

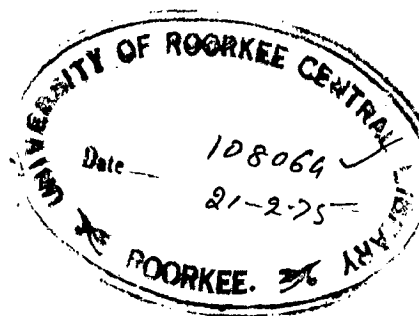
# A REVIEW OF ADEN WATER SUPPLY

A DISSERTATION

*Submitted in partial fulfilment of  
the requirements for the M.E. (WRD) Degree*

By

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ABBREVIATIONS

- A.S.C. = Public Corporation for ASIA
- P.S.C.P. = Public Corporation for ASIA (C) P
- S.S.S. = South Arabian Miner (approximately equal to M)
- S.S.S. = South Sudan Miner ( same as S.S.S.)
- S.S.S. = State Star Level
- S.S.S. = Top Water Level
- M.G.D. = million gallons p. d. day
- M.C.M. = Million cubic metres
- P.p.m. = parts per million
- U.N.F. = United Nations Development Programme
- R = Responsibility
- S = Sustainability

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## SUMMARY

A piped water supply system was started in Aden in 1929 when the first borehole was successfully completed at the Sheikh Uthman headworks. In the following two decades several more boreholes were drilled at the same headworks, which by 1950 were supplying nearly two million gallons per day. Aden water supply has increased many fold since then and the consumption nowadays is about 10 million gallons per day, about 80 per cent of which is raised at the Mir Bahir headworks which were developed from 1956 onwards. The other headworks in the system are at Mir Ahmad. These were commissioned in 1954 to supply water to the oil refinery town of Little Aden.

This dissertation attempts to present a systematic review of the Aden water supply. The first chapter presents a background of the Republic of Yemen in terms of its location from the view points of geography, topography, hydrology, geology and economy.

The second chapter describes the location of Aden, its climate, the growth of its population over the last a century and the history of water supply of Aden.

The third chapter describes the Mir Bahir catchment area, from the ground water resources of which all the Aden and Little Aden supplies are withdrawn. The aspects of topography, rainfall, drainage, floods and geology are described.

The fourth chapter goes into the details of the ground water reservoir. The depths of the ground water and the fluctuations in levels are discussed. The Aden water supply withdraws from the reservoir about 92.5 MCM per annum whilst the recharge is estimated to be of the order of 140 MCM per annum. However water is also pumped from this reservoir for irrigation and it is planned to withdraw about 30 MCM annually for this purpose. This is likely to affect the levels in the Aden supply wells. The salt content of the Aden supply water is also discussed in this chapter.

The fifth chapter describes the Aden and Middle Aden water supply systems in outline. Water from the Red Sea through the Suez Canal supplies the towns of Sana'a, Aden, Hodeidah, Al Hudaydah, Al Hudaydah, Al Hudaydah, the village of Al Hudaydah, and the shipping supply stations at Middle Hodeidah, Al Hudaydah water supplies Middle Aden, the Middle Aden official known as the District Administration, and the villages of Al Hudaydah, Al Hudaydah, Al Hudaydah and Al Hudaydah.

The last chapter discusses the difficulties of the Aden water supply system, as well as the high population for water, which is also responsible for water supply systems throughout the country. Many other organizations in the Republic, the P.A.S. is suffering from financial difficulties, as a result of the economic crisis through which the country is passing. The P.A.S. is also suffering from a acute shortage of qualified personnel, from organization and planning



difficulties and from a number of technical problems. Some of these shortcomings are discussed, and in some cases remedies are suggested based on the experience of the writer in the Adam Water Authority and the Government Administration in Aden.

## CHAPTER - I

### A BREVITARY OF THE COUNTRY

#### 1.1. General:

The Peoples Democratic Republic of Yemen occupies the northern part of the southern coast of the Arabian Peninsula. It lies between latitudes  $12.5^{\circ}$  north and  $19^{\circ}$  north and longitudes  $49.5^{\circ}$  east and  $53^{\circ}$  east. The country is bounded by the Yemen Arab Republic on the north-west, by the sand sea of the Rub-al-Khali in Saudi Arabia on the north, by the States of Kuwait and Oman on the east, and by the Gulf of Aden on the south. The Republic has an area of about 125,000 sq. miles. It comprises a coastal plain varying in width between 5 and 40 miles leading to steep bare rocky mountain ranges rising to about 6000 feet above sea level.

The country gained its independence from British rule in November 1967. Constitutionally, the Republic is now divided into six governorates or regions. The first governorate consists of the capital, Aden, with a number of islands plus the islands of the Republic. The other governorates are in a numerical order with the second governorate furthest west and the sixth furthest east.

#### 1.2. Legislation:

No census has been held in the Republic except in Aden where the last census was held in 1955. A general census is being planned for to be held in a year or two. The

population is estimated by the Central Statistics Office to be about 1.475 million of which almost 300,000 inhabitants live in Aden. The population is irregularly distributed with an average density of about 4.5 people per square km. The population distribution is tabulated by the Central Statistics Office, Aden as in Table-1 below:

Table-1

Population Distribution

Governorate	Area sq.km.	Days	Population	Days sq./sq.km.	
First	7,000	2	301,000	20.6	63.0
Second	13,000	6	266,000	16.0	17.0
Third	21,000	7	270,000	19.6	14.0
Fourth	70,000	22	163,000	11.0	2.6
Fifth	123,000	33	633,000	27.5	3.0
Sixth	69,000	27	60,000	5.5	0.8
<b>Total:</b>	<b>323,000</b>	<b>100</b>	<b>1,473,000</b>	<b>100.0</b>	<b>4.5</b>

1.3. Geography and Climate

The western part of the Republic has a north-south width of about 30 to 37 miles. The coastal region has a slight land slope. However, the northern area of this western part contains the highest mountains in the Republic (altitude of up to 6000 feet). Moreover, the country is also wider with a north-south width of about 300 km. An east-westwardly

waterways divides this part into two precipitation areas, one draining into the Indian Ocean while the other into the Arab-el-Bahri. The low and high features of the country may be summarized as follows :

A. Low Features:

- (1) The coastal plains varying up to about 40 km. in width and extending all along the coast and incorporating the deltas of the rivers.
- (2) The inland alluvial plains around Koyun, the Arab-el-Bahri and the western catchment of the Arab-el-Bahri.
- (3) The valleys of the rivers in the western part of the country and the canyon to the east.

B. High Features:

- (1) The mountains of the west
- (2) The escarpment running from the mountains of the west almost to the Arab-el-Bahri valley and the high lands north of it.
- (3) The plateau of the fifth government with the outlying masses in the Arab-el-Bahri and Arab-el-Bahri catchments.

C. Climate:

The country lies within the monsoon belt. There are two distinct seasons; a warm season from May to September and a cool season from October to April. The coastal climate is warm and is rainy, for instance, the temperature varied normally from a minimum of 6° (43° F) to a maximum of 36° (97° F). In the plateau and mountain regions it is

cooler with greater diurnal range. For instance, Helsinki, which is about 7000 ft. above sea level, has a range from  $28^{\circ} F$  ( $-2^{\circ}C$ ) to  $65^{\circ} F$  ( $31^{\circ}C$ ). A south-westerly wind blows during the warm season, and a north-easterly wind blows during the cooler season.

### 1.5. Rainfall:

The available records of rainfall for the country are very scanty as very few stations exist for recording rainfall. Moreover, the records available from most of these stations are incomplete. Now of these stations are in the mountainous areas where the rainfall is significant. The average rainfall in the coastal low lying areas like Aken and Alton (3rd governments) is less than 2" per year, while in Jyvan and Jyvan (both in the 9th governments) it is only about 2 1/2". At high elevations, where the precipitation is far more significant, the scanty available records indicate for instance an average annual rainfall of about 12" at Jyvan (3rd governments) which is at an elevation of 6000 feet, and about 14" at Jyvan (2nd go. govts) which is at an elevation of about 5000 feet. The map of mean annual rainfall in the country which was prepared by the Department of Agriculture and Irrigation, Alton in 1971 is included as Figure 1.

For computation purposes, the following average figures have been suggested as the values obtained were

(1) p.9:

Elevations over 2000 m	400 or 600 mm
" 1000 to 2000 m	150 mm
" below 1000 m	40 mm

A large portion of the rainfall at high elevations runs down as surface runoff into broad alluvial filled valleys or low lying alluvial fans sometimes flooding them and replenishing the ground water reservoir. Underground flow in the wide alluvium to low lying coastal fans is of considerable importance. The wide river (5th government) and the wide (5th and 6th governments) are the only perennial small rivers, there is also water flowing perennially in several wadis like the wide (3rd government), the wide (2nd government), and the wide (4th government).

Most of the rain falls in the spring and in the summer and early autumn periods. The heavier rainfalls occur during the winter season and the lighter rains fall during the March-May period. The rain frequently occurs as drizzles making it difficult for the farmers to make use of the water.

1.6. Geology

The rock systems are discussed in some detail by Bear (1) pp.6 to 25. The geology of the western part of the country known before independence as the Kingdom of North Arabia which incorporated the 1st, 2nd, 3rd and most of the 4th governments, is discussed by Robinson (2). The location of the various rocks may be summarized as follows:-  
A. The Recent, Pleistocene and Miocene rocks:

The alluvial rocks are confined to the wide beds, the coastal alluvium, and the western part of the wide

Madrasat catchment except for the sand-dunes on the low hills and the sand-reef of the 'Ab-al-Jabal.

The intrusive rocks occur in relatively small patches on the coast line near Aden and west of it. Also they occur over a large area in the north and east of the Hadramout catchment, in the west and north of the Hadramout catchment, in the half-day coasted catchment area, and in small patches along the coast well to the east of Aden, the capital of the 5th governorate, and in the Hadramout.

#### D. The Oligocene-Miocene Series:

The most important location is along the coast just east of Aden. Small patches occur west of Aden and in the Dayr catchment.

#### E. The Pliocene Series:

These cover the greater part of Madrasat. A small area is found in the extreme east of the Hadramout catchment area.

#### F. The Tertiary and Quaternary Series:

The sedimentary rocks are found everywhere and underlying the Hadramout and its tributaries and also along the coast from east of Aden westward to the catchments of the Hadramout and the Hadramout. Also small patches are found in the Hadramout and the Hadramout.

Intrusive rocks are found along the Hadramout which rocks Aden and west of the Hadramout catchment

area is underlain by the Cretaceous lavas except for that of the Haddi Abraia. Another area occupies the extreme north-west corner of Djibouti, in the Hadj governorate.

#### 6. The Jurassic System:

These are found around Adowa in the Haddi Abraia catchment. Patches occur in the coastal hills behind Hadda. They are also found in places in the Haddi Abraia, Hadda, Abura and Hadda.

#### 7. The Mio-Sandbian and Igneous rocks:

These are found in the coastal strips around Adowa, in the southern parts of the Haddi Abraia and Hadda, and underlying parts of the catchments of the Haddi Abraia, Hadda, Abura and Abraia. They underlie most of the western part of the Haddi Abraia catchment area. Granite occurs near Adowa, Hadda and Hadda.

A geological map of the Republic (scale 1:2,000,000) is shown in Figure 2.

#### 1.7. Economy:

Before independence the area was a flourishing free port, with an economy depending on services rendered to the British Military Base, British migratory population, post services and bank rates, extensive trade and commerce. The other governments lived at near subsistence level on agriculture, mainly of cotton and grain, and on fisheries.

The country has been suffering an economic depression since 1967, and, according to the Central Statistics Office



In Aden, the GDP (gross national product) has decreased from 167.4 million Ad\$ (local Arabian dollar, approximately equal to \$) in 1966, to 98 million Ad\$ in 1967, and has reached 73.9 million Ad\$ by 1969. The depression was caused mainly by the following factors:

- (i) The closure of the Suez Canal in 1967 leading to the diversion of shipping from Aden.
- (ii) the withdrawal of the British base which was providing employment to some 14,000 persons.
- (iii) the termination of the British budgetary aid which had arisen to 22.9 million Ad\$ for the year 1967/68.

The government resorted to very stringent measures to cope with the crisis. Firstly, government expenditure was cut down by more than 50% from 1968. Secondly, all banks and foreign firms (except the local oil refinery and the banking companies) were nationalised in 1969, with the idea of using the profits from these concerns in an economic development plan. In the following years the government seized all farm tractors in the country, fishing boats, hotels, cinemas, clubs, and most of the houses. The government also turned to friendly countries and international organisations for loans for the first 3-year economic development plan.

CHAPTER - II

THE FEDERAL GOVERNMENT OF THE STATE OF ADEN

2.1. Aden

Aden, the capital of the People's Democratic Republic of Yemen, is situated in the third governorate in the south western part of the country on the Gulf of Aden at a distance of about 100 miles from the Straits of Bab-el-Mandeb on the southern coast of Arabia. It lies on longitude 45° 10 E and latitude 12° 47' N. The area of Aden is about 75 sq. miles. Before independence Aden was a British colony as distinct from the other five governorates which previously constituted the Eastern and Western Aden Protectorates.

Aden comprises:

- (1) the peninsula on which are situated the old main town known as Crater, the well known harbour known as South of Aden's coast, and the new harbour area known as Aden,
- (2) The isthmus known as Socatra,
- (3) the area of land extending to the Little Aden Peninsula. The towns of South Oman, Al-Macarra, Aden, Al-Mahad and the village of Al-Mahad on this area,
- (4) The Little Aden peninsula on which lies the old refinery. The villages of Al-Mahad and Al-Mahad are on the eastern side of this peninsula.

The Aden and Middle Aden peninsulas are rocky and of volcanic formation, Jebel Shuman, the summit, being 1725 feet high. Dwellings and buildings are constructed on the ridges of the mountains and in the valleys between them wherever the land is level. Greater Aden includes an extinct volcano on the east of the peninsula at a distance of about 5 miles from the harbour which lies on the western part of the peninsula.

## 2.2. Climate

There are practically only two distinct seasons during the year, a very warm season and a cooler season each lasting about 6 months. The temperature in Aden varied normally between  $61^{\circ}\text{F}$  and  $106^{\circ}\text{F}$ . The annual rainfall is extremely scanty, 2" to 3", and in some years no rain falls at all, but as much as 10" have been known to fall in one year.

## 2.3. Population

The population of Aden has fluctuated throughout the ages with its rise and fall. The rise and decline of Aden may be linked with the rise and decline of the Yemeni ports, particularly the port of Sana'a and to a much lesser degree the port of Ibb. For instance, Aden was flourishing around 600 B.C. and again around 1300 A.D. under the Ibbids, and had a sizeable population in those days, although no population census is so officially made until British occupation. By the time the British conquered Aden in 1839, it had dwindled to a small fishing village and the

census taken that year revealed that the population was 1927 of whom 600 were locals. The population continued to increase sharply as can be seen from the results of the six censuses taken thereafter (3).

Table-1

Population: Summary of Aden

<u>Year</u>	<u>Population</u>
1856	28,736
1901	63,020
1911	45,211
1931	60,380
1946	80,576
1955	190,451

The population in 1965 was estimated to be 225,000. However, the population began to decrease after 1965 due to the political uncertainty of the future of the area, and again there was a decrease at Independence mainly due to the withdrawal of the British force, but there was a heavy increase afterwards due to the movement of population from the up-country into Aden. The population at the present is between 270,000 and 300,000 people.

2.5. Historical:

A civilisation based on the management of water flourished in south-west Aden as possibly as far back as the first millennium before Christ in the days of the legendary Queen of Sheba. The remnants of the historically famous Harab Jan, the bursting of which spawned a civilisation

in the Northern Euphrat around that era, can still be seen. On the other hand in Southern Euphrat, the most important archaeological features are the Warka Tanks. There are 16 tanks known, constructed in a series one below the other so that when the highest is full it overflows into the next lower tank. The approximate capacities of some of these tanks were estimated as follows (5):

Table-1

Capacities of Some of the Warka Tanks

<u>Name of Tank</u>	<u>Capacity in Gallons</u>
The Upper Tank	1,000,000
Tank below Main Wall	60,000
Small Tank in Aqueduct	150,000
Small tank right and left of aqueduct	500,000
Capital Tank	1,000,000
Playfair Tank No. 17	2,000,000

The tanks seldom ever became filled but it is recorded that they were filled in 1857 and again in 1907 and probably a few other times in between. It is still not certain to whom the construction of these tanks is to be attributed, but most probably they were constructed over several eras starting from pre-Christian times.

The population of Uruk depended on them as well as on open wells for their water supply at one time. Mention of some 60 open wells and a few cisterns in Uruk was made by some historians.

It is useful to follow up the historical development of the existing water supply system to be able to assess the present problems and their possible solutions.

The rapid increase in population which took place after British occupation resulted in an acute shortage of water, but nothing was done to remedy the situation until 1867 when an aqueduct was constructed to convey water from two open wells at Madhāh Ghānā to Alon. Also, a distillation plant was erected. (A few privately owned small distillation plants were also erected about 50 years ago and sold water to the public through the owners of canal-driven tubwells).

Up to 1925 several reports and schemes were prepared to upgrade the system which had become seriously inadequate long before that date but none of these schemes was followed through. Between 1925-26 a deep borehole was sunk to 1700 feet in Madhāh Ghānā but had to be abandoned after the drilling pipe jammed in, although a small artesian flow had been found. In 1928-29 a 120 foot deep shallow borehole was successfully completed, and a fresh water supply was delivered to Madhāh through the Royal Air Force 6" main previously laid to replace the old aqueduct. Between 1929 and 1931 three more bores were sunk in Madhāh Ghānā. In 1931 the Government of Bombay approved the allocation of 8 lakhs of Rs. to cover the cost of these bores and a piped distribution system. Between 1931 and 1950 ten more bores were sunk in Madhāh Ghānā. The population

had increased by 1950 to 110,000 and the demand to 1,900,000 gallons per day. The next expansion of this system was completed in 1954 at a cost of £ 195,000 increasing the daily output to about 3 million gallons. This consisted of five additional boreholes in the main column, a 9" diameter main to Icthus where a new pumping station and a high level reservoir were proposed together with new feeder mains to Uster and Lumb. A new shipping supply along the main was also proposed to relieve some of this heavy load.

In the same year i. e. 1954 the Little Acre water supply system was commissioned at a cost of £ 700,000. The development of this system has been directly related to the development of the D.F. railway. The system consisted of 12 shallow bore holes on 3 parallel lines running east-west a little to the west of the village of Sir Alford. The system was designed to supply up to 2.5 m.g.d. but the demand at that time was only for just over half that quantity.

In 1956 the 520 Main Boreholes was commissioned to supply Icthus supplying water. This consisted at that time of 16 shallow boreholes 10" in diameter, and having a maximum output of 3.5 m.g.d. These wells are on an east-west line at 1000 foot intervals situated between the main Alford and the main to the north, the two distributaries of the main system, and at a distance of about 7 miles to the south of the town of Leeds, the capital of the East Yorkshire. 15 S.F. counterbores were also installed in them.

In 1961 the second line of Mir Amir Agha water supply wells was constructed. This consisted of 14 shallow type boreholes on a line parallel to the previous line and 2000 feet to the north of it. Numerous pumps of 30 H.P. were installed in these boreholes.

In 1966 two larger boreholes were drilled between these two lines with the idea of replacing four boreholes, two from each line, with one of these bigger holes. However, a trial in which the writer participated to run a 100 H.P. submersible pump in one of these holes showed that the permeability of the aquifer was not sufficiently high, for the pump began to pump up air within a few seconds from switching on. Later on 30-H.P. pumps were installed in these boreholes.

A similarly larger bore, 10" diameter, was drilled in the Mir Amir field and was unsuccessful for a 100-H.P. pump. This well was later on handed over to the Department of Irrigation.

In 1971 two more wells were drilled in the Mir Amir field bringing the total number of wells in that field to 32 wells.

At the present the total number of wells that are or can be made operative in the Agha water supply system is 8 boreholes at Mir Amir, 32 boreholes at Mir Amir, and 12 boreholes at Mir Amir.



## CHAPTER - XIX

### THE JUBA RIVER CATCHMENT AREA

#### 3.1. The Source:

All of the water of the Juba supply emanates from the underground water resources of the delta part of the Juba catchment area. This includes the Sheikh Osman borewells, the Al-Faris borewells and the Al-Awad borewells.

#### 3.2. Location and Dimensions:

The delta of the Juba River falls between latitudes  $12^{\circ} 45' N$  and  $13^{\circ} 15' N$  and longitudes  $43^{\circ} 7' E$  and  $43^{\circ} 00' E$ . It has a triangular shape, the base, about 35 km long, lies along the Gulf of Aden, and the apex is near the village of Al-Awad which is about 320 metres above sea level. From Al-Awad the plain slopes down to the Indian Ocean about 50 km away. The Juba River catchment area is shown in figure 4.

In addition to the delta, the catchment area includes the hills, which start just north of Al-Awad and are parallel to the coast and which are about 2,500 metres high rising to 3,000 metres at Jebel Umm al-Hadid. The catchment area also includes elevated plains about 2 km wide and extensive, approximately between the Juba River and Al-Awad i.e. a distance of about 15 km.

#### 3.3. Vegetation:

Three kinds feed the delta area:

Table 15Average Annual Rainfall at Stations close to the River

Station	Elevation m. Av.	Rainfall cm/year	No. of observations
Alcañ	27	49	62
Alcañ	1,400	369	5
Alcañ (North Junction)	1,350	527	6

The Andú River receives much of its water from the rains which fall on the mountains north of the border. The rainfall in Ymas is fairly appreciable. ( It is about 300 mm in Ymas, the capital).

For computation purposes, the following rainfalls may be assumed, (5), p. 3.

<u>Elevation m.</u>	<u>Rainfall mm</u>
Over 200, west & central areas	600
Over 2000, centre & north areas	400
1000 to 2000	150
Under 1000	60

The flood seasons are March to May and July to September. Only during very heavy floods do the flood waters of the Andú Alcañ reach the sea near the village of Alcañ. The floods of the Andú River last 10 to 20 hours while the floods of the Andú Alcañ last 5 to 6 hours. The average annual potential discharge in the Andú River is estimated to be about 100 million cubic metres, whilst the flood discharge is about 125 million cubic metres, (5) p. 6. During the

- (1) the Balih, draining down the eastern side,
- (2) the Adrain, draining down the western side, and
- (3) the Tuban, the most important of the three, draining down the central region.

All three wadis flow to the south-east. The wadi Tuban splits into the wadi Al-Sabbir and the wadi Al-Magfir near the village of Mahachaba. The wadi Al-Sabbir is on the west of the wadi Al-Magfir (Magfir in Arabic means large walled canal means wall).

A perennial flow continues in the Tuban between Debat Daman and Inbij except after a succession of dry seasons when the flow continues only in the underlying boulders.

Geor produced the following table, (5) P.2.

Table 5  
Catchment Areas (Sq. Km.)

Elevation in metres	North of Adrain	of Tuban	Al-Sabbir	South of Al-Magfir	Totals
Over 2000	-	2100	-	-	2100
1000 to 2000	75	2090	110	-	2275
Under 1000	375	255	305	1210	2145
<b>Totals:</b>	<b>450</b>	<b>5045</b>	<b>415</b>	<b>1210</b>	<b>7120</b>

3.4. Rainfall and Floods:

Table 5 shows the average annual rainfall at the three stations close to the Balih basin area, (6) P.4.

Table 15Average Annual Rainfall at Stations close to the Sudan

Station	Elevation m. Av.	Rainfall cm/year	No. of obser- vations yrs.
Aden	27	49	62
Rasna	1,400	309	5
Sala (North Sudan)	1,350	521	6

The Red Sudan receives much of its water from the rains which fall on the mountains north of the border. The rainfall in Yemen is fairly appreciable. (It is about 300 mm in Sanaa, the capital).

For computation purposes, the following rainfalls may be assumed, (5) p. 3.

<u>Elevation m.</u>	<u>Rainfall cm.</u>
Over 200, west & central areas	600
Over 2000, centre & north areas	400
1000 to 2000	150
Under 1000	60

The flood seasons are March to May and July to September. Only during very heavy floods do the flood waters of the Red Sea reach the sea near the village of Aden. The floods of the Red Sea last 10 to 20 hours while the floods of the Red Nile last 5 to 6 hours. The average annual potential discharge in the Red Sudan is estimated to be about 100 billion cubic metres, whilst the flood discharge is about 125 billion cubic metres, (5) p. 6. During the

Flood season much silt is carried by the water.

### 3.9. Geology

The geology of the part of the catchment area that lies within the RAB is available and is shown in Figure 4. Alluvial deposits are found in the coastal plains up the land to just beyond the village of Aswanir. Recent lavas occur as isolated hills near the coast while the weathered Cretaceous lavas cover most of the area except the catchment area of the Wadi Abrah which is underlain by Eo-Cambrian rocks. Part of the village of Almad and also in the Wadi Al-Rubir can be seen a patch of granite. In the whole the geology here favours high run-off and low infiltration.

In the catchment basin there is much faulting particularly N-S. Also a number of dykes occur east and north of Al-Mad. Also, north and east of Al-Mad Cretaceous lavas outcrop and to the west can be seen Eo-Cambrian rocks lying upon a granitic basement. North movement resulted in elevation and faulting, after the lavas had consolidated.

Logs of wells drilled in the delta indicate that the sedimentary rocks of the area consist of beds of silt, clays, sands, gravels and boulders, and these beds are of irregular thicknesses. The silts are often 30 to 50 feet thick. At Bahig the logs indicate that beneath the surface silt, boulders and gravel form the bulk of the soils. Near Kattia Almad up to 100 feet consists of clays, sands and lava boulders. A considerable degree of cementation has taken place in the soils of this area mainly by calcium carbonate (Figure 5)

Complete a typical strata log for the Mill Creek area, (7).

### 3.5.1. North Creek Area

The logs of the boreholes in the North Creek headwaters shown in figures 6A & 6B indicate alternating series of clays, fine sands and gravels with the lower layers consisting of cemented sands and the sticky clays. Gravel beds were found at two or three depths particularly at 135 feet and 200 feet below ground level, varying between 3 and 15 feet in thickness and water was struck at both these depths. There is only one deep borehole in this field reaching 1700 feet in depth. This showed a boulder bed 12 feet thick at a depth of 700 feet and a clay layer at a depth of 800 feet. This field occupies an area of about 1400 feet by 2200 feet.

### 3.5.2. Mill Creek Area

The thickness of the top silt layer varies between 15 feet and 60 feet according to the logs of the boreholes which were drilled in 1956 shown in figures 7A and 7B. Below the top layer of silt are layers of boulders with thin beds of clay, sand and gravel extending the full depth of the bore, 250 feet and more.

### 3.5.3. Mill Creek Area

For the top 150 feet there are fine-grained sediments and alternating layers of clay and sand and gravel beds, rather than boulder beds. The logs of the boreholes are shown in figures 8A & 8B.

## CHAPTER - IV

### WATER RESOURCES

#### 4.1. Occurrence and Depth of the Ground Water

Both confined and unconfined aquifers are present in the area. The confined aquifer waters are under some pressure and the piezometric levels of the water are about equal or perhaps a few feet higher than the static level of the water in the unconfined aquifer above. The best available evidence is that from the deep bore No. 1 at South Okara 1740 feet deep. Water was struck at normal depth, however, the rock level from the layers at depths 60', 140', 240' and 700' were 13', 14', 15' and 7' respectively. The prospects of striking artesian flowing water in this area is most unlikely except perhaps in small quantities from isolated lentils, which had built flood level pressures that would soon drop (8). With the existing limited hydrostatic pressures and a strata that is too permeable, only the above mentioned sub-artesian conditions are encountered. Depths of unconfined water are shown in Figure 9.

Essential layers are found in the northern area and down to the third and fourth layers and ensure free movement of the ground water. Fine-grained sediments, silt and clay bands are obvious from the well logs of the area and may suggest that the vertical discharge to the unconfined aquifer is important; however, the general fluctuations of levels in the above boreholes are small, indicating little

Local connection between the confined and unconfined aquifers.

The static water level of the ground water in this area varies from 6 feet to about 120 feet. Near the coast and to seven kilometres from it the static water level is less than 20 feet deep. To the east of this area it is usually 20 to 40 feet deep. Around Al-Hanf and to the west of it and even close to the land the level rises suddenly to more than 60' and also the water is less rather suggests a high permeability layer, possibly coarse gravel.

The static water levels in some of the boreholes at Sheikh Osman, Sir Hanan and Sir Ahmad were found sometimes to be as indicated in Tables 64, 65 and 66 below:-

Table 64

Wells at Sheikh Osman, (1955)

<u>Well No.</u>	<u>Static Level</u>
2	25' 5"
3	26' 9"
4	27' 9"
5	25' 9"
6	25' 0"
7	25' 0"
8	22' 0"
9	30' 0"
10	21' 5"
11	28' 6"



Table 61S.W.L. at Dir. Amir, (1953)

<u>Bore No.</u>	<u>Water Level</u>
1	71'
2	66'
3	70'
4	72'
5	62'
6	62'
7	70'
8	72'
9	70'
10	70'
11	74'
12	73'
13	83'
14	90'

Table 62S.W.L. at Dir. Ahmed, (1951)

<u>Bore No.</u>	<u>Water Level</u>
1	79' 0"
2	75' 6"
3	75' 0"
4	73' 6"
5	69' 0"
6	71' 6"
7	77' 0"
8	75' 0"

The available logs particularly those of the Delta Column boreholes indicate the presence of a number of confined aquifers lying below each other separated by relatively impermeable layers. The main two such layers appear from the Delta Column records to be at about 155' and 200' below ground level.

The Almad boreholes were drilled using a rotary drill and, therefore, the records do not give clear water-supply and geologic information; however, the perforated parts of the casings usually were not with their top 100' to 250' below ground and their bottom at about 100' below ground level.

At the Almad sites no aquifer depths were not collected when the boreholes were drilled, however, the perforated casings were not between 100' and 250'.

Figure 10 shows the well depths in the Delta

(5) Fig. 10.

#### 4.1.1. Fluctuations in the Delta Almad Boreholes

The long period variations of U.S.G. may be studied from the data in Tables 7, and 8) extracted from the statistics kept by the P.W.C., Almad.

#### Table 7

#### Fluctuations in U.S.G. at Delta Column

Well No.	1950	1955	1977
3	25' 0"	31' 0"	27' 0"
6	27' 0"	27' 0"	27' 0"
8	13' 0"	27' 0"	35' 0"
11	20' 0"	27' 0"	65' 0"
19	"	27' 0"	27' 0"

Table 2Fluctuations in U.S.L. at Air Base

<u>Yore No.</u>	<u>1950</u>	<u>1963</u>	<u>1972</u>
2	66'	50'	60'
0	72'	50'	63'
13	60'	-	65'
14	90'	-	100'
19	-	57'	65'
21	-	57'	90'

Assuming that these measurements were reasonably accurate it seems that there are marked long ported fluctuations in rock levels. The levels of 1972 indicate a fairly appreciable drop in U.S.L. This drop is not, however, fully understood but partly explained later (p. 55). Moreover, the levels at Air Base in 1963 are several feet higher than the levels in 1950. Figure II shows the fluctuations in the water levels in the older 16 boreholes at Air Base. Also the following study was case made for that line of boreholes, (5), p. 9:

Table 3Long Ported U.S.L. Fluctuations at Air Base

<u>Date</u>	<u>In. ab. of U.S.L. Below Ground</u>
June 1956	60' to 70'
June 1957	62' to 70'
June 1958	54' to 77'
Dec. 1959	49' to 57'

Now the annual discharge in million cubic metres during the same period was as follows, (5), P. 6.

<u>Year</u>	<u>1957</u>	<u>1958</u>	<u>1959</u>
Flood discharge	59.2	48.0	102.7
Perennial discharge	117.5	91.5	97.7

From these results the relationship between the flood volume and the actual rise in level is not clear, since the rise from 1956 to 1957 was of the order of 2' and that from 1958 to 1959 was of the order of 10' whilst both 1957 and 1959 seem to have been good flood years.

There is no obvious sign of annual fluctuations in the confined aquifer levels; however, in the confined aquifer the levels rise 3 to 7 feet within 10 to 48 hours after floods, and in some areas of deeper post-level rise of about 15' have been reported, but there are wells near Sheikh Ghanem in which the levels have been reported to be unaffected by the floods.

#### 4.2. Shortages of Water Abstracted

Fairly good statistical records have been kept of the quantities of water raised and billed by the P.W.C. since the tubewell supply was started in 1929. The statistics for the last 20 years are fairly complete with only few gaps. As already explained in Chapter 2, population censuses were rare and it is difficult to estimate with accuracy the present population, let alone make future forecasts. Figure 12 is a

graph of water raised, water billed, and population against time. The population curve beyond 1965 is difficult to complete in the absence of more reliable information.

L. Jackson in 1955 plotted graphs of population and consumption against time, (9) App. VII. He estimated a demand of 10 m.g. per day in 1970 and a population of 200,000 to 250,000. Such a situation was in fact reached by 1965. The graphs which were drawn by J. Jennings in 1960 for the Aden Water Supply (excluding Middle Aden) predicted a mean daily demand of 6.9 m.g. by 1965 and 10 m.g. by 1970. (10) App. I Jennings also predicted a mean daily demand of about 1.6 m.g. by 1965 and about 2.55 m.g. by 1970 at Middle Aden, (11) App. A. The prediction for 1965 was quite close with the actual figures higher by only 0.9 m.g.d. Because of the major changes that took place due to independence by predictions beyond 1967 were bound to be inaccurate.

Table 9 shows the water raised at the three locations for the years 1955 through 1970.

With reference to Table 9 it can be seen that the quantity raised at Middle Aden has been decreased from about 1600 m.g. in 1960 to about half that quantity in 1970. This is because this source has been increasingly silted, and because the Sir Hinds source had been developed sufficiently by 1962 to take over the major role with Middle Aden as a supplement with which it is raised before distribution. The water raised at Sir Hinds has risen from 122 m.g. in

Table 9Index 1955=100 in 1955 for 1955-71

YEAR	INDEX 1955=100	INDEX 1955=100	INDEX 1955=100	INDEX
1955-56	927.0	132.0	372.6	1473.6
1956-57	727.0	677.0	403.0	1637.0
1957-58	892.0	683.5	455.2	1701.5
1958-59	935.5	835.1	391.0	2101.6
1959-60	970.0	939.1	412.0	2311.9
1960-61	1103.7	864.5	413.1	2440.2
1961-62	1222.0	507.1	407.9	2579.0
1962-63	453.0	2179.9	413.9	2679.6
1963-64	310.0	2610.5	436.0	2815.3
1964-65	363.0	2693.3	541.7	3520.6
1965-66	362.3	2731.7	602.5	3903.5
1966-67	734.5	2803.1	726.9	4444.5
1967-68	523.1	2535.7	636.7	3720.7
1968-69	477.0	2300.1	535.2	3513.9
1969-70	562.1	2572.5	551.0	3692.0
1970-71	494.0	2643.5	604.6	3005.7

1955-56 to a peak of 2738 m.c. in 1963-64 and then decreased a 28000 during the following three years mainly due to the withdrawal of the British Base, but again started to increase after that to about 2000 m.c. in 1970-71 due to the movement of population from rural parts into the Capital, water demand at Sir Ahmed increased gradually from 400 m.c. per year during the first few years of operation of the system to a maximum of 787 m.c. in 1966-67 and after some drop following Independence increased again to 600 m.c. in 1970-71.

The monthly variations in demand during the year may be studied from Table 10 which shows that the demand increases appreciably during the summer.

Table-10

Distribution Water Requirements (1967-68) - Volume 1

Month	Monthly Demand	Maximum Demand
April 1967	56,552,000	58,632,200
May 1967	65,564,000	53,777,000
June 1967	59,725,200	32,733,900
July 1967	64,735,000	35,615,500
Aug. 1967	61,471,700	33,824,000
Sept. 1967	58,455,000	30,692,100
Oct. 1967	52,291,000	33,439,000
Nov. 1967	48,356,000	29,265,000
Dec. 1967	48,455,700	26,543,300

Contd.

January 1968	66,625,000	25,147,600
February 1968	65,169,300	25,637,900
March 1968	59,495,400	32,521,200
<b>Total:</b>	<b>191,289,700</b>	<b>83,306,700</b>

From Figure 12 and from Table 11 it is seen that the loss i.e. the difference between water raised and water billed for the years between 1955 and 1968 also between 1955 and 1968 and has an average value of 21%. A maximum of 30.7% was reached in 1966-67, a year during which political violence was approaching a maximum and curfews were often enforced even on the waterworks staff, resulting in difficulties in the collection and checking of water.

Table-11

Water billed in Million Gallons:

Year	Water raised	Water billed	Loss %
1955-56	1613.6	1267.0	19.0
1956-57	1637.0	1434.0	22.0
1957-58	1701.0	1512.6	23.0
1958-59	2101.6	1772.0	17.0
1959-60	2311.6	1931.5	16.0
1960-61	2442.2	209.6	17.1
1961-62	2597.5	2156.1	17.1

Contd.



1962-63	3077.4	2409.8	69.0
1963-64	4134.6	2372.3	30.7
1967-68	3720.7	2790.5	23.0

### 4.3. Water and the Potential of the Damages

The average annual precipitation on the catchment is given in Table 12, (5), p. 19

Table 12

Average Annual Precipitation on Catchment in M.S.L.

Elevation meters	Area Abada	Area Baba	Area Musa	Totals
Over 2000	-	1,010	-	1,010
2000 to 1000	11	403	16	430
Under 1000	15	10	13	38
<b>Totals:</b>	<b>26</b>	<b>1,423</b>	<b>29</b>	<b>1,478</b>

Of a total average annual precipitation of 1,478 million cubic meters about 270 M.C. reach the dam from the three dams, and of this about 50 actually enter underground beyond the reach of the plant roots, about half into the deeper confined waters and the other half into the shallow water, (5) p. 20. According to Table, (6) p. 20. The average annual flood to the dam is 210 M.C.

Darby, (12) p.2, assumed a similar figure for the mean annual recharge equal to the M.C.M. He mentioned that the extraction rate by the 4000 tubewells during 1956 to 1969 was about 10 M.C.M. per year without any appreciable draw-downs, and that the gross withdrawal during 1971 had increased to 13 M.C.M. This figure is more accurately 12.6 M.C.M. (13) p.1.

It is appreciated in India that the Ganga region of the Madh Bihar is one of the most important agricultural regions in the country and that it has appreciable potentialities yet to be developed. The United Nations Agencies were, therefore, approached for assistance to develop the area, and a project was identified in November 1969, and a request submitted in April 1970. Already this U.N.D.P. project is underway making studies and conducting pilot projects, and has also engaged consultants to do part of the work.

At the time of the submission of the request i.e. April 1970, in addition to the tubewells tubewells at the Ghazal Khan, Sir Sadiq and Sir / Sir, there were another 20 tubewells of an average depth of 70 m. and about 350 hand-dug wells in the area. The gross extraction from all these wells including tubewells wells was estimated to be 20 M.C.M. per annum, (13) p. 2. Since the two years that followed the request an additional 20 tubewells tubewells were drilled or planned, all to a depth of about 70 m. The gross water withdrawal from the area developments became fully

operational is estimated to be approximately 51 M.G.D. per annum, (12) p. 5. This total was estimated as follows:

Adm Water supply	13 M.G.D.
State Farms	13 M.G.D.
Private Mills	25 M.G.D.
<hr/>	
Total:	51 M.G.D.

However, since the above estimate was made, most of the private farms were taken over by the Government together with the private mills, and were converted into state farms. Thus it is not expected that more private mills will be drilled under the present circumstances, and therefore, the gross withdrawal may tend to be less than 51 M.G.D.

All the available data indicate that the ground water reservoir is enormous and assuming that the recharge is in fact approximately 100 M.G.D. per year, then it is probable that the management of the ground water reservoir on the annual cycle should not present difficulty, the annual extraction being about one third the recharge only. Figure 13 shows the U.S.P. Land and Water Utilization and Conservation Project and also the main low wells and wells in the State.

#### b. irrigation

Still complete analysis need to be carried out more or less annually in the U.S. for any years before independence.

However, since Indianapolis only makes tests for bacterial contamination of the water and measurements of residual chlorine are conducted daily in the U. S. C. Laboratory at the Indiana State. Also conductivity measurements are made, and such records are available for some years as far back as 1937. The results of field sample analysis conducted in October 1963 by the Southern Public Health Laboratories, U.S. for six samples, two from each treatment, are included as Tables 13 A, 13 B, & 13 C.

The following comparison was made in 1964 of the quality of the water from different sources within the Indianapolis region, (9), p. 4.

Table 13

Total Solids in Different Sources Within Indianapolis 1964

<u>Source</u>	<u>Total Solids, P.P.M.</u>
Average of English Branch Supply	1040
Average of Air Aired Supply	1400
Good Hole at Air Tower	1230
Surface well at Air Tower	1000
Surface well at Entry	1040

Table 134Chemical Analysis of Urinal Citrus, 1937, Oct. 1945

Results in Parts per Million (ppm)

	<u>Run 13.6</u>	<u>Run 13.11</u>
Appearance	Bright with a few particles	Bright with very slight yellow-brown deposit.
Microscopical Exam.	-	Few mineral particles
Turbidity	Less than 3	Less than 3.
Colour	Nil	All filtered
Odour	Rubber from bottle cap.	Rubber from bottle cap.
p.H.	7.6	7.5
Free carbon dioxide	0	?
Microlie conductivity	1000	1000
Dissolved solids dried at 100 C.	1510	3700
Chlorine as Chloride	600	1000
Alkalinity as Calcium carbonate	165	100
<u>Hardness</u> Total	530	2200
Carbonate	165	100
Non-carbonate	365	2100
Nitrate Nitrogen	16	15
Nitrite Nitrogen	Less than 0.01	Less than 0.01
Ammoniacal Nitrogen	0.00	0.05
Crystn observed	0.5	1.2
Ammoniacal Nitrogen	0.05	0.05
Residual chlorine	-	-
<u>Metals</u> : Iron	0.9	0.9
Zinc	0.5	0.6

## Table Contd.

Copper & Lead	-	-
Fluoride (F)	1.1	0.5
Sulfur	65	60
<u>Mineral Analyses:</u>		
<u>Sulfuric Acid</u>	70	305
Hg <sub>2</sub>	05	390
Zn	205	930
K <sub>2</sub>	6	7
<u>Ammonia Gas</u>	99	60
CO <sub>2</sub>	355	925
O <sub>2</sub>	600	1050
H <sub>2</sub> O	71	62
<u>Elemental combinations:</u>		
Calcium carbonate:	105	100
Calcium sulphate	15	755
Calcium chloride	-	125
Magnesium sulphate	621	-
Magnesium chloride	-	1, 369
Sodium chloride	653	1292
Sodium nitrate	97	65
Potassium chloride	7	19
Sulfur	65	60
Total	1802	3767

Table 13.11

Chemical Analysis of Air Data, Nov. 1965

Results in Parts per million (ppm)

	<u>Run No. 14</u>	<u>Run No. 21</u>
Appearance:	Bright with yellow-brown deposit	Bright with yellow-brown deposit
Microscopical Exam:	Mineral particles and traces of organic matter	Mineral particles
Turbidity	3	Less than 3
Colour	Nil filtered	Nil filtered
Odour:	Rubber from bottle cap	Rubber from bottle cap.
pH.	7.6	7.5
Free carbon dioxide	26	10
Electric conductivity	1900	1900
Dissolved solids dried at 100 C	1260	1950
Chlorides present as Chloride	230	320
Alkalinity as calcium carbonate	315	320
<u>Acidity Total</u>	325	385
Carbonate	315	320
Non-carbonate	10	15
Nitrate Nitrogen	9.8	7.5
Nitrate Nitrogen	Less than 0.01	Less than 0.01
Ammoniacal Nitrogen	0.15	0.11
Oxygen absorbed	1.0	0.80
Ammoniacal Nitrogen	0.10	0.08
Residual Chloride	-	-

## Gross.

<u>Antimony</u> Arsen	2.6	1.6
Bism	0.23	0.13
Copper & Zinc	-	-
Fluoride	0.6	1.1
Stront	55	70
<u>Mineral Analysis</u>		
Calcium Ca	67	58
Mg	37	50
Na	310	405
K	3	1
<u>Ammonia</u> NH <sub>3</sub>	103	153
CO <sub>2</sub>	353	405
Cl	250	320
H <sub>2</sub> O	63	33
<u>Emmetrical Combustion</u>		
Calcium carbonate	170	155
Magnesium carbonate	123	140
Magnesium sulphate	7	15
Sodium sulphate	447	502
Sodium chloride	375	225
Sodium nitrate	51	40
Potassium chloride	5	3
Water	55	70
Total	1280	1530

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Table 13 CChemical Analysis of Big Head Water, Cal., 1965

Results in Parts per Million (ppm)

	<u>Sample No. 1</u>	<u>Sample No. 2</u>
Appearance:	Colorless with very slight yellow-brown deposit	Colorless with very slight yellow brown deposit
Microscopical exam	Mineral particles	Mineral particles
Turbidity	Less than 3	Less than 3
Colour	Nil-filtered	Nil-filtered
Odour	Rubber from bottle cap.	Rubber from bottle cap.
pH.	7.6	7.5
Free carbon dioxide	0	9
Electric conductivity 1960		2000
Dissolved solids dried at 100 C.	1450	2000
Chlorine present as chloride	400	740
Alkalinity as calcium carbonate	170	170
<u>Hardness</u> Total	535	630
Carbonate	170	170
Non-carbonate	365 15	460 22
Ammonia Nitrogen	Less than 0.01	Less than 0.01
Nitrate Nitrogen	0.03	0.06
Ammoniacal Nitrogen (N <sub>2</sub> absorbed)	0.20	0
Abundant Nitrogen	0.02	0.03
Residual chlorine	-	-

## Contd. Table

<u>Nitrate</u> Iron	0.10	1.6
Zinc	0.1	0.1
Copper & Lead	-	-
Fluoride	1.0	0.9
Chloride	55	55
<u>Mineral Analysis:</u>		
Calcium Ca	03	102
Mg	23	129
Na	225	320
K	5	6
<u>Ammonia</u> NH <sub>3</sub>	102	102
CO <sub>2</sub>	340	410
Cl	600	740
SO <sub>3</sub>	00	97
<u>Practical combinations:</u>		
Calcium carbonate	170	170
Sodium sulphate	51	352
Magnesium sulphate	302	292
Magnesium chloride	5	171
Sodium chloride	047	073
Sodium nitrate	110	133
Potassium chloride	0	10
Silica	55	55
Total	1420	2101

American Standards set total dissolved solids at a permitted maximum of 500 ppm, but in the British Isles waters containing more than double this quantity have been used for many years without ill effect, (14) p. 165, Table, (15) p. 165, however, states that total solids permitted by American Standards are 1000 ppm. In all cases it is clear that the Eden Supply water is unacceptable by American Standards. Moreover, the dissolved solids are not readily removable. Sodium and Magnesium are the important cations, and sulphate and chloride are the anions. Calcium cations and bicarbonate anions are also found but in a lesser degree of importance. As can be seen from Tables 13A, 13B and 13C the general water in the Eden valley is slightly alkaline with a p.H. of about 7.5.

The following comparison was made in 1961 between the conductivities of the confined waters in the valley, (5), p. 19.

Table 15

Conductivity Variation with Area - Confined waters, 1961

<u>Location</u>	<u>ppm Conductivity</u>
Long	1200
Big Moor	1600-2000
Big Wood	2000-25000
South of Eden	1500-2000

The electrical conductivity varies directly with the total solids along the graph shown in Figure 16.

It is seen from Table 15 and from Figure 15 which shows the variation of the conductivity of the unconfined waters with the area, that the conductivity of the unconfined waters has a range between 1100 and 2000 and the conductivity contours are roughly parallel to the coast, with the conductivity increasing towards the coast, except for the contours of the deeper lens called waters of Al-ahd area. The difference in conductivities of confined and unconfined waters is little except in the Al-ahd area, and possibly in the Al-ahd area where the confined waters are of a better quality (5), p. 11.

The increase of salinity towards the coast is probably due to the absorption of mineral salts from the earth by the ground water as it flows towards the coast, and due to the sub-sea flow which remains within the capillary zone and hence is affected by the evaporation. The salinity increases further near the sea because of the added effects of intermingling with the sea water and of the sea-water intrusion. In the Al-ahd area these are the secondary effects of the salt pond in the Al-ahd area. These caused the water of a bore which was once drilled in their vicinity to be higher in salinity than sea-water itself, (9) p. 5. The salinity of the Al-ahd area increased considerably over the years because of over exploitation of the limited aquifer which is close to the sea and the salt pond. Moreover, the bores had been drilled too close to each other, resulting in sea-water intrusion or invasion of the deeper waters by the more saline shallower waters.

The salinity of the Sheikh Othman bores improved appreciably after the commissioning of the Bir Nasir Headworks which resulted in a decrease of pumping of Sheikh Othman Water.

It appears that the waters improve slightly with depth in the northern area of the Delta and also in the Sheikh Othman and Bir Ahmed areas, (5), p.13.

4.4.1. Temperature of the Water:

The temperatures of the well waters range from 29°C to 35°C for unconfined waters as can be seen from Figure 16, (5) Fig.6. The temperature of the confined waters range between 34°C and 41°C and the temperature was found to increase by 1°F for every 57 feet depth in a borehole in confined waters, (5), p.17.

These high temperatures cause some difficulties for the pumping, and for instance the P.W.D. Annual Report 1958-59, (16) p.17, mentions that some difficulties were experienced during that year with submersible borehole pumping units at Bir Nasir headworks and a full supply was not possible from those headworks and additional supplies had to be taken from Sheikh Othman to avoid restrictions of supply.

## CHAPTER V

### WATER SUPPLY SYSTEM (LAW)

#### 5.1. General

The supply of water to Addis is vested in the Addis Corporation for water. This self-financing body was formed in 1970 out of the water Authority which was a department within the Ministry of Works and Communications. It is responsible for the water supply systems in the country as a whole, the system serving Addis being by far the largest and most important one. The layout of the Addis water supply system is shown in Figure 17.

The Corporation is headed by an executive chairman who is simultaneously the chairman of the board of directors, which is appointed by the Minister of Works and Communications.

The Corporation employs more than 530 persons to deal with all technical and administrative work involved. The P.W.C. had sections for planning, design, pumping, distribution, mechanical and electrical workshops, building maintenance, stores, establishment, accounts etc.

#### 5.2. Factories

As can be gathered from the previous chapters, there are three factories all consisting of shallow boreholes, located at Sheikh Ahmed, Sir Hagg and Sir Ahmed.

##### 5.2.1. Sheikh Ahmed (the oldest)

These works are the oldest of the three the first

boreholes having been first put into operation in 1969. There are 17 boreholes altogether varying in depth between 200 and 300 feet. Only bores No. 3, 4, 6, 7, 8, 10, 11 and 13 are still in use. The layout of the South China boreholes is shown in Figure 10. 10- E.P. Cameron-type pumps are usually used in the boreholes. Previously Aden depended entirely on this source for its water, however, it is now of secondary importance and is used to supplement the air lift supply to Aden. South China water is mixed with the much less saline water from the Red Sea before supplying the towns of Sana'a, Ta'izz, Hadda, and Aden. South China itself with the towns of Mansura and Jibla are supplied with unmineralized Red Sea water. These boreholes were originally designed to supply up to 3 m.g.d. However, after the construction and expansion of the Red Sea boreholes, the supply from this source was decreased from about 1200 m.g. in 1969-70 to about 500 m.g. in 1970-71. It was 450.0 m.g. during 1970-71.

There is a one million gallon low level emergency reservoir, 11 feet high, in the South China National System in which several of the boreholes are located. The tank collects all the water raised in the boreholes through a common 18" pipe main. A booster station with four boosting units ranging in capacity between 7.5 and 10 H.P. are used to boost water from the South China to the main line, and the middle and high level supply stations. The level of South China is about 60 feet above sea-level, whereas the others is less than 10 feet above sea-level. There is

supplied to the machinery by the Public Corporation for  
 London power, and there is a 250 H.P. diesel generator  
 at the headworks as stand-by.

### 3.2.2. The Waste Treatment:

The first part of these works was commissioned  
 in 1955. This consisted of a line of shallow boreholes 16  
 in number 16" diameter each at 1000 foot intervals along the  
 main line. This line was approximately along an east-west  
 direction perpendicular to the line of flow of ground  
 water. 16 nos. 10 H.P. centrifugal pumps set at an average  
 depth of 120 feet below ground pump water through 4" dia.  
 rising main to two 12" dia. collecting mains which feed  
 water into the reservoirs.

The second part of the works was commissioned  
 in 1961. This consisted of 16 boreholes each having  
 an average depth of 250 feet and a diameter of 15" lined with  
 10" diameter cast slotted tubes. The boreholes are at  
 1000 foot intervals along a line parallel to the old line  
 and 2000 feet to the north of it. 10 H.P. centrifugal pumps  
 are used in these wells, the water being led into the  
 reservoirs via two 12" dia. collecting mains.

The first line was designed for a total extraction  
 of 2.5 m.g.d. while the second line was designed for about  
 5 m.g.d. All three wells have pumps which are to operated by  
 either remote control from the bank-end or from the  
 individual pump houses.



In 1966 two more wells of a slightly larger diameter were drilled in between the existing lines, the idea being that each of these wells was to replace two existing wells. This idea did not succeed and the handholes were fitted with 30 in. pumps. Two more wells were drilled in 1971 directly by the water authority but these have not been fitted with pumps or pump houses.

An elevated rectangular steel tank with a capacity of 100,000 gallons was constructed with the first line of handholes in 1956. The tank was a height of 17.5' above ground and 231.5' above sea level. The water was delivered by gravity to 40th via the main tank and the main ran through a 16" main constructed of spun iron pipe with bolted gland joints, but with the second line of wells a 24" asbestos cement main was laid between 40th and 20th via the main tank, to deliver to 20th by gravity.

Water is supplied by the water authority and the numerous pumps are fed through individual pipes to each handhole. A 30" diameter electrically operated gate valve is used to stop water flowing to the handholes.

### 3.2.3. 21st Street Handholes

Three handholes were installed in 1956 to supply 21st Street. These were drilled at the north end of the main boreholes situated on ground level running in an east-west direction. All wells are equipped with 10 in. diameter pumps which feed a 12" dia. cast iron main through 4" dia. rising mains. The water is collected into a 12"

level welded casing tank, from which it is connected to the discharge main which is pumped through a 12" dia. steel pipeline. There are four 5-stage centrifugal boosting pumps each rated at 350 g.p.m. at 500' head, driven by 100 H.P. motors which pump to the main line. There are also six 4-stage pumps in the same main line contributing to the village of 127' and a short distance away. The town of Lakshmi Achal is also supplied from the main.

The centrifugal pumps can be controlled from the bore-head or from the field control pump house. Electricity is supplied by the E.S.S. and the centrifugal pumps are fed through 11 KV/400 roof transformers. There is also a 25-kw. stand-by generator at the bore-head.

### 5.3. The Remaining Stations

As already described the main lines pass at both the Lakshmi Achal and the Lakshmi Achal. The other main pumping stations are at Lakshmi Achal and Lakshmi Achal. There are other very small stations also, one in the city and one near the station which supply the Lakshmi Achal village.

#### 5.3.1. Lakshmi Achal

There is the largest storage station within the system. It supplies the Lakshmi Achal, Lakshmi Achal and Lakshmi Achal areas. Water from Lakshmi Achal is stored in the low-level 32 foot high welded casing tanks, each having a capacity of 5 million gallons. The water is pumped

after chlorination to the main line high-level reservoir and to the two lateral main high-level equal-section tanks.

The pump house incorporates 9 centrifugal booster pumps operated by electric motors of different capacities varying between 100 and 270 H.P. There is also a 500 H.P. stand-by diesel generator for use during failures of power which is supplied by the S.E.C. at 11 kV and transformed down by the station main transformer.

As these works are situated in main streets, the mechanical and electrical installations, the water supply lines, the laboratory which carries out daily routine tests, and other facilities. The valuable water main section is located at the South China Institute. Chlorination is effected through 1-inch chlorinators, the chlorine being obtained from overhead in cylindrical water pressure.

### 3.3.2. Water Distribution System

These works are located at the end of the straight street line along the road leading to the main water supply. Water is pumped from the main through a 10" pipeline into two low-level vertical circular tanks (total of one million gallons capacity in three units). There is also a tank in which there are three 100 H.P. centrifugal pumps that pump the water to 100 feet above the ground level.

There are also storage tanks in a street line owned by the S.E.C. which are used for storage of water as distinct from the treatment. There is about 100,000 gallons, the water

primary village at the west-end end of the 240000 Area  
pondworks, through a 6" dia. steel pipeline.

Water is supplied to the village house works through  
two routes and there is no standby generator.

5.2. Infrastructure

5.2.1. The water system

Water flows by gravity from the intake to the house  
via the 240000 intake through a 16" open area pipe and a 16" asbestos  
cement pipe. The ground level at the intake is 170 feet above  
sea level. At the house there are two reservoirs, a low level  
welded circular reservoir of 1 million gallons capacity and a  
high level cast-iron steel reservoir of 0.1 million gallons  
capacity with a h.t.l. of 97.5 feet above ground. The  
intake is at 60 feet A.S.L. It has an old masonry tank 22  
feet high with a capacity of 1 million gallons. The intake  
water is pumped off to the house. In addition to the 16" A.C.  
and 16" open area pipes running between the intake and the house  
via the 240000 intake there is also a 16" A.C. pipe running between  
the 240000 intake and the house.

A 9" A.C. pipe runs from the 240000 intake to the  
the village works at 240000. A low-level welded  
circular tank was installed at 240000 in 1967 but has  
never been commissioned. As to the water supply in the  
number of days calling at the intake since the closure of the  
the 240000 intake the use of the pipeline has not been  
not been possible.

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Returns to about 10 feet above sea-level. From the  
 two low-level welded cylindrical tanks, each 32 feet high and  
 with a capacity of 5 million gallons, water is pumped to the  
 high level tanks constructed on the ridges of the mountains  
 over-looking Guter, Shalla and Aburakur. The military  
 tank at the high level was partly constructed on the old  
 Turkish fortification walls which run along some of the  
 ridges surrounding the town of Guter. The reservoir has  
 a S.W.L. of 320 feet. It has two compartments, one about  
 1.2 m.g. in capacity for supplying Shalla and Simat, and  
 one about 1.2 m.g. in capacity for Guter. This  
 tank was commissioned in 1950. It is supplied from Returns  
 via the rising main one 24" in dia. and the other 18" in  
 dia. Water is also pumped from Returns via a 16"/18" dia  
 rising main to the two steel lined tanks constructed with  
 occasional steel tank plates at the top of Sobal Fadaa  
 Mountain. Each of these two tanks has a capacity of 0.5 m.g.  
 a height of 12 feet, and a S.W.L. of 270 feet above sea  
 level.

From the high level and Sobal Fadaa tanks water is  
 supplied to Guter, Shalla, and Simat via 18" pipelines  
 in duplicate and to Aburakur via a 18" dia. water.

There are several small tanks particularly  
 in the military area in result of the mountain. A 1 m.g.  
 welded cylindrical military tank was constructed on a ridge  
 in the Guter area of Shalla in 1957 but has not been  
 commissioned since then.

The Agra water supply system has more than 200 miles of pipes varying in diameter between 3 and 36 inches, and has several hundreds of miles of (obscured) iron service pipes connecting supply to houses. These connections are individually noted, whilst bulk meters are used for the major consumers such as military camps, public corporations, factories, etc. The total number of meters in the Agra system (as distinct from the Middle Agra system) is about 20000 meters.

#### 5.5.2. The Middle Agra System

Water is pumped from the 100' lined reservoir via a 12" steel pipeline to the two elevated tanks of the Agra House, each having a capacity of 1 m.g. From which it is pumped to the service reservoir for the Middle Agra township and also to the two 1 m.g. capacity, vertical circular tanks at Chhatrapati well which service the A. S. C. B. Military. The distance from the Agra House to the service reservoir is about 2000 feet and the distance from the Agra House to the Chhatrapati tanks is about 3000 feet. The distribution is carried out both as 12" lined main lines and 6" mains.

The Middle Agra water is supplied to a few places and a few villages in the valley, viz. at Agra and Alwar. The main supply is to the villages of 12' high and also to the area supplied by the 12' high lines.

### 5.3. Cathodic Protection

Ingalium Anodes are employed in the water in the steel reservoirs at Istina, Sir Sir, Sir Ahmed, Slaughter House and Middle Area to protect the inside of the tanks against corrosion. Also, Galvanic Zinc Anodes are used with impressed current for the protection of the outside of some of these tanks such as the case of Istina and Sir Ahmed. Sir Sir tanks are not given external cathodic protection because the soil at Sir Sir has a high resistivity.

### 5.4. Communications

There are asphalt roads to practically all parts of the network. Telephones and radio communications connect all the main parts of the system with the head-quarters at Giza and with each other.

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### ANNEX TO THE REPORT OF THE COMMITTEE

#### 6.1. Introduction:

With an organization like the ILO responsible for work of such a wide range of problems and characteristics could be encountered and as several countries have engaged a large the setting up of this org. Indian Govt as a small water supply department, and would like to see that it should not be too difficult to find such a structure these problems and study ways and means of combating them. However, this assumes that records and statistical data have been kept and that there is a the necessary qualified personnel to analyse them.

Also any studies organization in a developing country these could be problems of a first class nature, shortage of skilled manpower, and a multitude of other problems of various degrees of importance. The ILO should concentrate about the ... and any such other organizations as well as that the difficulties could be in increasing day after day. This is to create a picture of the economic and other circumstances of the country as a whole.

#### 6.2. General outline:

These ... should be ... responsibility was ... only for the ... and after ... the ... case ... resources and will ...



the sudden departure of the British administrative personnel staff with the rest of the British civil servants) was made responsible for water supply systems throughout the Republic. However, many of the country towns and villages which had so far no water supply systems installed such systems immediately after independence, and started to bring considerable pressure upon the central government to give aid to these districts. However, the slow financial stabilization of the country did not permit such expenditure. To give effect, the Government dispatched the already depleted staff of the Aden Water Authority to these various points to make technical reports.

The Aden Water Supply System has been usually a money making concern. A U.K.C. consultant wrote in 1969 about the economic aspects of the Aden Water Supply. There are about 30,000 connections and the charge is £ 0.250 per 1,000 gallons. Revenue in 1966-1967 was £ 777,000 and expenditure £ 661,000 involving a profit of £ 117,000 representing approximately a payment on various loans totalling approximately £ 2,000,000 (17) P.S. However, there were three years when the Aden Water Authority was at a loss as can be seen in one report. A report on overall reduction in water costs of £ 2,000,000 from £ 2.3.00 to £ 2.5.00 per 1000 gallons raised, the water authority estimated to run at a loss, the deficit for the year ending 1967, £ 127,000. The basic reason for this estimated deficit was the fact that due to the large capital expenditure incurred in recent years, interest and redemption of loans charges have appeared in the balance sheet for the first time in the history

of the undertaking" (16) p.15. Two of the most important developments concerning the steel wire supply were the transformation of the organization into a self-financing corporation in 1970, and the introduction of a 3-year National Development Plan. To begin with, the plan obliged holders the U.S. of the pressure of the allocations from the various by-country plans, as development funds then onwards had to follow the plan. Unfortunately, the plan was far too ambitious as can easily be seen in retrospect. Most of the projects to be created by the U.S. have already been cancelled or postponed to be included in a future plan. This may tend to encourage by-country allocations to remain putting pressure on the U.S. The 3-year plan states that over 9.0 million was nearly 10% of the total allocations for the plan - are to be allocated from the reserves of the Public Corporation, of which probably only the first third, the Public Corporation for Foreign Loans, and the U.S. have come, and the sum of 2.9 million represents most of these reserves.

As the U.S. is suffering from the accumulation of large and debts because some organizations and y funds are not paying their debts and the U.S. is not able to discharge their equally because of political and other reasons, the loss of the reserves should be even more and only that, and should have given enough into the U.S. if they had to proceed to increasing the U.S. as a of U.S. funds have already decreased considerably as unemployment is increasing.

### 6.3. Organization and Government Involvement

The transformation of the steel wire supply organization

from a department within the same Ministry of Public Works  
 and Communications into a self-financing Public Corporation  
 in 1970 was no doubt a bold step in the right direction,  
 and was preceded by a 5% devaluation. This idea was  
 toyed with before independence but the vote was not  
 reluctant to release 100 million over a money raising concern,  
 for though the revenue went directly into the central  
 treasury, yet the P.W.C. probably felt that it could get a  
 larger annual budget than one of 100 million was making  
 money. The Corporation was setup to deal with water supply  
 through out the country and not merely in AGON. An  
 Executive Chairman was appointed to run the organisation  
 with the help of a Board of five part-time members. Unfor-  
 tunately, this Board has been suspended for over a year now  
 and the Chairman has to refer to the Minister of Works and  
 Communications in all important matters, and this situation  
 should not be allowed to persist for the sake of efficiency.  
 Moreover, the Chairman has already been changed three times  
 during a short period of about two years. In addition, the  
 present appointment has been mostly on political grounds since  
 the present Chairman has no technical or administrative  
 back ground to help him in discharging his duties. It is,  
 therefore, not surprising that most of the senior technical  
 and administrative staff of the P.W.C. have been sacked in  
 August, 1972 leaving the organisation in a critical position.

The question of merging the P.W.C. with the well  
 established P.S.D.C. is currently under study by the Government.  
 A report is being prepared by the Executive Chairman of the

E.I. ... on this subject. In the article preceding Independence, the Labor Authority was negotiating with Atom Electricity Corporation (which was transferred after Independence to the E.I. A.) the question of having a combined monthly bill to the consumer. After Independence the Labor Authority changed from monthly to bi-monthly billing to reduce overheads a little. Even if it is decided not to merge the two corporations they should consider common billing and common meter reading, etc.

As an independent corporation the E.I. A. has now the advantage of being able to order any necessary spare parts and equipments without having to go through the very slow government financial procedures which often take between one and two years, due to the economic difficulties of the country.

6.4. Manpower Problems

The Corporation employs about 500 persons. There is no difficulty in obtaining unskilled labor, but there is an acute shortage of qualified personnel, particularly engineers, accountants and technicians. This shortage has been existing even in the pre-independence days but of course in a much milder degree. It would be desirable to in the introduction to this report mention the case of the urgent need for additional staff. I must emphasize, therefore, it is my conclusion to refer to the matter again, but I consider it one of the most serious problems facing the Labor Authority at the present time. The fact that

Area's water supply can continue to function satisfactorily  
 irrespective of the number of control staff may have had its  
 origins in the lack of installations during the past 5 years,  
 but such ideas are false. The water supply has only contin-  
 ued to be satisfactory because a number of conscientious  
 officers were prepared to work long hours to avoid  
 failures. With the increase over the past 5 years of  
 installed mechanical and electrical plant, plus that proposed  
 in this report, the ability of staff to prevent interrup-  
 tions to the supply depends increasingly on adequate  
 maintenance of installations. This can only be achieved  
 with sufficient supervisory staff and hence until such  
 additional staff are made available the risk of failure  
 will continue to increase. (10) p.14.

After the departure of the British supervisory staff  
 the water works was run by their local assistants some  
 of whom were of not adequate experience. A UK consultant  
 reported in 1969 that there was only one qualified Engineer  
 directing a staff of 536 including 100 non-readers, clerical  
 and construction staff, and that operation, maintenance  
 and construction was carried out under 300 inspectors of  
 works. It also stated: "The organization does not include  
 a senior administrative officer or an accountant and the  
 inter-divisional relations are generally poor. It also  
 lacks a substantial electrical control staff" (17) p.5. The  
 staff situation has considerably worsened since that

Report was written due to the expansion of the duties of the organization to cover the whole country, and due to the termination of the services of a number of the senior employees. The effects are a lower efficiency of the system and poor maintenance of assets.

The Corporation has to recruit local and foreign senior staff to improve its situation. It has also to give considerable attention to the question of training. Local and overseas training programmes should be evolved to produce senior and supporting staff of a satisfactory caliber. Such programmes need to be followed before independence.

#### 6.5. Planning Problems

The Aden water supply system was started about forty years ago and was gradually built up into a fairly sophisticated system. Before that, several preliminary reports had been prepared on suggested schemes such as the report of Messrs Hobb and Day dated 26th April, 1912, and the report by Major Robertson and Captain Taylor dated 14th April, 1915, and the notes by Percy Johnson in 1917; however, usually no action was taken upon the recommendations of these reports. Development of the system continued to lag behind the increasing demand as can be seen in the report by Johnson in 1934: "It seems that, with the exception of the report of Messrs. Hobb and Day which was not acted upon, no real attempt was previously made to provide more than palliative measures, usually very such in character

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of requirements", (9) p.1. The fact that planning tended to be short term in nature until at least 1960 is inferred from Jennings statement in 1960: "Unfortunately, the present policy of short term planning combined with the fact that service staff strength is always lagging well behind minimum requirements, makes it impossible to achieve the proper standard of planning and design", (10) p.1.

Jackson and Jennings themselves managed to achieve some degree of service planning and perhaps the fact that water rationing and restrictions of supply were not resorted to after the water rationing during 1952/53, except for occasional short periods particularly during the last phase of the Independence struggle, supports this statement.

Due to the withdrawal of the British forces, the water supply was left with spare capacity. This spare capacity, however, has now been used up due to the increase in population as a result of the movement of up-country population into Addis, and the Government has to plan for development. Planning for development should be rather difficult because of the difficulty in estimating future demand.

The present population is not likely to any degree of accuracy and this difficulty will be solved when the projected country data comes to completion. Future increase is difficult to estimate as one will be justified to assume that increase would be following previous trends due to the major political and economic changes in the structure of

the country. Furthermore, the 3-year plan envisages the establishment of several industrial projects such as factories for spinning and weaving, silk rearing, cement production, soap, bicycles, metals, animal feed, etc. and mills for flour and cotton seed oil. However, up till now, the capacities of several of these projects have not been finally decided upon, although more than half the duration of the plan is already over. However, several of the projects have been cancelled or postponed.

Another problem facing planning is that the P.S.U. does not know clearly the state of its assets and does not have a schedule for replacement, although many of these assets are approaching the end of their useful life. The concentration on up-country development distracted attention from the need to maintain the Alan system properly. The P.S.U. has to draw up preventive maintenance schedules and to conduct a major survey of the system.

A U.K. expert estimated that the needs of the industrial plan would require an additional 1 million gallons/day, and that the present capacity should suffice up till about 1982, (18). On the other hand the water works engineer estimated that the requirement by 1970 should be 13.5 million per annum i.e. almost twice the present consumption, (19) p.1. As both these estimates were made around the middle of 1972 this very wide difference in opinions goes to show the difficulty in planning in the absence of adequate data.



A small statistics section exists in the P.W.D. but it is at present manned by only one junior clerk, whereas a considerable amount of statistical data and useful information concerning the quantity and quality of the water, the works and assets, the surveys, etc. can be collected and classified if the necessary qualified staff were found to run the section. A planning section has recently been set up to be run together with the design section and the drawing office by the most experienced engineer in the P.W.D. This is no doubt a good step, and it is suggested here that the statistics section be also placed under that section. It may also be suggested here that the P.W.D. should develop a reasonable library which should include copies of all reports on the water supply systems in the country and some useful reference books and catalogues connected with water supply, and the library be placed under the planning section.

As much work is entailed in this planning section such as :-

- (a) Following up the progress of the J-7-37 plan and its modifications,
- (b) Developing the statistics section so that various useful data is recorded, stored and classified,
- (c) Bringing up to date the drawings of the water supply systems of the country,
- (d) Developing the assets of the P.W.D. and in the light of the drawings carrying up approximate maintenance and replacement schedules.

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- (o) Drafting out local and overseas training programmes,
  - (P) evolving standards to be followed particularly in the purchase of equipments, as after independence equipments from various sources did not always conforming to the British standards previously adhered to have been obtained,
  - (G) development of a library for the Corporation,
  - (h) development of an embryo research section to investigate matters such as the effects of local climatic condition on the different types of pipelines, cost comparisons of different types of pipelines taking into account initial and running costs, the possibility of local manufacture of well slotted casing, etc.
  - (A) introduction of more up to date technology.

It is only with forward long term planning that expensive mistakes might be avoided. Thus, for instance, only by keeping in close contact with the irrigation projects in the field such that the effects of these projects on the water levels in the area under supply wells can be followed and a dangerous crop which can render these wells useless may be avoided. The selection of locations for future reservoirs is another example of a project that calls for long term planning. Further suggested that the new water supply wells should be sited in the sand dune areas in order to preserve the areas of good soil for ground water irrigation development (12) p.3.

## 6.6. Technical Problems:

The P.S.C. has several technical problems which require consideration and which can be solved provided there are the necessary funds and the necessary qualified people. The C.M. has made it easier to look back and examine any technical weaknesses of the system. Two technical points which seem to deserve consideration by the P.S.C. are discussed below.

### 6.6.1. Design of Wells:

Wester states, " It is considered surprising that a well depth of 70 metres has become so generally accepted in the delta. It appears to have been established in the Bir Basser area where there is a fairly thick silty clay at about this depth. This clay may be expected to be of limited thickness and a return to a lighter material may be forecast at greater depth." (12) p.6. He goes on to say, " It should be noted that the Government wells in the future should penetrate to about 150 metres, thus providing hydrological data on the deeper part of the system. The Bir Basser well exploration well may provide some data on the complete thickness of the alluvial sequence in the lower delta. Several wells in the upper delta already penetrate to the crystalline basement and these (13) may maintain some deep exploratory drilling", (14) p.7. Wester also states, " The Bir Basser well since the sea water supply represents the largest single source of water in jeopardy. However, these wells must be approaching the end of their useful life and will in any event require replacement

During the next few years. They can therefore be gradually phased out and replaced by wells to a better design, distributed on a more effectively spaced well field (12) p.6 It should be remarked here that only the first line of wells, 14 in number, goes back to 1956, while the second line, also 14 in number, was constructed in 1961, the other remaining wells being only a few years old. As to spacing of the wells, the author, p.6 on p.2, states "within each line, the wells are spaced at 250 metre intervals. Despite this close spacing, excessive water interference and drawdowns were not experienced" (Incidentally, the spacing is 165 feet and not 250 metres). Jackson wrote about the spacing at South Othman, "It is, therefore, most unfortunate that such a concentration of wells has been sunk at South Othman. At this bore the water flowing through an unobstructed aquifer of a limited capacity, it is not wise to sink wells too close together. In fact all authorities are agreed that a spacing of 1600 feet is the least that is desirable. This has in fact been proved in tests carried out in connection with the Little Eden supply" (9) p.5. The spacing of the wells at South Othman can be seen in Figure 13.

Barber states, "According to Mr. Agnew, hydro-geologist, water levels in the Othman about 9 inches in the 1950s. Such was over the past years. The reasons for this fall in water level, was not fully understood. Increased usage of water, storage and discharge had

undoubtedly contributed. It is also possible that these years were of below average rocking. (12) p.3. As the water table continues ground-water is developed for irrigation, the levels are expected to fall further. The yield of the wells, particularly the older ones, has decreased due to corrosion, and it is not possible to increase yield by using more powerful pumps because the sand would shorten the life of the pumps considerably.

Darbar suggests that the most suitable design of new wells appears to be wells 150 metres long consisting of 12" casing and screen to about 100 metres with a 6" liner of casing and screen from 100 to 150 metres, without the necessity for screens in the uppermost part of the casing as these will be non-effective due to corrosion in levels. This is suggested as a temporary/interim well design. This well design has been determined, (13) pp. 3 & 6. However the U.S. should investigate the possibility of deepening some of the wells particularly the older ones, possibly by the use of a rotary drill as this should be easier to use in such a case.

Darbar suggested that if a third line of wells to be drilled at the same time should be sited at least 500 feet north of the existing second line of wells to avoid interference, (14) p.13.

#### 6.6.2. Recommendations

The generalization of the quality of the water

are practically not known. Downing in 1968 stated clearly that the permeability of the beds was unknown, (10), p.13. The USA project being conducted in liaison with the Department of Irrigation for the development of the Indian Ganga for agriculture is making a study of permeability in this area. It appears that the values for transmissibility and permeability for the Ganga well field has been found to be of the order of  $T = 1000 \text{ m}^2/\text{day}$ , and  $K = 20 \text{ m/day}$  (11) p.5. The permeability for the Ganga well field should be determined on this value is needed for determining the pump & design and spacing of future wells in the area.

### 6.6.3. Ground water contamination

The water supply system of the well field has both corrosion and encrustation. The quality and quantity of the atmosphere enhance corrosion considerably, whilst the hardness of the water is responsible for the high rate of encrustation which affects the water conveyance system quite adversely. Detentive-current pipelines suffer considerably less than the metallic pipelines of the system from either of these effects, and this explains the wide use of detentive-current pipelines in the Ganga system.

Encrustation causes appreciable increase in the internal friction of the pipeline, and in some cases warrants regular cleaning of the pipeline. Encrustation of the air lines carrying water is also reduced by the use of air necessary to clean the main canals, to maintain conditions, (12) p.3. Encrustation also affects the water meters

availability and shortage of water requirements so very high. During 1958-59 for instance, 7% of all the meters in the system were overhauled and repaired. Shortly before independence, the interested agencies are considering seriously the introduction of the use of regulating valves in place of water meters in case of the districts, at an experimental level.

The dependence less and less on British Ocean water and more and more on the local water will produce the combined good consequences of this construction in the conveyance system, and water of a better quality to the consumer.

The replacement of cathodic protection anodes and the repairing of the various nozzles regularly are particularly essential in the local water supply system because of the climatic conditions which tend to promote the rate of corrosion.

#### 6.6.4. Water Quality Statistics

Annual analysis of water in place from the three locations used to be carried out by the D.S. through the, means. Regular sample analysis should be required, so that deterioration and variation in the quality of a source can be noticed in good time the appropriate steps to be taken. The suitable public drinking tests can be determined by the statistics section of the D.S. by examining the trend in all areas.

### 6.6.5. Chlorination

Chlorination of the water is carried out with chlorine gas, but some of the chlorinators are no longer functioning properly, and it is most essential that they should be repaired or replaced without delay. Chlorine and other water borne diseases are not unknown in this part of the world and every precaution should be taken to ensure the safety of the water for drinking.

### 6.6.6. High Pressures

The mountainous topography of the area presented is responsible for the high pressures of water particularly in the distribution system of the city of Dar-es-Salaam. The reservoir which receives water from the high level high-level reservoir which has a h. l. of about 300 feet. These high pressures necessitate the use of the most up-to-date valves in most part of the system, and cause the usual problem

that go with high pressures such as higher leakage and more difficult valve operation. Where the topography permits, care for storage reservoirs and balancing and service tanks should be selected so that both too high and too low pressures are avoided.

### 6.6.7. Water Treatment

Water in the system is treated at over 100% and is possibly closer to 20%. During the period that preceded independence water was not treated at all. It is also possible that distribution, and certain distribution within the



country including that of the maintenance staff, even though they belong to the essential services.

The water authority used in the past waste detection meters advantageously and it is suggested here that waste detection should be carried out in the various districts, beginning with those where higher waste is suspected.

#### 6.4.8. The 16" Pipeline from the Works

The technical problem seems to exist in the 16" pipe from gravity main between the main and station. This pipeline was laid to ground levels as opposed to a continuous gradient and consequently it was found necessary to install automatic air valves at the high spots to improve flow characteristics, (19) p.12. and although the flow improved considerably upon the installation of these valves, yet there still seems to be difficulties with this pipeline. Considerable vibrations are felt in the pipeline. This problem deserves investigation and to determine these vibrations can be eliminated or reduced if the people working from it are taken. It is likely that the air valves installed are not able to get rid of all the entrapped air.

#### 6.6.9. Maintenance

It is worth while to mention again the importance of maintenance which, if properly and rigorously carried out, can reduce many of the existing problems, and can stop many other more serious from occurring. Negligence of maintenance might not show its consequences immediately or in a very obvious manner but they are there all the time

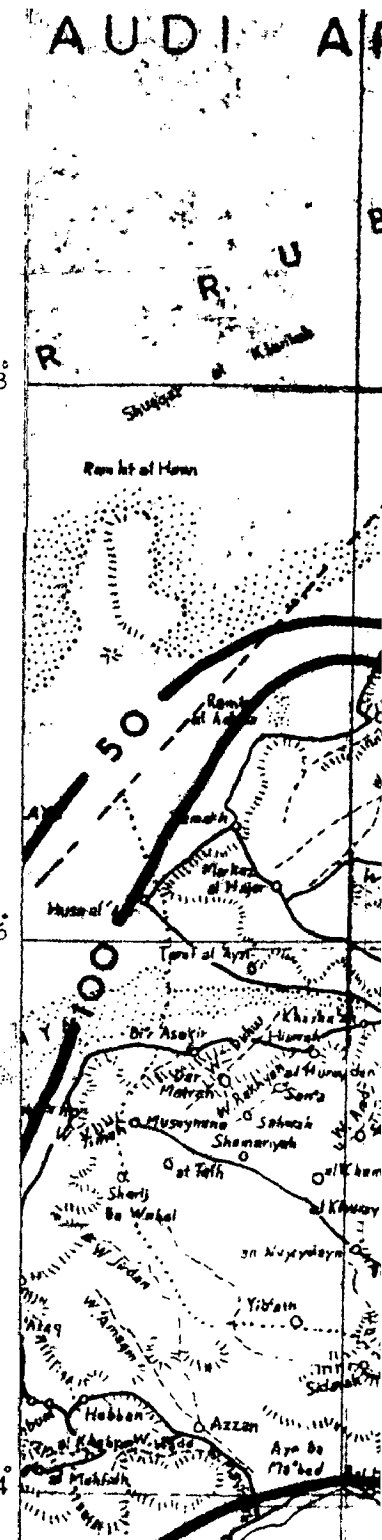
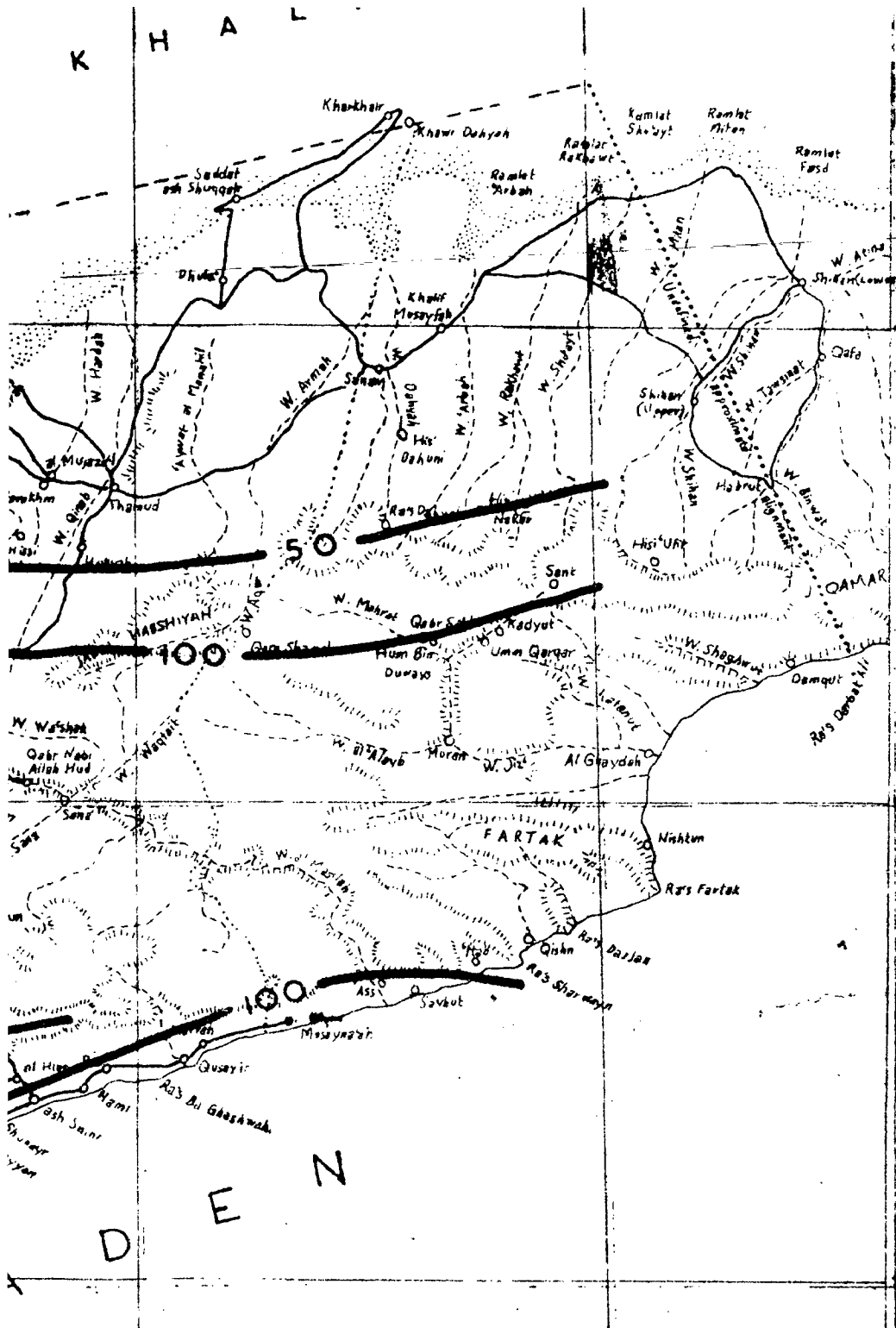
in the case of rapidly changing situations, overtime work, etc.

Maintenance schedules should be drawn out and followed strictly. Corrosion should be fought by the regular painting of assets and the regular replacement of cathodic protection anodes, etc. The stores of the I. V. should be correctly stocked with spare parts, spare tanks (such as spare submersible pumps) and materials of all types that may be needed ranging from steel plates to chlorine gas cylinders.

#### 6.7. Anticipate the requirements.

The problems discussed above are merely indicative and do not form a comprehensive list of problems facing the P.S.C. The 1-million gallon reservoirs which were completed in 1967 have still not been commissioned five years later. This gives an indication of the problems facing the P.S.C. In short, these problems can be tackled if the necessary funds and qualified staff were available. However, manageable, with a proved organization, the P.S.C. can make the most optimum use of the available resources to greatly reduce the difficulties.

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DEPARTMENT OF AGRICULTURE  
&  
IRRIGATION

FIGURE 1

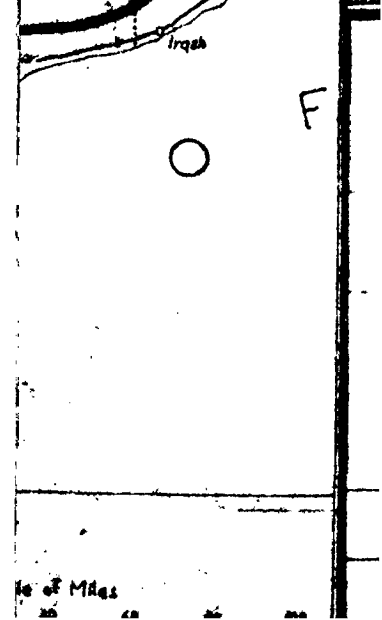
GENERAL

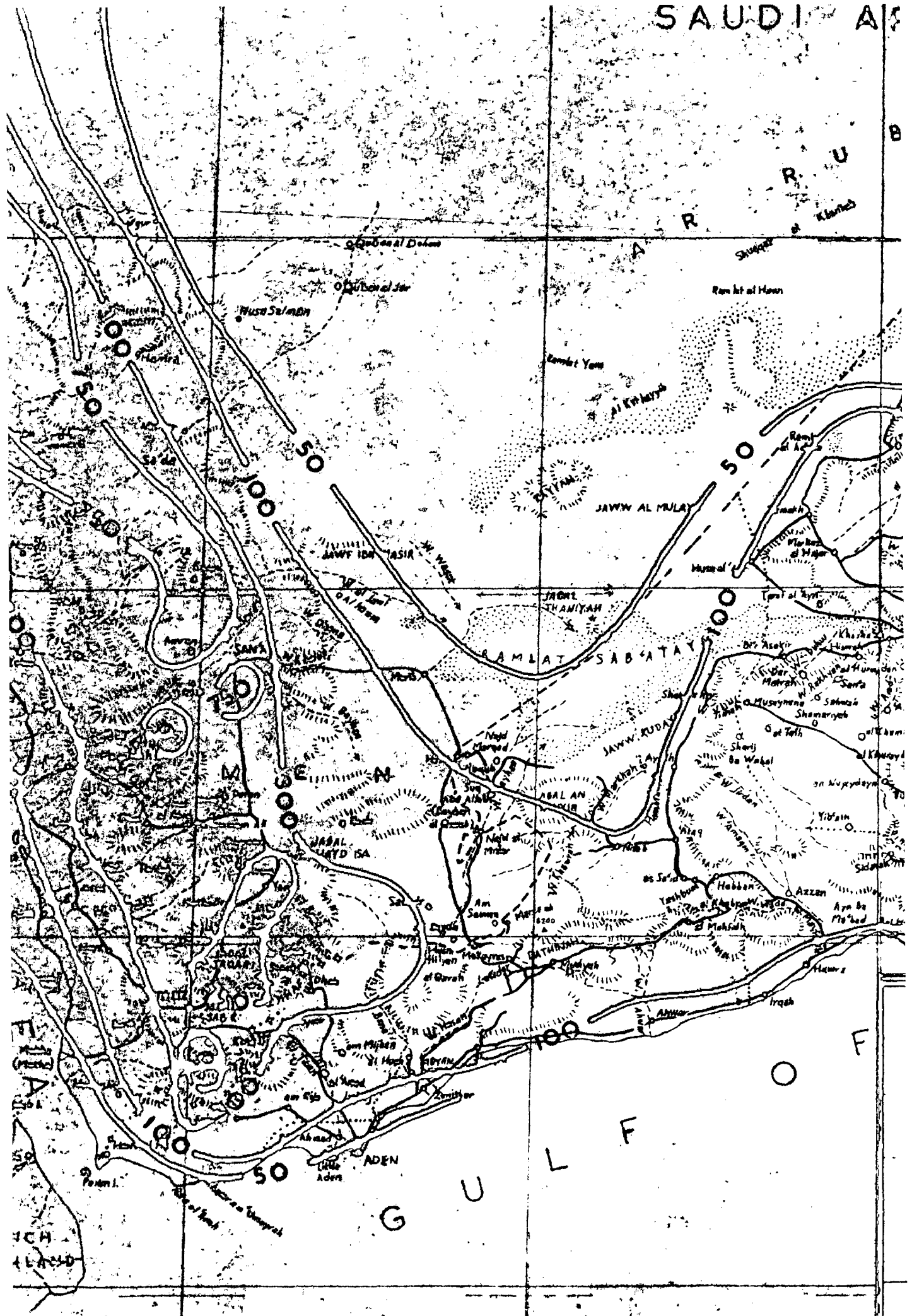
REPUBLIC OF SOUTHERN YEMEN MAP

ANNUAL RAINFALL OF P.R.S.Y

DATE  
OCTOBER 1970  
SCALE

DRG. NO.  
10.000 / 34



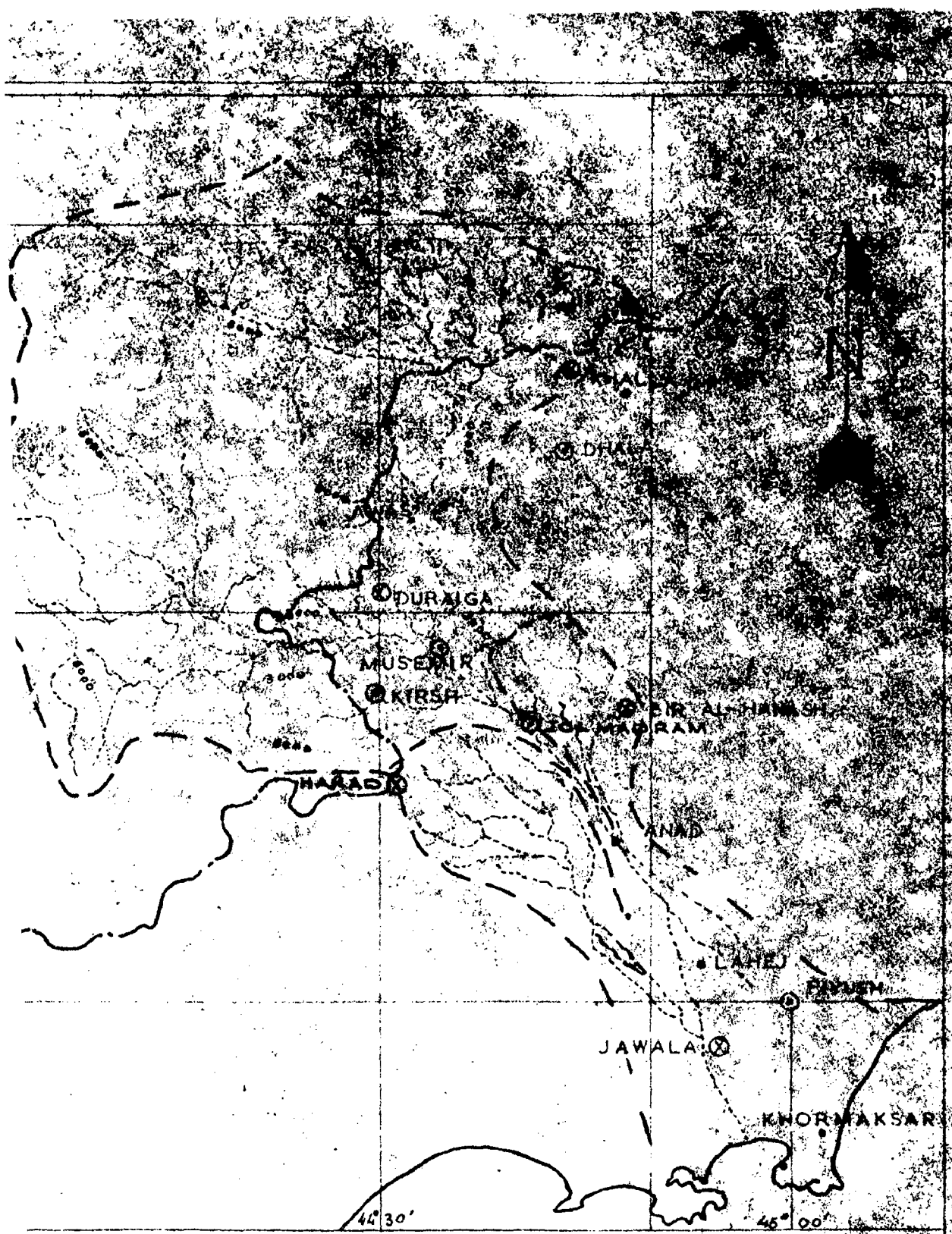


Scale of Miles



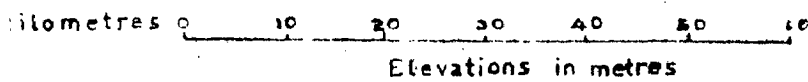


QUATERNARY		Qe	Eolian sand
		Qb	Alluvium on related surficial deposits (May include rocks of Pliocene age)
		Qa	Aden Volcanic Series (May include rocks of Pliocene age)
TERTIARY	Oligocene and Miocene	Ts	Shihr Group and equivalents
	Eocene	Tha	Habshiya Formation
		Thj	Habshiya and Jeza Formation
	Paleocene	Yo	Umm er Radhuma Formation
		TKt	Trap Series
	CRETACEOUS		Tt
		Kt	Mahra Group
JURASSIC			Jj
	PRECAMBRIAN TO PALEOZOIC	Pp	Precambrian (?) Synkinematic and Paleozoic postkinematic granitic rocks



THE WADI TUBAN CATCHMENT AREA.

MAP SHOWING METEOROLOGICAL STATION



INTERNATIONAL BOUNDARY  
 DASHED BOUNDARY  
 MET STATION  
 GAUGE STATION

SCALE 1 : 820,000 (APPROX)

FIGURE 3

TRACED BY: A.H. BASHIEB.



Cretaceous lavas



Pre-Cambrian gneisses and schists



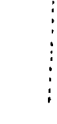
Undifferentiated granite



Faults



Wadis



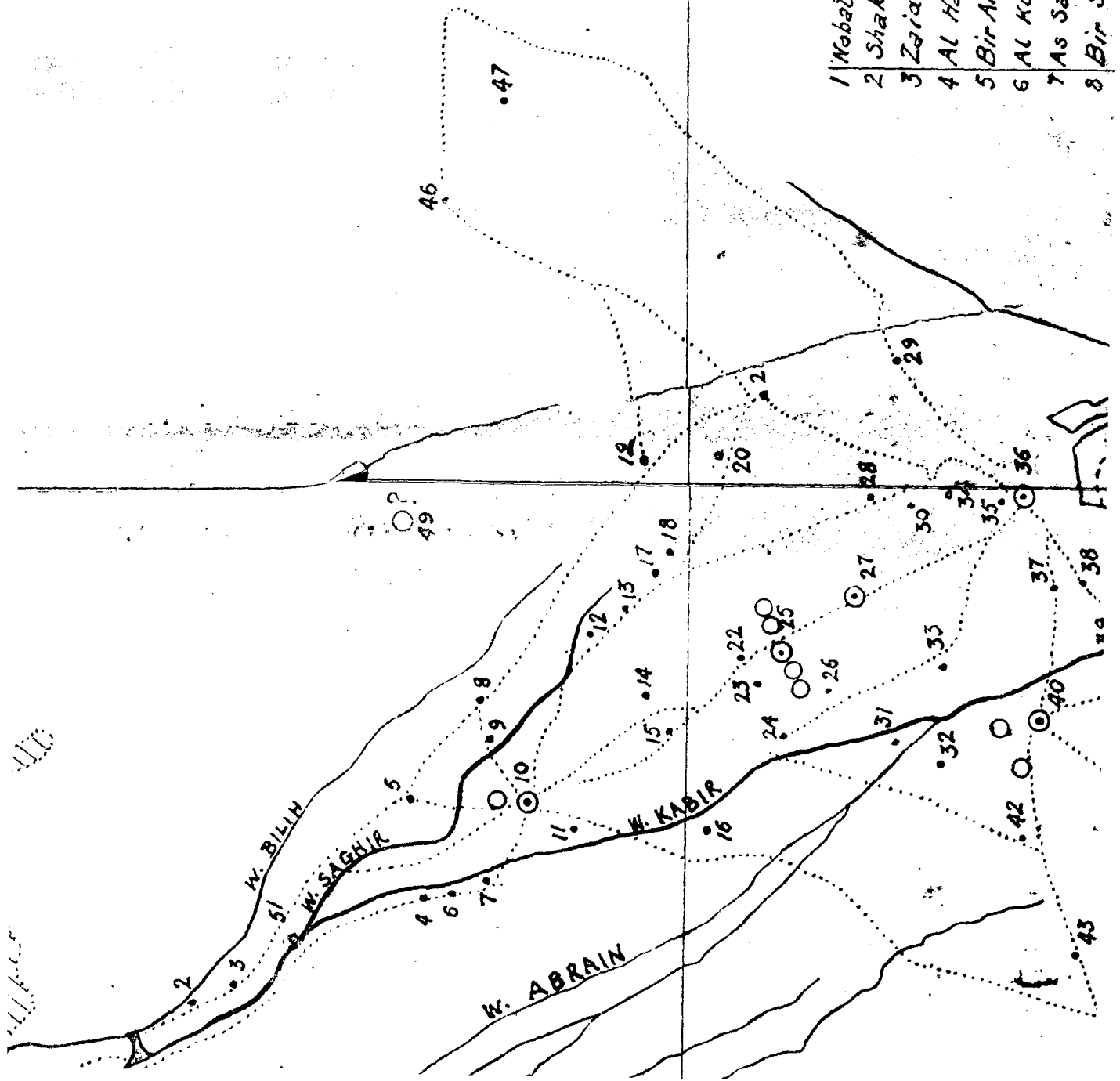
Tracks, roads



Well locations



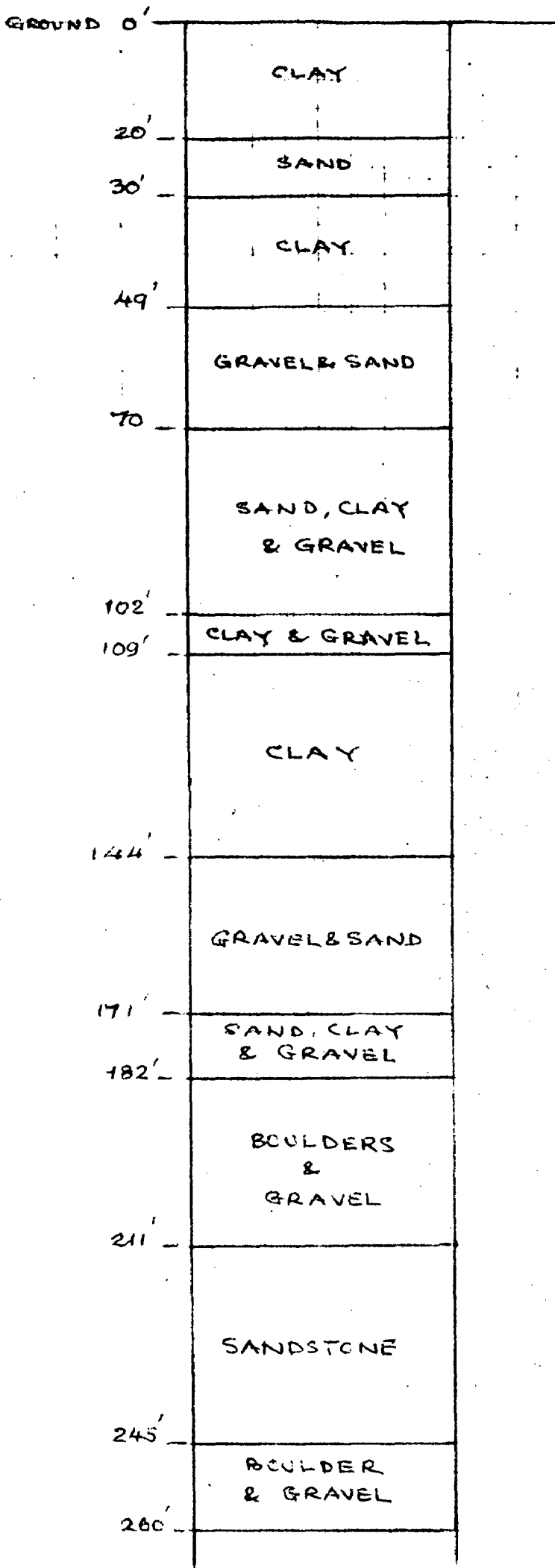
Borehole locations



LOCATION NAMES:

1	Noba' Dakim	18	Bir Hubil	35	Dar el Ai
2	Shaka	19	Bir Amr	36	Shaikh U
3	Zaido	20	Manat	37	Bir Fad
4	Al Hasiki	21	Bir Jabir	38	Bir Jebi
5	Bir Am Saafi	22	Bir Amr	39	Hiswa
6	Al Kureishi	23	Noba' al Ubada	40	Bir Ama
7	As Saidain	24	Wght	41	Bir Ruba
8	Bir Salim	25	Bir Nassir	42	Bir Fad

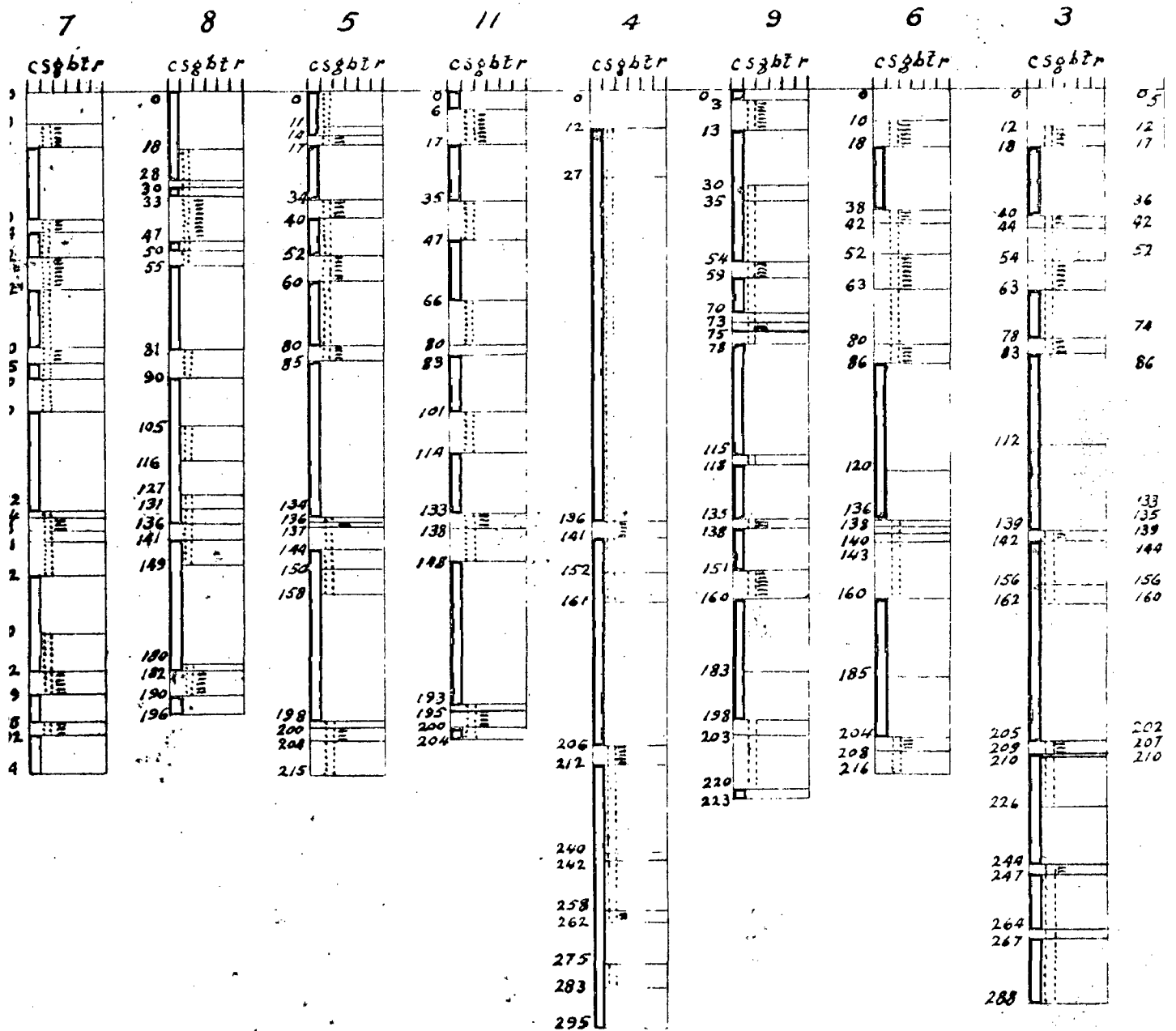




WADI TUBAN

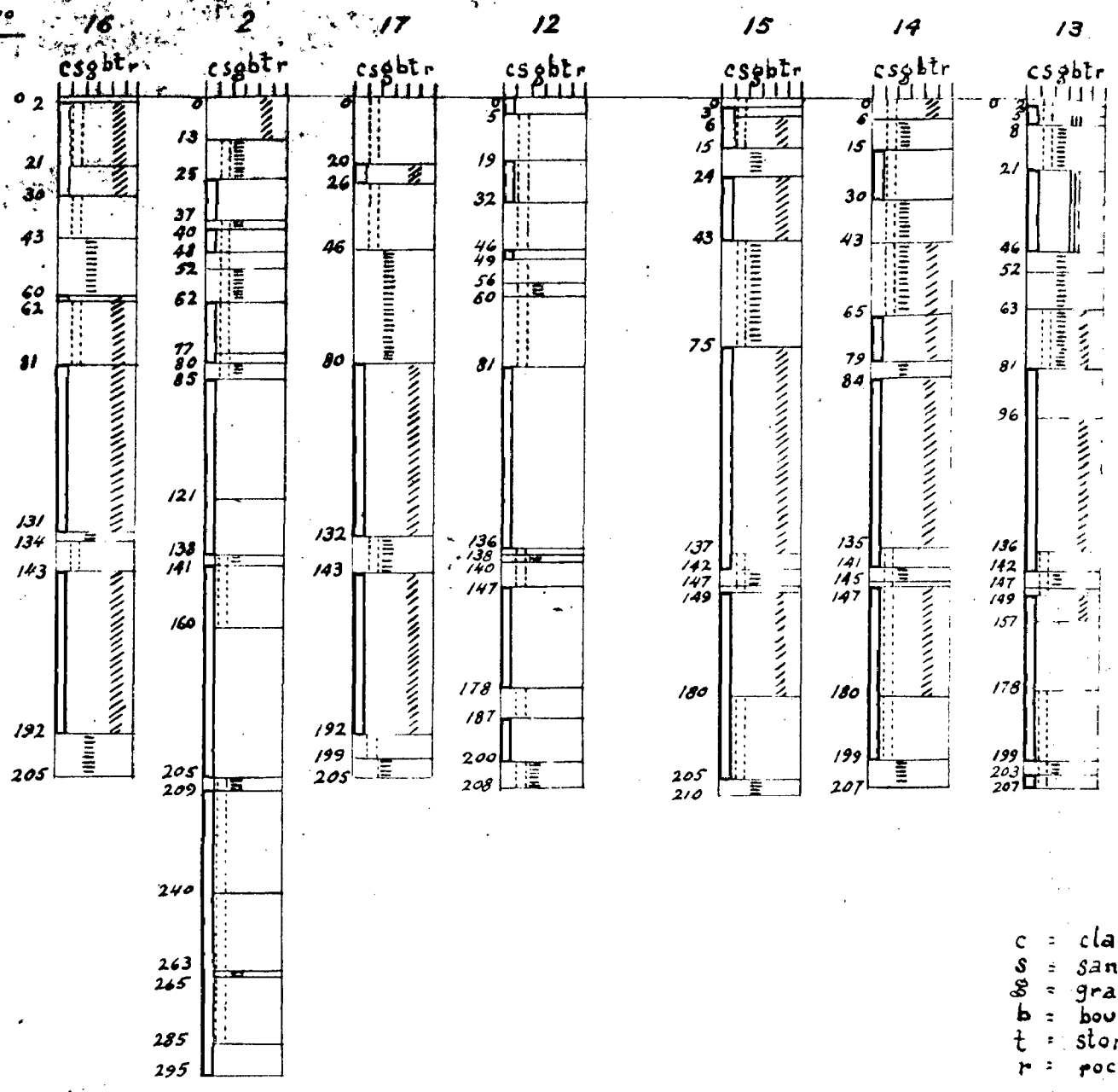
TYPICAL STRATA  
LOG

FIGURE 5



**DRILLER'S LOGS OF THE SHEIKH OTHMAN BOREHOLE.**  
(DEPTHS IN FEET)

do N°



DRILLER'S LOGS OF THE SHEIKH OTHMAN BOREHOLES  
(DEPTHS IN FEET)

NNW

SSE

Elevation at Borehole 7 about 175 feet above sea level

lev. feet

0

+0.33

+1.32

-2.89

-4.41

-1.88

+2.75

ve Bh. 1

borehole No

1

2

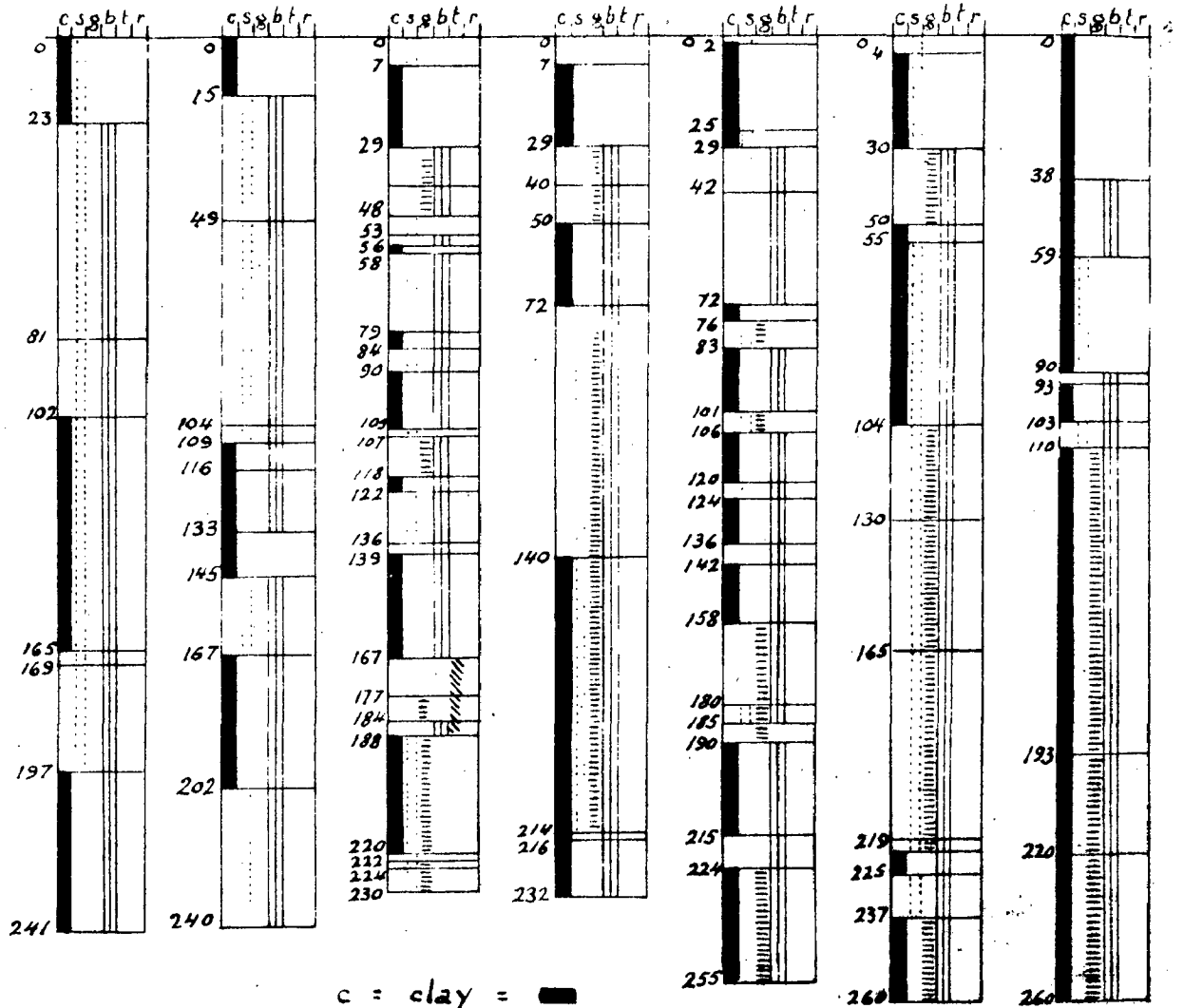
3

4

5

6

7



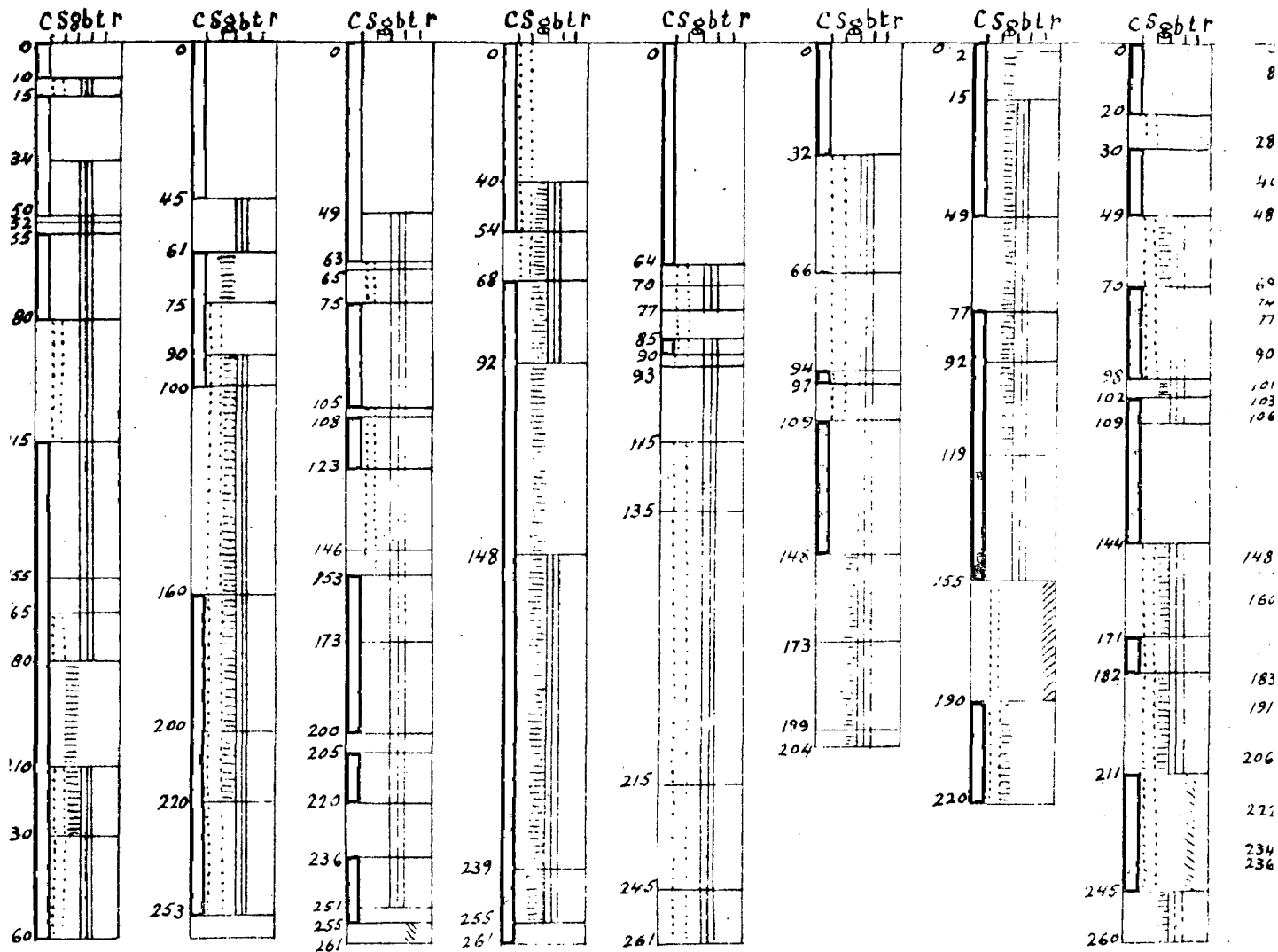
**DRILLER'S LOGS OF THE BIR NASIR BOREHOLE**  
(DEPTHS IN FEET)

NNW

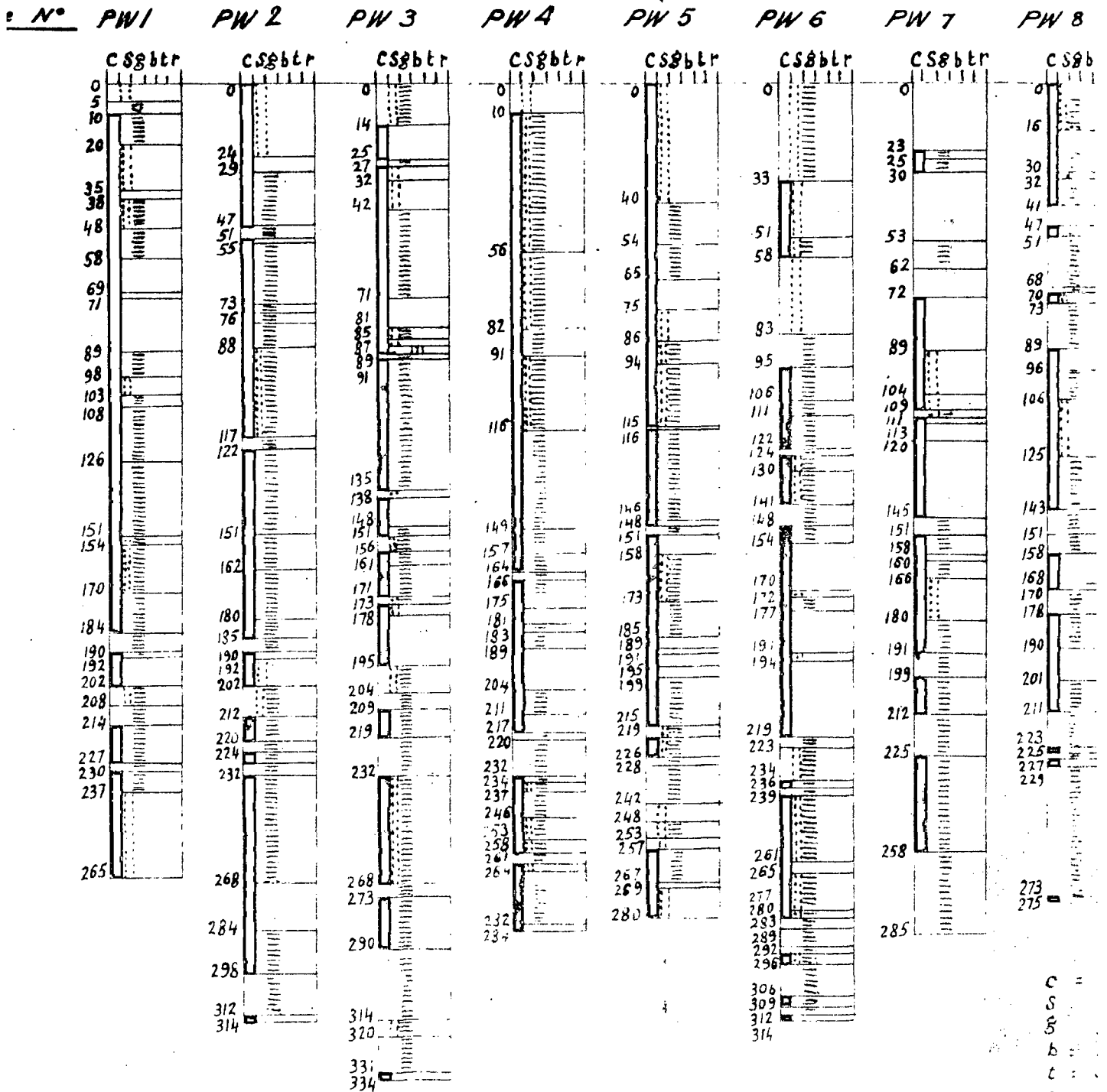
Elevation at Borehole 7 about 175 Feet

-0.34    +1.41    +3.12    +6.95    +11.10    +6.08    +11.42

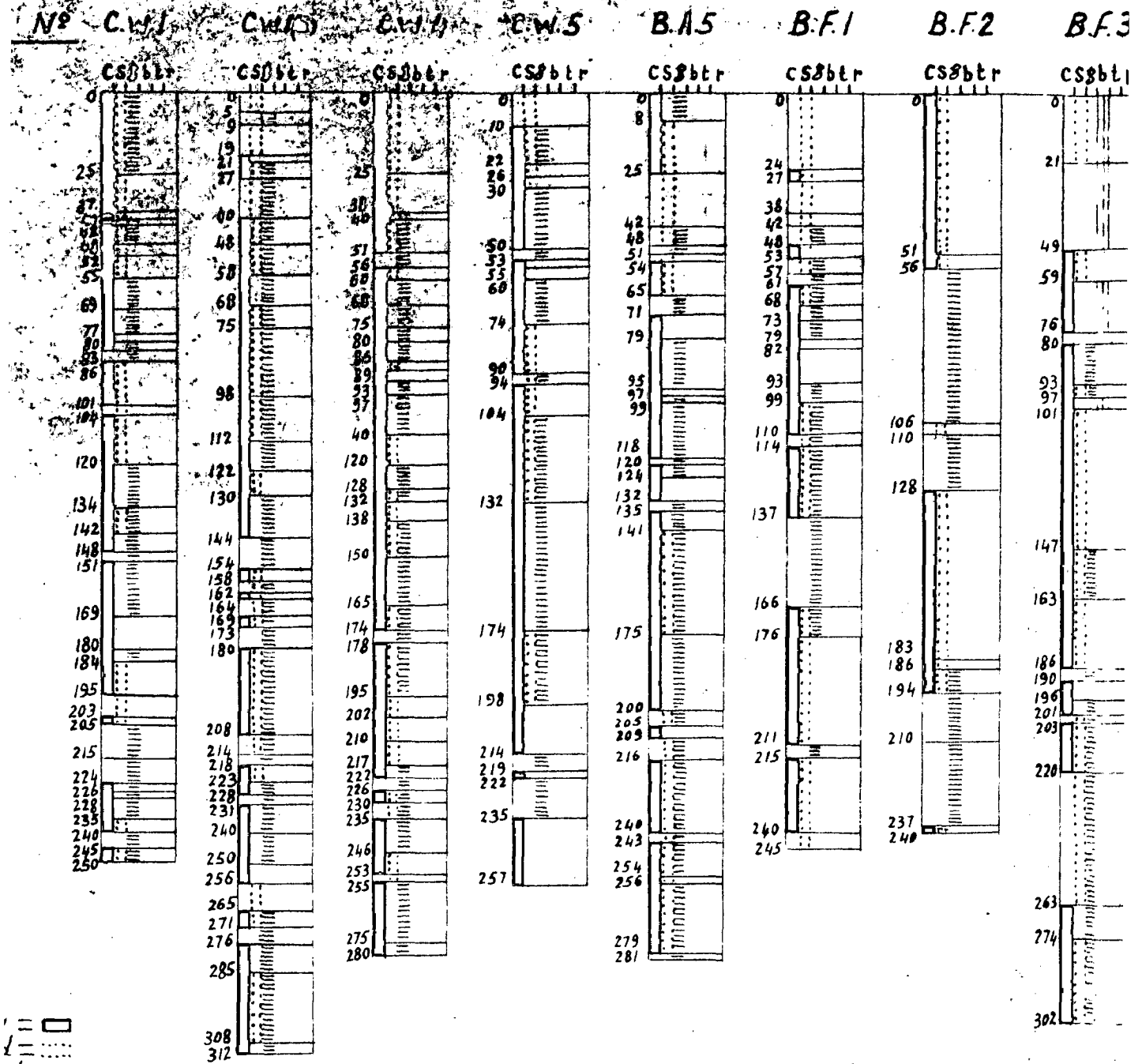
8            9            10           11           12           13           14           1



DRILLER'S LOGS OF THE BIR NASIR BOREHOLE  
(DEPTHS IN FEET)



**DRILLER'S LOGS OF THE BIR AHMED BOREHOLES**  
(DEPTHS IN FEET)



□ =   
 L =   
 el =   
 lers = |||   
 res =   
 = //

DRILLER'S LOGS OF THE BIR AHMED BOREHOLES  
 DEPTHS IN FEET

WELL-DEPTHS IN THE WADI TUBAI DELTA

0 5 10 15 20 25 Km.

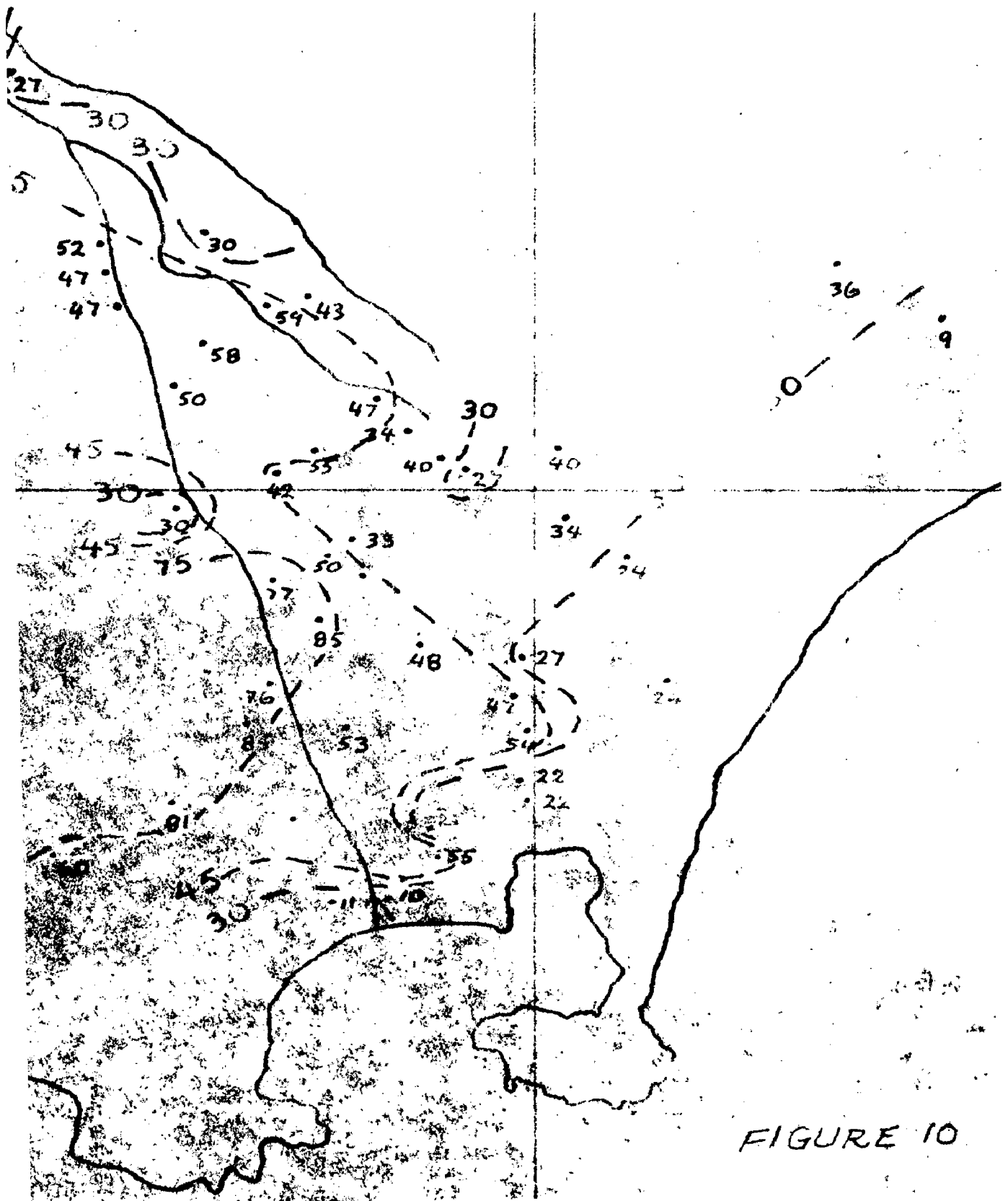


FIGURE 10

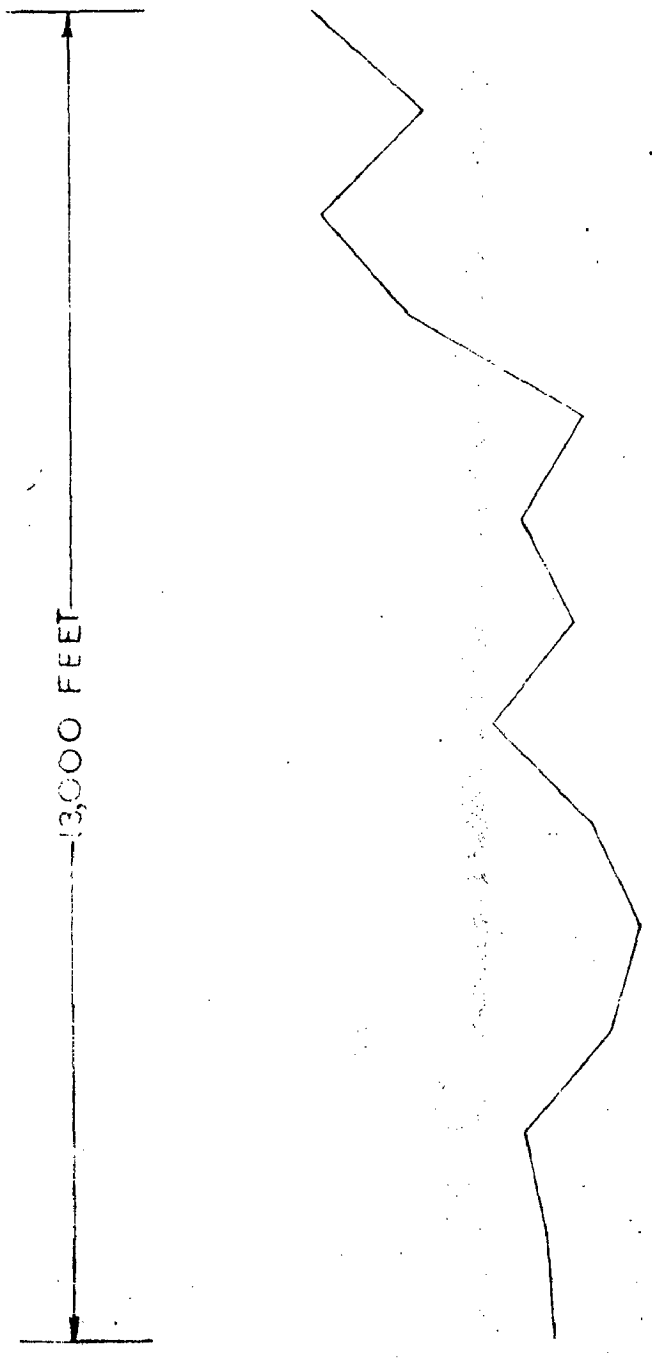


NEWPORT TRUST DATUM

130  
140  
150  
160  
170  
180  
190

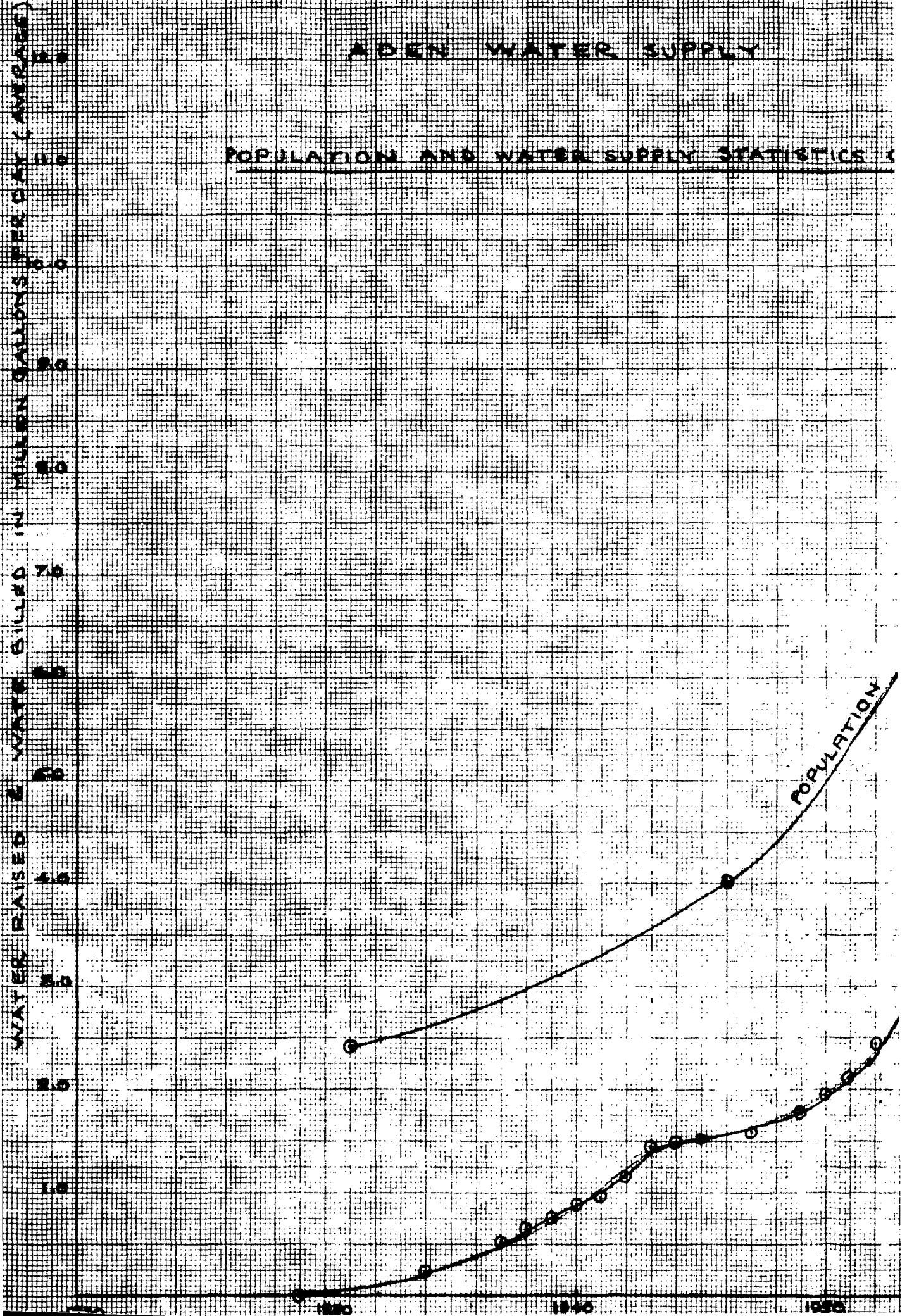
GROUND  
LEVEL

13,000 FEET



# ADEN WATER SUPPLY

## POPULATION AND WATER SUPPLY STATISTICS



LOCATION OF



Bore Well N° 8

Feet

Bore Well N° 11

Bore Wel.

107

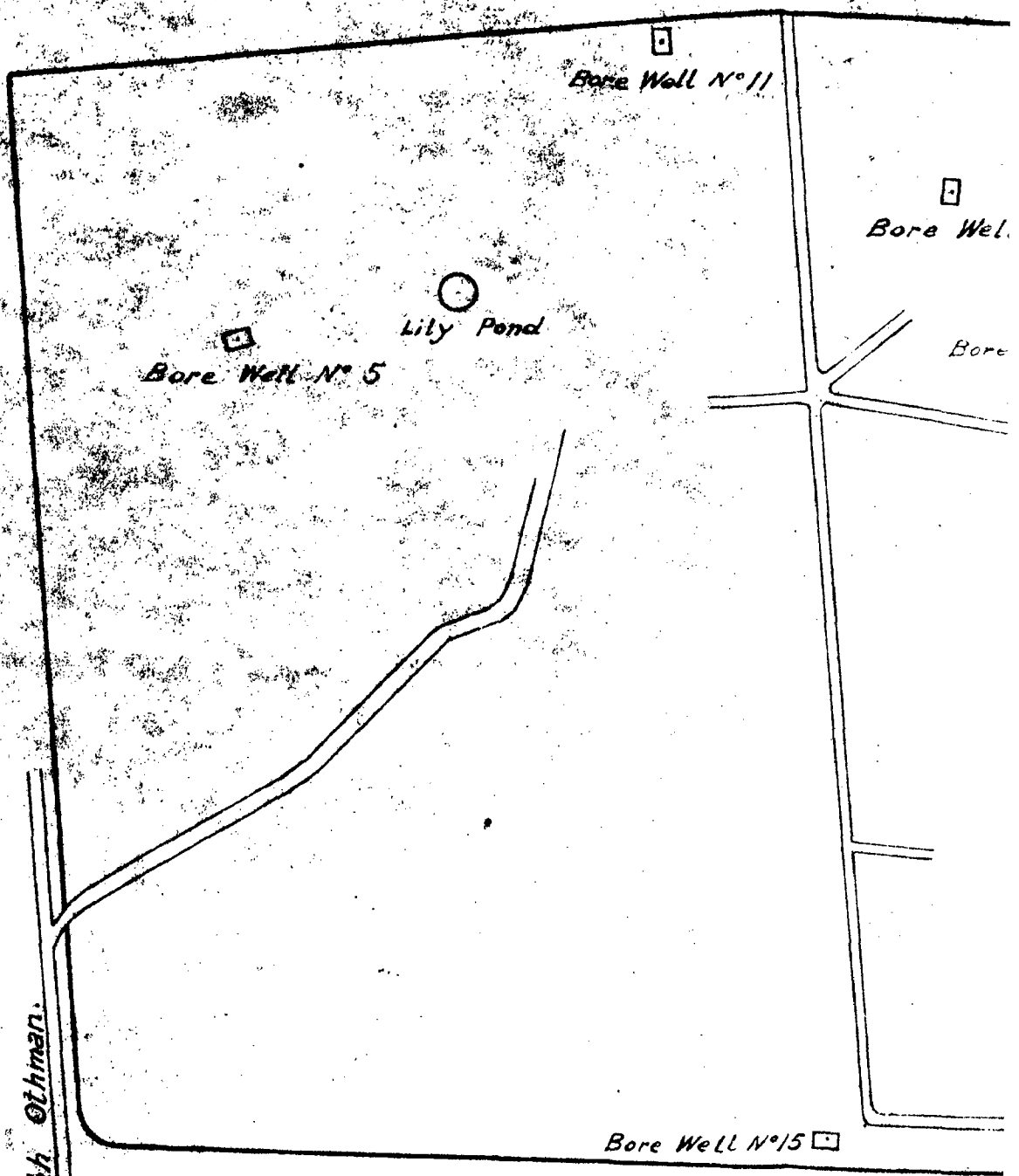
Bore Well N° 5

Lily Pond

Bore

from Sheikh Othman.

Bore Well N° 15



LIST OF FIGURES

FIG. NO.

TITLE

1. MAP AT THE ENTRANCE TO THE COUNTRY
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4. THE GREAT RIVER BASIN (1880)
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- 6A & B DETAILS OF THE GREAT RIVER BASIN
- 7A & B DETAILS OF THE GREAT RIVER BASIN
- 8A & B DETAILS OF THE GREAT RIVER BASIN
- 9 GREAT RIVER BASIN (1880)
10. GREAT RIVER BASIN (1880)
11. DETAILS OF THE GREAT RIVER BASIN
12. DETAILS OF THE GREAT RIVER BASIN
13. GREAT RIVER BASIN (1880)
14. DETAILS OF THE GREAT RIVER BASIN
15. DETAILS OF THE GREAT RIVER BASIN
16. DETAILS OF THE GREAT RIVER BASIN
17. DETAILS OF THE GREAT RIVER BASIN
18. DETAILS OF THE GREAT RIVER BASIN.

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