-	-	-	79
1600	-	-	-	-	78
1800	-	-	-	-	77
2000	-	-	-	-	77
2500	-	-	-	-	77



$$S = \frac{\sum_{i=1}^n (D_i + D_{i-1}) l_i}{L^2}$$

- Where :
- S = Equivalent stream slope (m/km)
 - L = Length of longest stream course (km)
 - $I_i = I_1, I_2, I_3, \dots, I_n$ = Segment lengths (km)
 - $e_i = e_1, e_2, e_3, \dots, e_n$ = Contour elevations (km)
 - $D_i = D_0, D_1, D_2, \dots, D_n$
 = $(e_0 - e_0), (e_1 - e_0), (e_2 - e_0), \dots, (e_n - e_0)$ (m)
 - A = Catchment Area (km^2).

Fig. 1C Physiographic Parameters



U.H.	=	Unit Hydrograph
t_r	=	Unit Rainfall Duration adopted in a specific study (hr)
T_m	=	Time from the start of rise to the peak of the U.H. (hr)
Q_p	=	Peak Discharge of Unit Hydrograph (cumecs)
T_p	=	Time from the centre of rainfall excess duration to the U.H. Peak (hr)
W_{50}	=	Width of the U.H. measured at 50% of peak discharge ordinate (hr)
W_{75}	=	Width of the U.H. measured at 75% of peak discharge Ordinate (hr)
W_{R50}	=	Width of the rising limb of U.H. measured at 50% of peak discharge ordinate (hr)
W_{R75}	=	Width of the rising limb of U.H. measured at 75% of peak discharge ordinate (hr)
T_B	=	Base width of Unit Hydrograph (hr)
A	=	Catchment Area (Km^2)
q_p	=	$Q_p / A =$ cumec per Km^2

Fig. 2C Unit Hydrograph Parameters

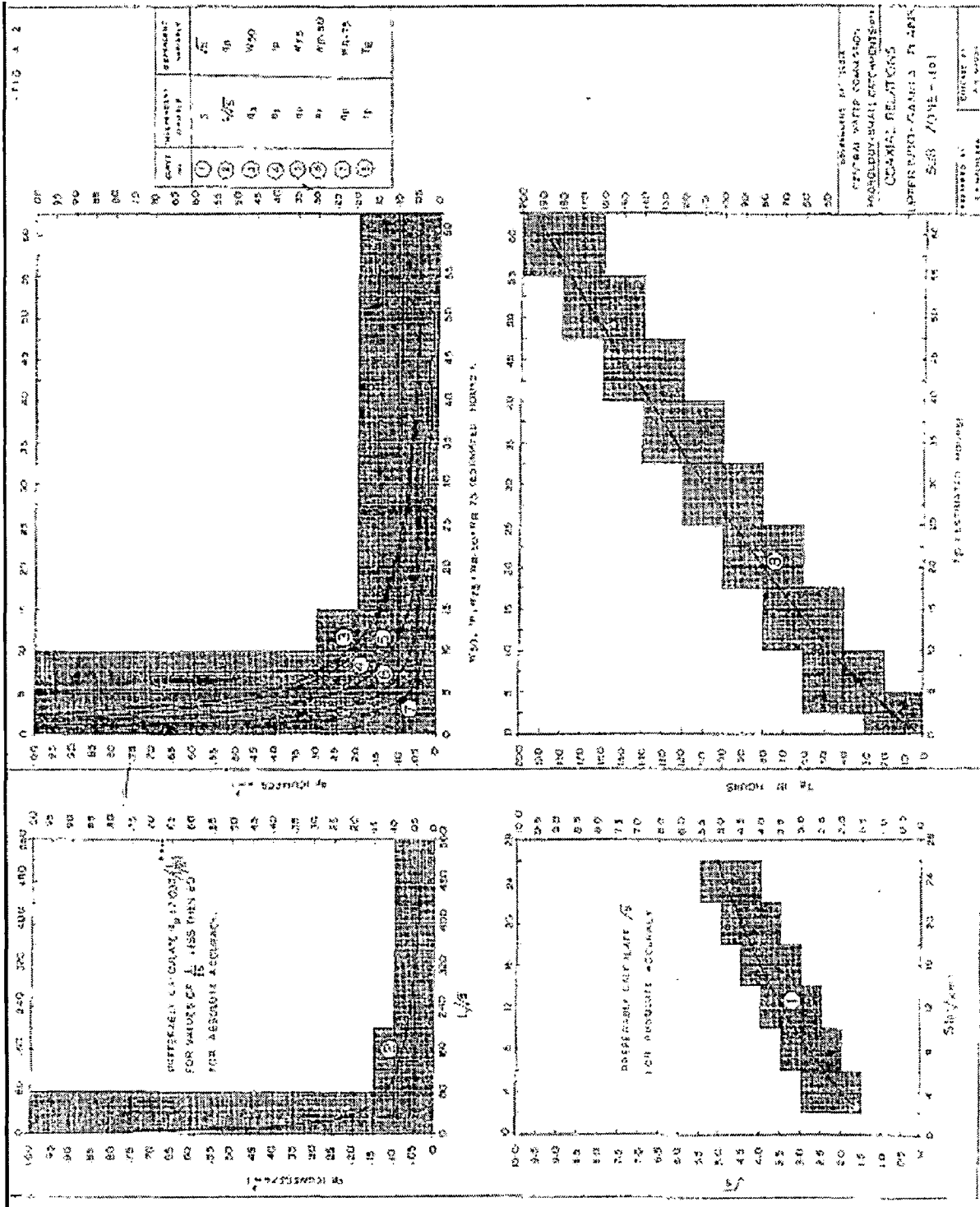


Fig. 3C Coaxial Relations Graph

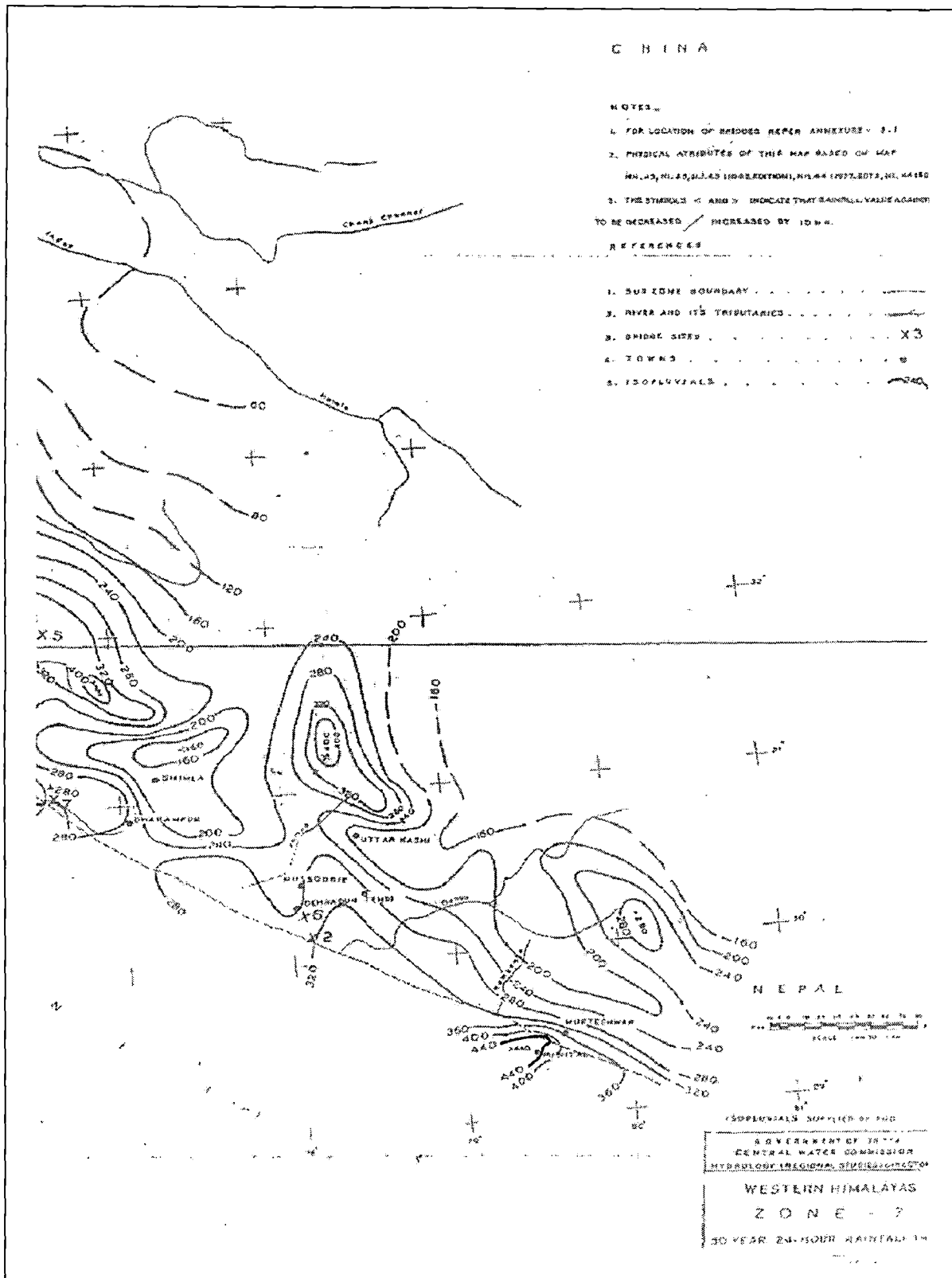


Fig. 4C Isopluvial Map for 50 Year – 24 Hour Point Rainfall (mm)

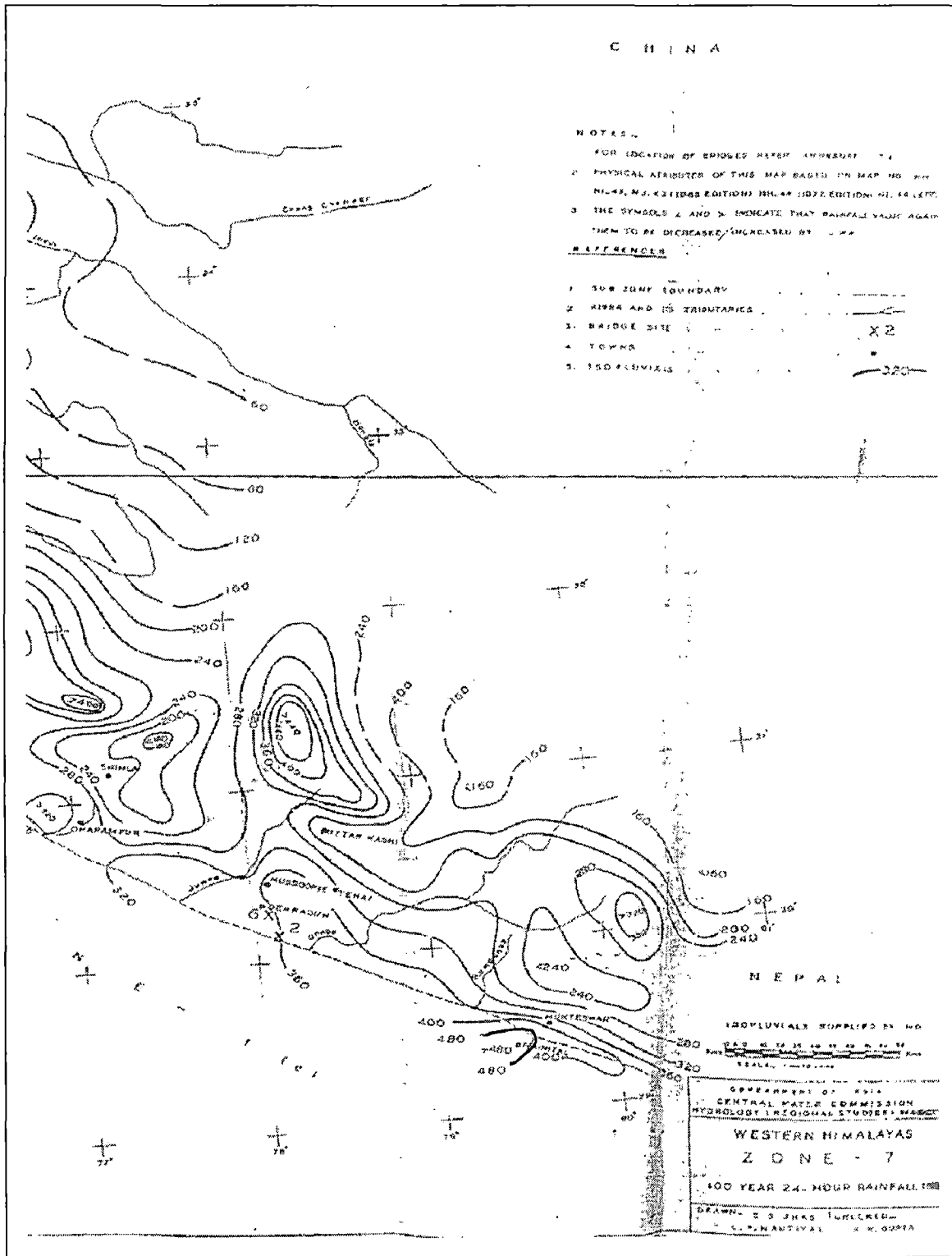


Fig. 5C Isopluvial Map for 100 Year – 24 Hour Point Rainfall (mm)

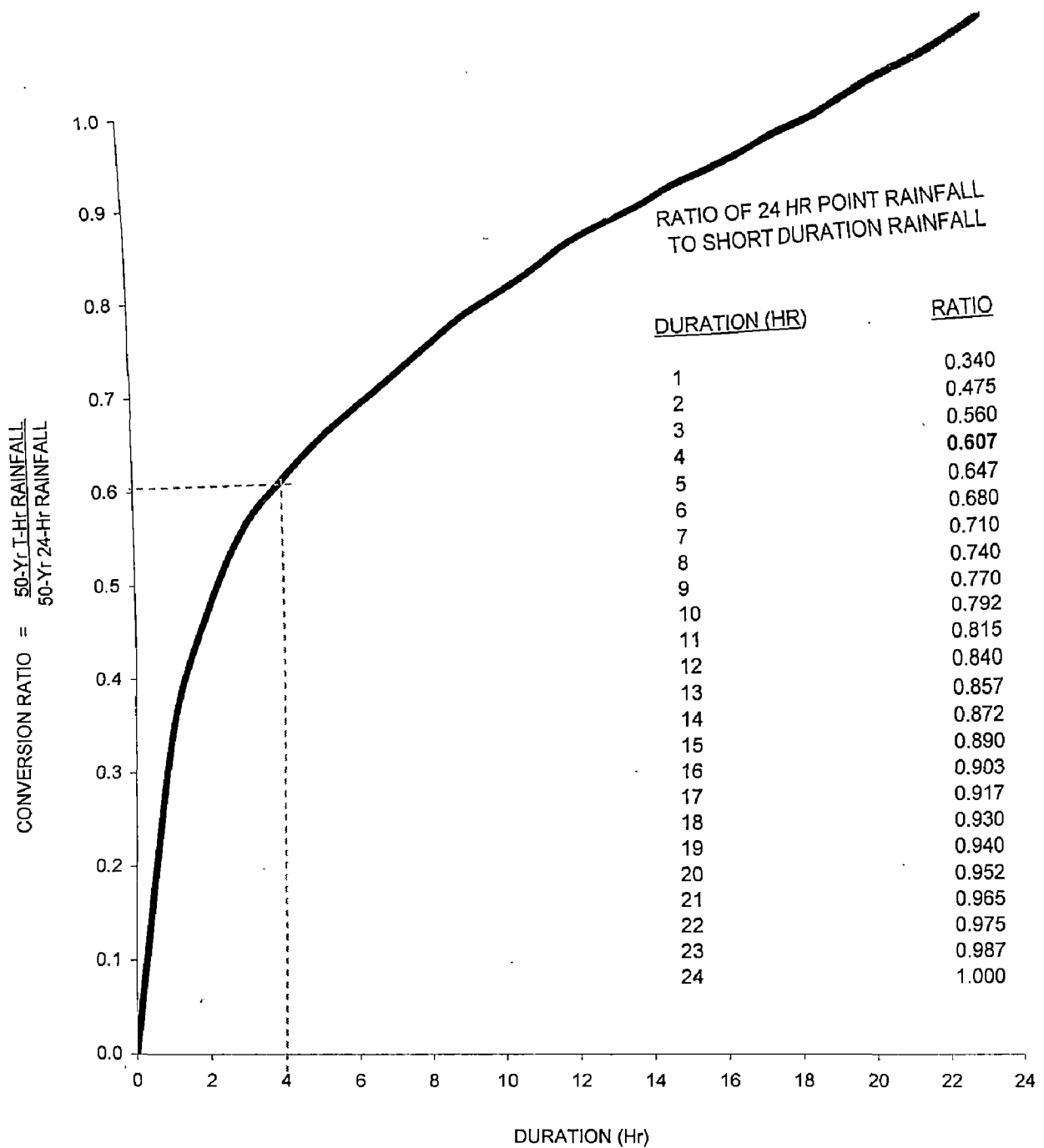


Fig. 6C Design Storm Duration (T_D) vs Ratio

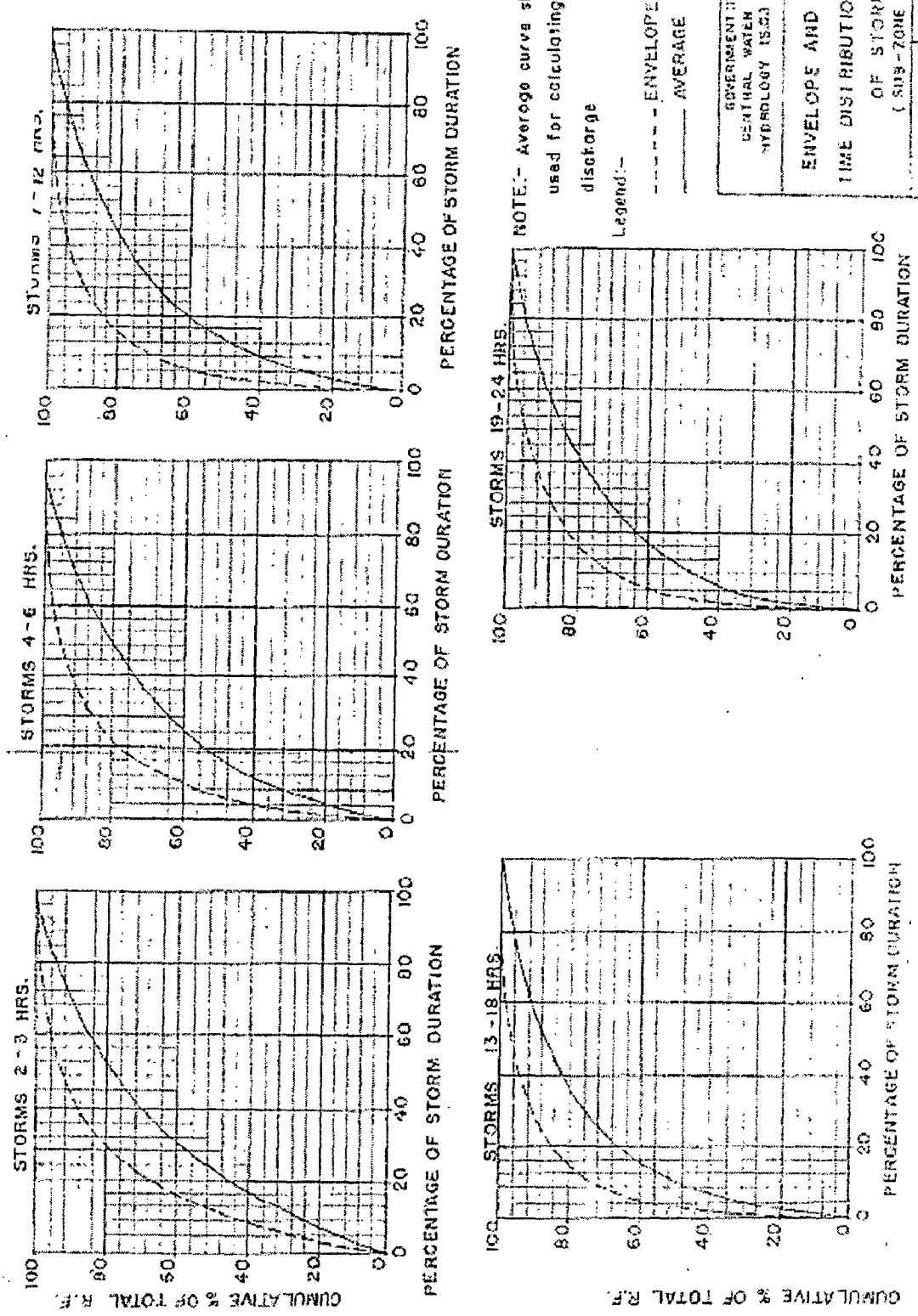


Fig. 7C Time Distribution Curves of Storms